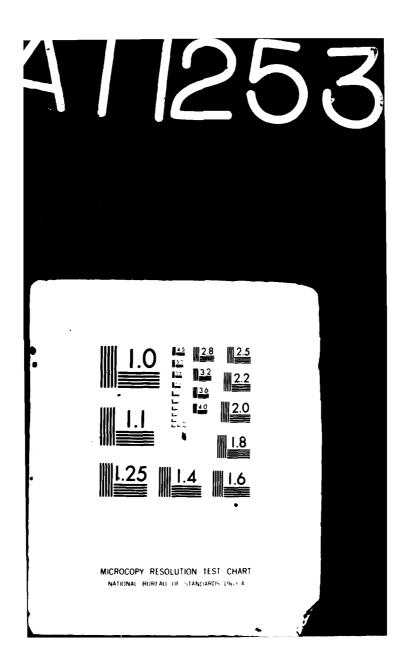
FUGRO NATIONAL INC LONG BEACH CA F/6 8/13 AX SITING INVESTIGATION PRELIMINARY GEOTECHNICAL INVESTIGATION—ETC(U) MAR 61 FN-TR-65-VOL-2 AD-A112 530 UNCLASSIFIED ha 3



	PHOTOGRAPH THI	S SHEET
53	LEVEL	INVENTORY
D-A112 3	LEVEL  FN-TK-  DOCUMENT IDENT	irication
A	This document has be for public release and distribution is unlimited.	een approved i sale; its ed.  ON STATEMENT
ACCESSION FOR		
NTIS GRAAI DTIC TAB UNANNOUNCED JUSTIFICATION		DTIC MAR 2 5 1982
BY DISTRIBUTION /		E
AVAILABILITY CO	IL AND/OR SPECIAL INSPECIAL	DATE ACCESSIONED
DISTRIB	UTION STAMP	
	8 2	4
	DATE RECEIVED	IN DTIC
	PHOTOGRAPH THIS SHEET AND	RETURN TO DTIC-DDA-2

DTIC FORM 70A

DOCUMENT PROCESSING SHEET

# MX SITING INVESTIGATION GEOTECHNICAL EVALUATION

# PRELIMINARY GEOTECHNICAL INVESTIGATION PROPOSED OPERATIONAL BASE SITE BERYL, UTAH

**VOLUME II - GEOTECHNICAL DATA** 

PREPARED FOR BALLISTIC MISSILE OFFICE (BMO) NORTON AIR FORCE BASE, CALIFORNIA



SECURITY CLASSIFICATION OF THIS PAGE (When Data Entered) READ INSTRUCTIONS REPORT DOCUMENTATION PAGE BEFORE COMPLETING FORM 2. GOVT ACCESSION NO. 3. RECIPIENT'S CATALOG NUMBER Investigation Proposed operational Base 5. TYPE OF REPORT & PERIOD COVERED Final site, Beryl, Utah Volume II - Sectechnical PERFORMING ORG. REPORT NUMBER pata FN-TR-45 B. CONTRACT OR GRANT NUMBER(S) 7. AUTHOR(s) Fugro National, Inc. . F 04704-80-6-0006 10. PROGRAM ELEMENT PROJECT, TALE 9. PERFORMING ORGANIZATION NAME AND ADDRESS Ertee Western Ine Country Tage Named PC. BCX 7765 64312 F Long Beach Ca 9050 F 11. CONTROLLING OFFICE NAME AND ADDRESS 12 REPORT DATE 2 Socie our Messile Stancis Colonisine 20 Mar 81 NUMBER OF PAGES 140 から すらしろから つうらにものり 14. MONITORING AGENCY NAME & ADDRESS(il different from Controlling Office) 15. SECURITY CLASS. (of this report) 156, DECLASSIFICATION DOWNGRADING SCHEDULE 16. DISTRIBUTION STATEMENT (of this Report) Distribution Unlimited 17. DISTRIBUTION STATEMENT (of-the abstract entered in Block 20, If different from Report) Distribution Configurated 18. SUPPLEMENTARY NOTES 19. KEY WORDS (Continue on reverse side if necessary and identify by block number)

Byring Trench Test Pit logs, lab test results,

cone penetrumeter sessions seismic refraction and, electrical resistivity triaxial compression sires show

20. ABSTRACT (Continue on reverse side II necessary and identity by block number). This report contains maps of boring, Trench and test pit logs locations.

Seismic-refraction data and electrical resistivity data for the Beryl, Utah, operating base location decribed in volume I of this report.

CERTIFIETY OF ALCIEDATION OF THIS DAWS (When Data Entered)

# MX SITING INVESTIGATION GEOTECHNICAL EVALUATION

PRELIMINARY GEOTECHNICAL INVESTIGATION PROPOSED OPERATIONAL BASE SITE BERYL, UTAH

VOLUME II - GEOTECHNICAL DATA

#### Prepared for:

U.S. Department of the Air Force Ballistic Missile Office Norton Air Force Base, California 92409

Prepared by:

Fugro National, Inc. 3777 Long Beach Boulevard Long Beach, California 90807

20 March 1981

#### FOREWORD

This volume of geotechnical data was compiled for the Department of the Air Force, Ballistic Missile Office (BMO), in compliance with Contract No. F04704-80-C-0006, CDRL Item 004A6. It contains the field data and laboratory test results from the investigation of the proposed Operational Base Site, Beryl, Utah. A synthesis of these data is available in Volume I.

The data in each section of this volume are preceded by an explanation of the format and terms used in the compilation.

#### TABLE OF CONTENTS

	Page
FOREWORD	i
1.0 ACTIVITY LOCATION MAP	In Pocket
2.0 EXPLANATION OF BORING, TRENCH, AND TEST PIT LOGS	1
3.0 EXPLANATION OF TRENCH LOGS	19
4.0 EXPLANATION OF TEST PIT LOGS	51
5.0 EXPLANATION OF LABORATORY TEST RESULTS	73
6.0 EXPLANATION OF CONE PENETROMETER TEST RESULTS	. 109
7.0 EXPLANATION OF SEISMIC-REFRACTION DATA	. 125
8.0 EXPLANATION OF ELECTRICAL RESISTIVITY DATA	. 134
Figure No.  2.0 EXPLANATION OF BORING, TRENCH,	
AND TEST PIT LOGS	
II-2-1 Log of Boring BL-B-1, Operational Base Site, Beryl, Utah	8
II-2-2 Log of Boring BL-B-2, Operational	•
Base Site, Beryl, Utah	9
Base Site, Beryl, Utah	10
Log of Boring BL-B-4, Operational Base Site, Beryl, Utah	11
II-2-5 Log of Boring BL-B-5, Operational	4.0
Base Site, Beryl, Utah	12 ·
Base Site, Beryl, Utah	13
Base Site, Beryl, Utah	14
II-2-8 Log of Boring BL-B-8, Operational Base Site, Beryl, Utah	15
II-2-9 Log of Boring BL-B-9, Operational	
Base Site, Beryl, Utah	16
Base Site, Bervl. Utah	17

Figure No.			Page
II-2-11		Boring BL-B-11, Operational Site, Beryl, Utah	18
	3.0	EXPLANATION OF TRENCH LOGS	
II-3-1		Trench BL-T-1, Operational Site, Beryl, Utah	20
II-3-2	Log of	Trench BL-T-2, Operational Site, Beryl, Utah	2
11-3-3	Log of	Trench BL-T-3, Operational Site, Beryl, Utah	22
11-3-4	Log of	Trench BL-T-4, Operational Site, Beryl, Utah	23
II-3-5	Log of	Trench BL-T-5, Operational Site, Beryl, Utah	24
11-3-6	Log of	Trench BL-T-6, Operational Site, Beryl, Utah	25
11-3-7	Log of	Trench BL-T-7, Operational Site, Beryl, Utah	26
11-3-8	Log of	Trench BL-T-8, Operational Site, Beryl, Utah	27
11-3-9	Log of	Trench BL-T-9, Operational Site, Beryl, Utah	28
11-3-10	Log of	Trench BL-T-10, Operational Site, Beryl, Utah	29
11-3-11	Log of	Trench BL-T-11, Operational Site, Beryl, Utah	3(
II-3-12	Log of	Trench BL-T-12, Operational Site, Beryl, Utah	31
II-3-13	Log of	Trench BL-T-13, Operational Site, Beryl, Utah	32
II-3-14	Log of	Trench BL-T-14, Operational Site, Beryl, Utah	33
II-3-15	Log of	Trench BL-T-15, Operational Site, Beryl, Utah	34
II-3-16	Log of	Trench BL-T-16, Operational Site, Beryl, Utah	35
II-3-17	Log of	Trench BL-T-17, Operational Site, Beryl, Utah	36
II-3-18	Log of	Trench BL-T-18, Operational Site, Beryl, Utah	37
II-3-19	Log of	Trench BL-T-19, Operational Site, Beryl, Utah	38
II-3-20	Log of	Trench BL-T-20, Operational Site, Beryl, Utah	39

Figure No.		Page
11-3-21	Log of Trench BL-T-21, Operational	40
II-3-22	Base Site, Beryl, Utah	41
II-3-23	Base Site, Beryl, Utah Log of Trench BL-T-23, Operational	491
	Base Site, Beryl, Utah	42
11-3-24	Log of Trench BL-T-24, Operational Base Site, Beryl, Utah	43
II-3-25	Log of Trench BL-T-25, Operational	44
II-3-26	Base Site, Beryl, Utah	44
	Base Site, Beryl, Utah	45
II-3-27	Log of Trench BL-T-27, Operational Base Site, Beryl, Utah	46
II-3-28	Log of Trench BL-T-28, Operational	47
II-3 <b>-</b> 29	Base Site, Beryl, Utah	4.7
TT 2 20	Base Site, Beryl, Utah	48
II-3-30	Log of Trench BL-T-30, Operational Base Site, Beryl, Utah	49
II-3-31	Log of Trench BL-T-31, Operational Base Site, Beryl, Utah	50
	4.0 EXPLANATION OF TEST PIT LOGS	
II-4-1	Log of Test Pit BL-P-1, Operational Base Site, Beryl, Utah	52
II-4-2	Log of Test Pit BL-P-2, Operational	53
II-4-3	Base Site, Beryl, Utah	
II-4-4	Base Site, Beryl, Utah Log of Test Pit BL-P-4, Operational	54
11-4-4	Base Site, Beryl, Utah	55
11-4-5	Log of Test Pit BL-P-5, Operational Base Site, Beryl, Utah	56
II-4-6	Log of Test Pit BL-P-6, Operational	
II- <b>4</b> -7	Base Site, Beryl, Utah	57
11-4-1	Base Site, Beryl, Utah	58
II-4-8	Log of Test Pit BL-P-8, Operational Base Site, Beryl, Utah	59
II-4-9	Log of Test Pit BL-P-9, Operational	
II- <b>4</b> -10	Base Site, Beryl, Utah Log of Test Pit BL-P-10, Operational	60
11-4-10	Base Site, Beryl, Utah	61

Figure No.		Page
II-4-11	Log of Test Pit BL-P-11, Operational Base Site, Beryl, Utah	62
II-4-12	Log of Test Pit BL-P-12, Operational	
II-4-13	Base Site, Beryl, UtahLog of Test Pit BL-P-13, Operational	63
II-4-14	Base Site, Beryl, Utah	64
II-4-15	Base Site, Beryl, Utah	65
	Base Site, Beryl, Utah	66
11-4-16	Base Site, Beryl, Utah	67
II-4-17	Log of Test Pit BL-P-17, Operational Base Site, Beryl, Utah	68
II-4-18		69
II-4-19		70
11-4-20	Log of Test Pit BL-P-20, Operational	
II-4-21	Base Site, Beryl, Utah	71
	Base Site, Beryl, Utah	72
	5.0 EXPLANATION OF LABORATORY TEST RESULTS	
11-5-1	Summary of Triaxial Compression Test Results, Operational Base Site,	
	Beryl, Utah	86
11-5-2	Summary of Direct Shear Test Results, Operational Base Site, Beryl, Utah	88-91
II-5-3	Consolidation Test Results, Operational Base Site, Beryl, Utah	93-101
11-5-4	Grain Size Curves, CBR Test, Operational Base Site, Beryl, Utah	
II <b>-</b> 5-5	California Bearing Ratio (CBR) Curves,	
	Operational Base Site, Beryl, Utah	107,100
	6.0 EXPLANATION OF CONE PENETROMETER TEST RESULTS	
11-6-1	Cone Penetrometer Test Results, Operational Base Site, Bervl, Utah	110-124

Figure No.			Page
	7.0	EXPLANATION OF SEISMIC REFRACTION DATA	
11-7-1		Seismic Refraction Line BL-S-1, Time Distance Data and Velocity Profile, Operational Base Site, Milford, Utah	128
II-7-2		Seismic Refraction Line BL-S-2, Time Distance Data and Velocity Profile, Operational Base Site, Beryl, Utah	129
11-7-3		Seismic Refraction Line BL-S-3, Time Distance Data and Velocity Profile Operational Base Site, Beryl, Utah	130
11-7-4		Seismic Refraction Line BL-S-4, Time Distance Data and Velocity Profile, Operational Base Site, Beryl, Utah	131
II-7-5		Seismic Refraction Line BL-S-5, Time Distance Data and Velocity Profile,	
11-7-6		Operational Base Site, Beryl, Utah Seismic Refraction Line BL-S-6, Time Distance Data and Velocity Profile,	132
		Operational Base Site, Beryl, Utah	133
		8.0 EXPLANATION OF ELECTRICAL RESISTIVITY DATA	
II-8-1		Resistivity Sounding BL-R-1, Sounding Curve and Interpretation, Operational Base Site, Beryl, Utah	135
II→8-2		Resistivity Sounding BL-R-2, Sounding Curve and Interpretation, Operational	136
11-8-3		Base Site, Beryl, Utah	
11-8-4		Base Site, Beryl, Utah	137
II <b>-</b> 8-5		Base Site, Beryl, Utah	138
11-8-6		Base Site, Beryl, Utah	139
		Base Site, Beryl, Utah	140

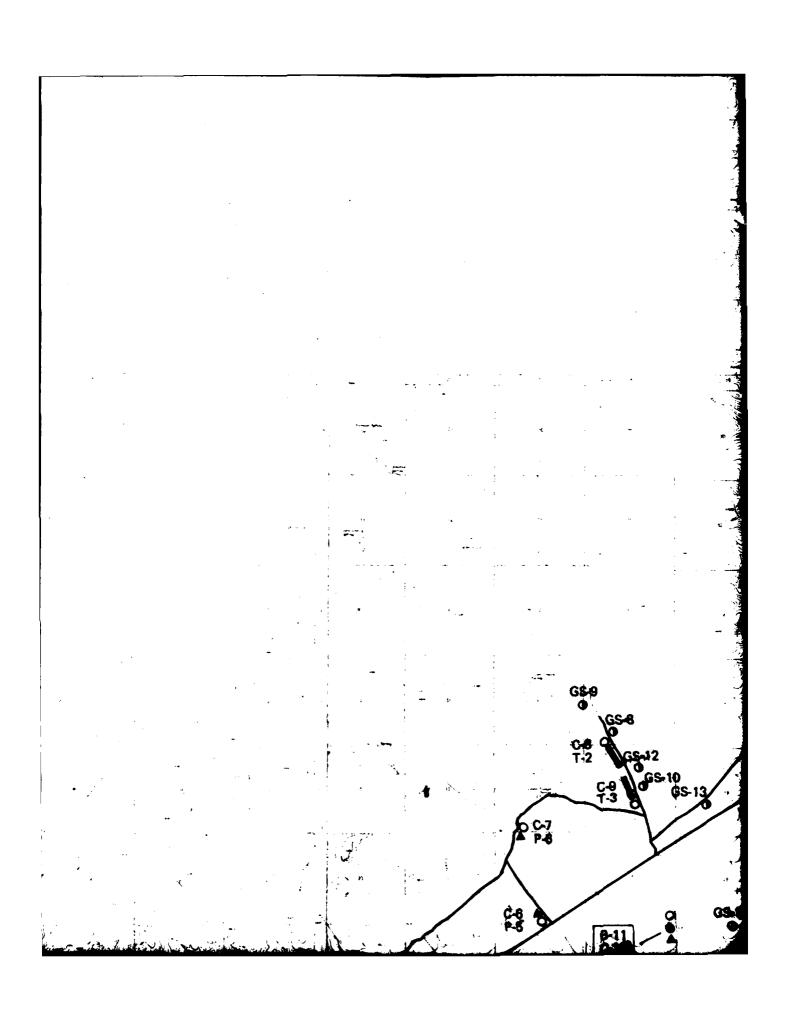
#### LIST OF TABLES

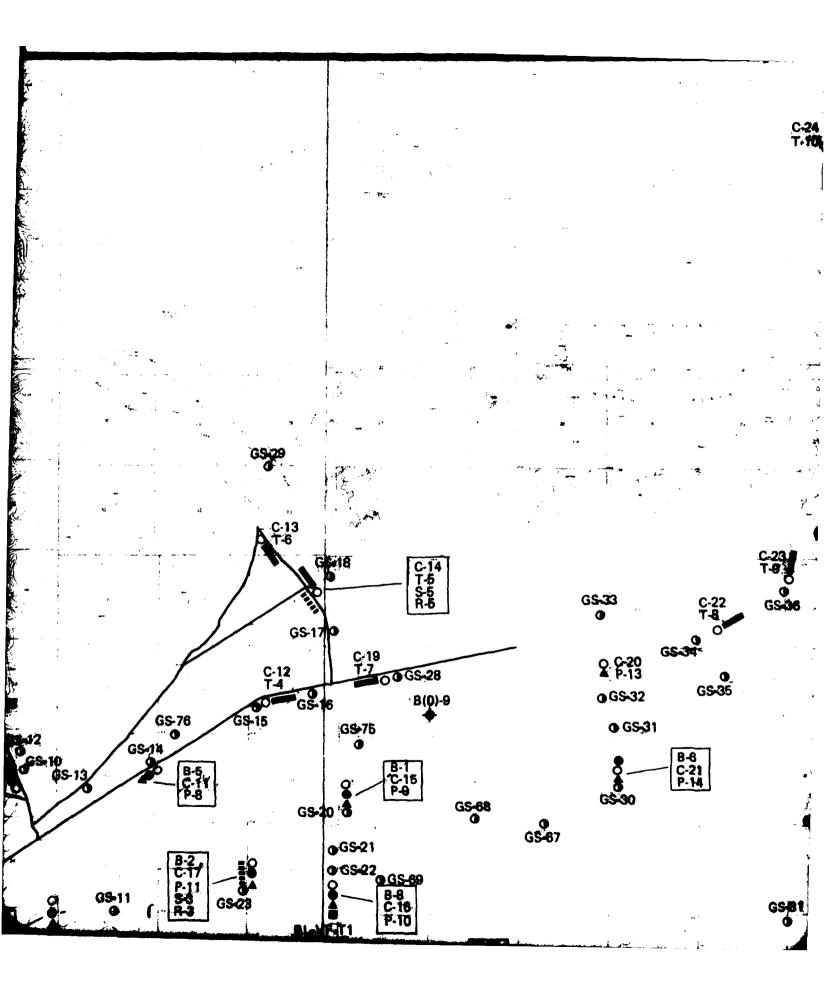
Table No.		<u>Page</u>
	2.0 EXPLANATION OF BORING, TRENCH, AND TEST PIT LOGS	
11-2-1	Unified Soil Classification System	3
5.0	EXPLANATION OF LABORATORY TEST RESULTS	
11-5-1	Summary of Laboratory Test Results, Operational Base Site, Beryl, Utah	79,85
11-5-2	Summary of Unconfined Compression Test Results, Operational Base Site, Beryl,	
11-5-3	Utah	87 102
11-5-4	California Bearing Ratio (CBR) Test Results, Operational Base Site, Milford, Utah	105,106
7.0	EXPLANATION OF SEISMIC-REFRACTION DATA	
11-7-1	Shallow Seismic Refraction Velocity Profile, Operational Base Site, Beryl, Utah	127

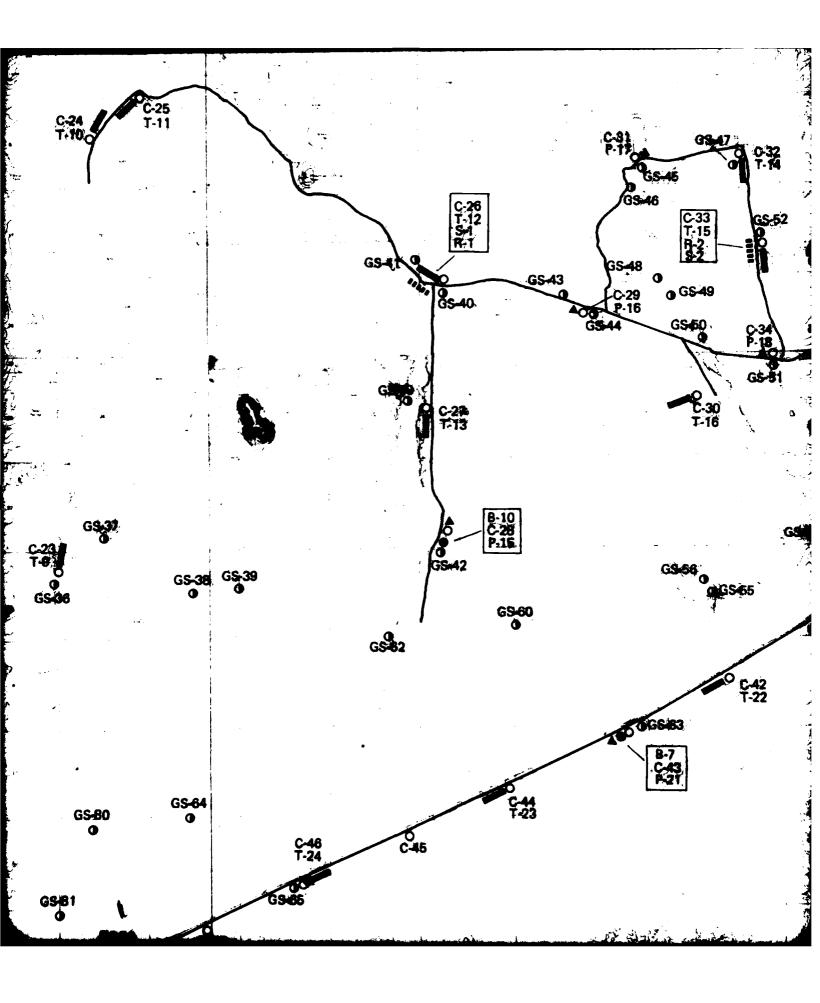
SECTION 1.0

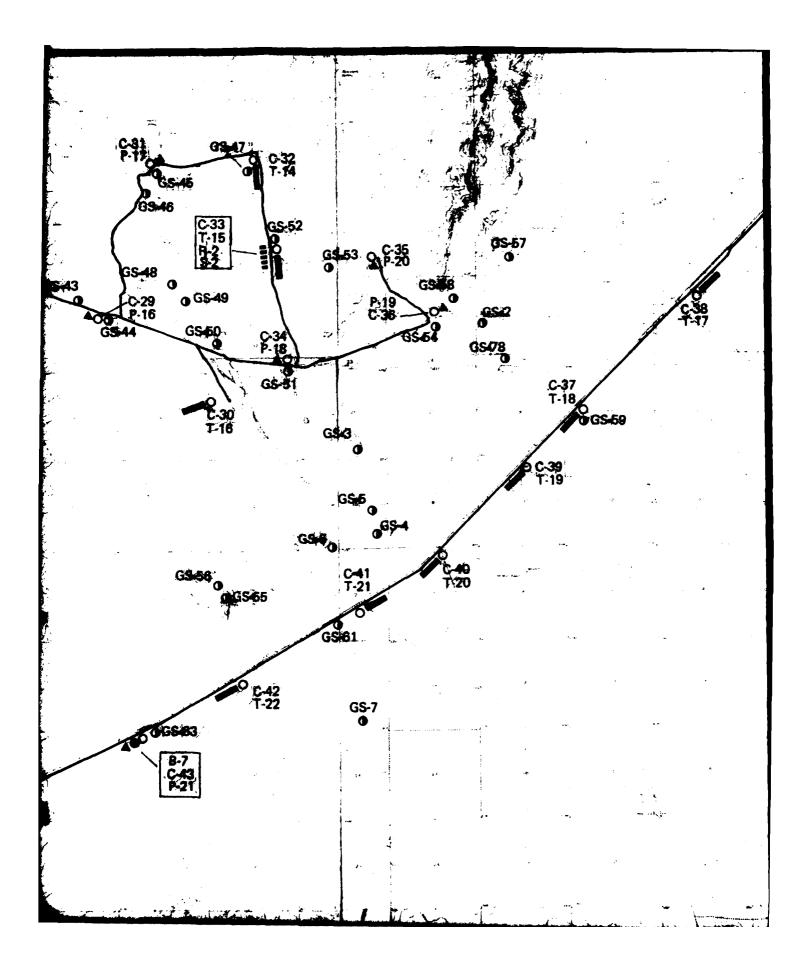
ACTIVITY LOCATION MAP

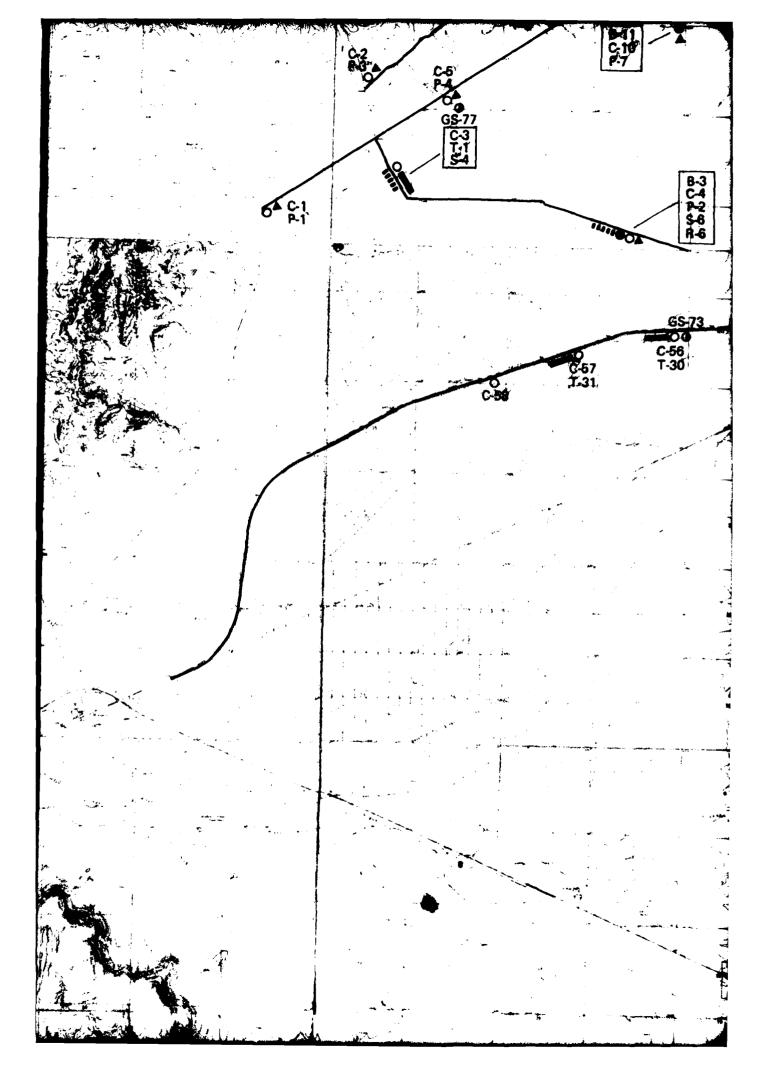
(IN POCKET)

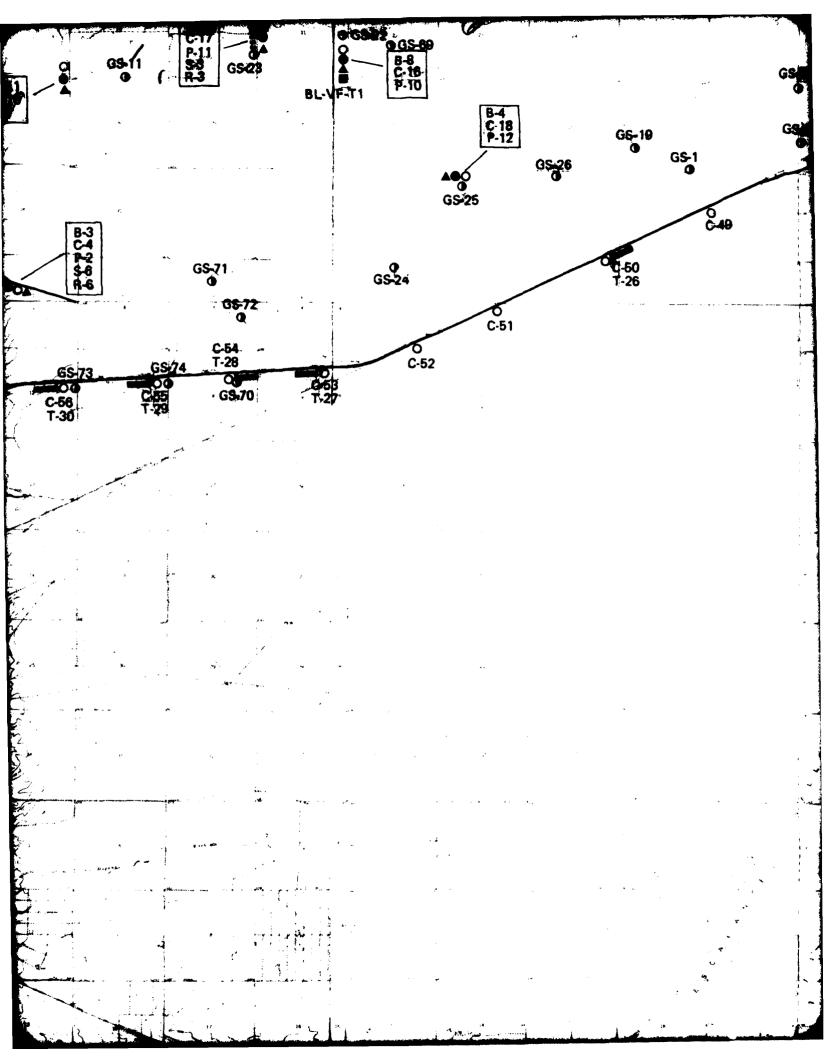


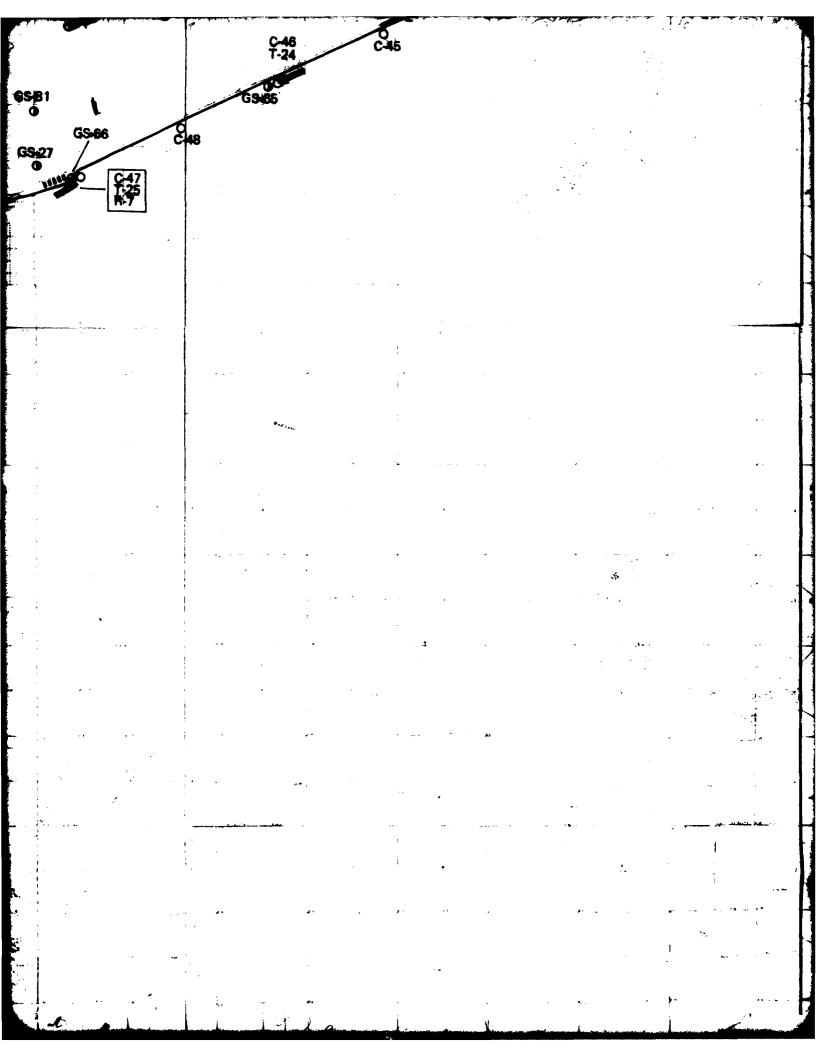


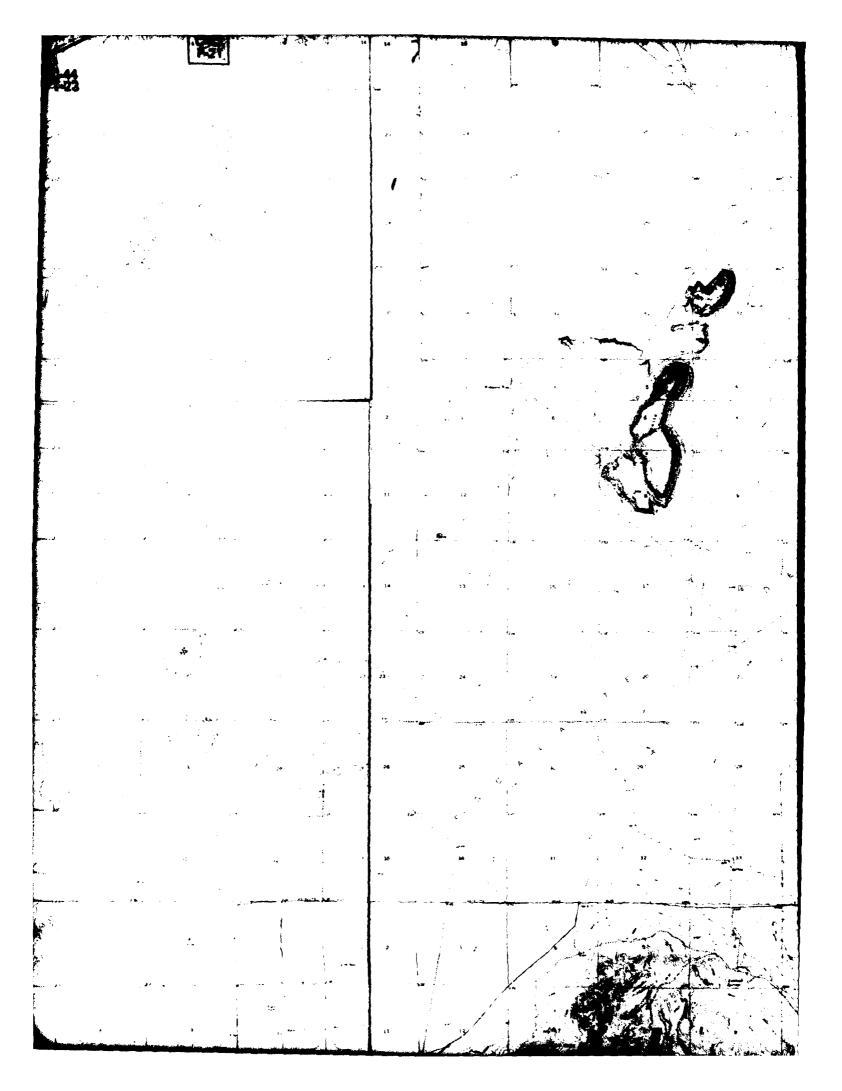


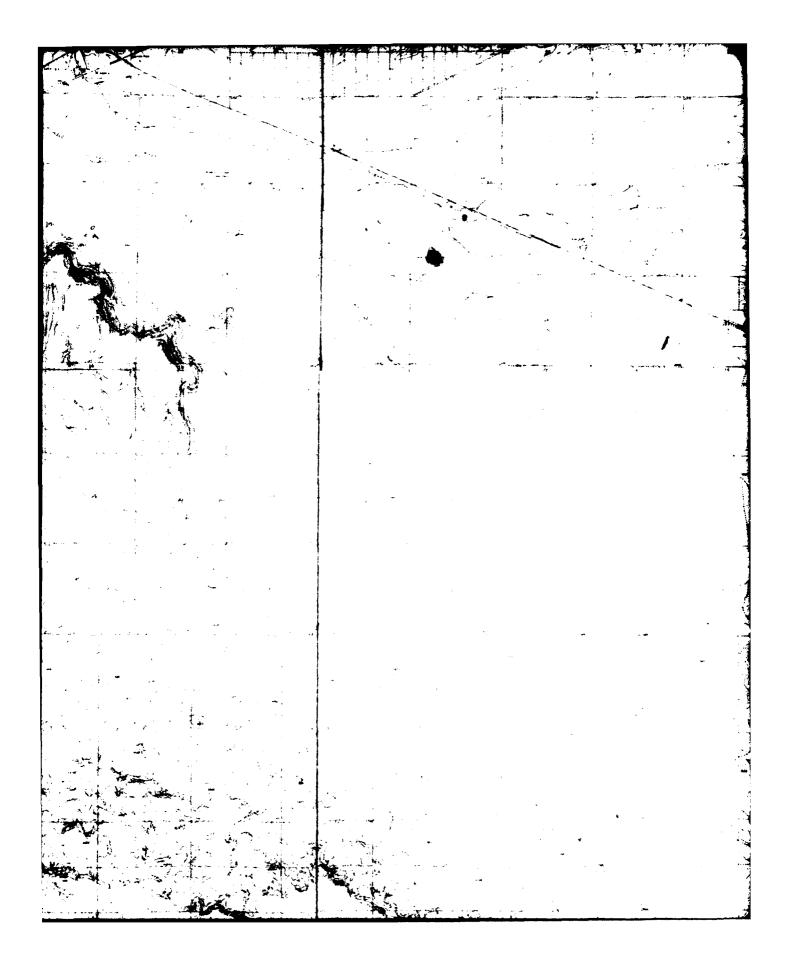


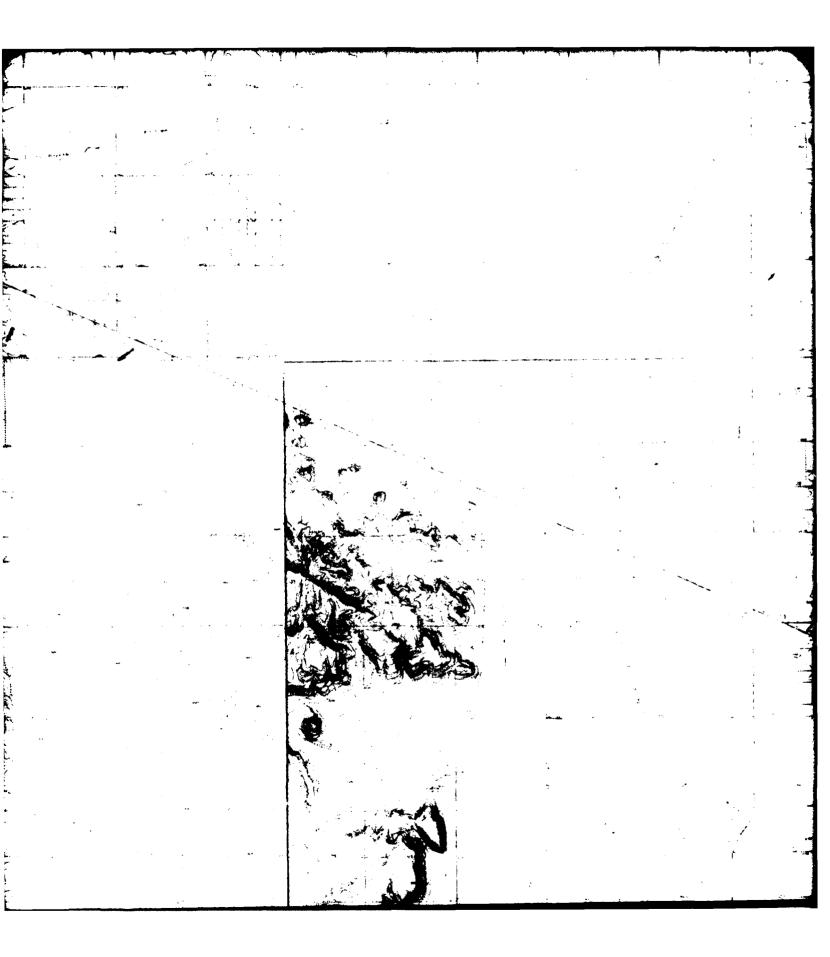


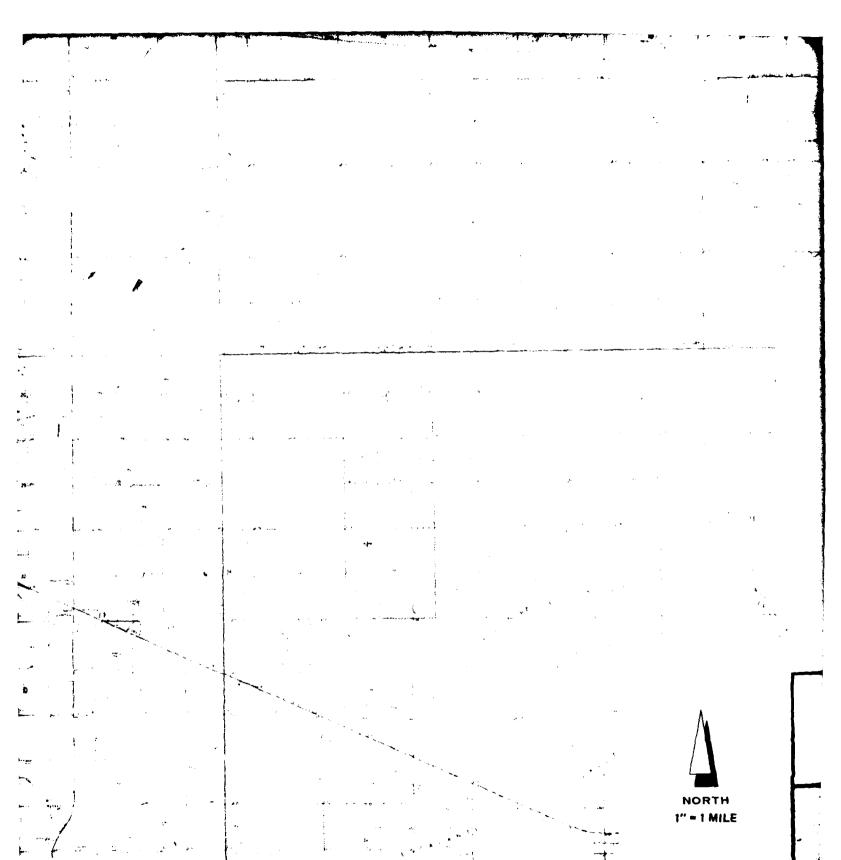














#### **EXPLANATION**

•	GS-1	GEOLOGIC STATION
<b>♦</b>	B(0)-1	BORING WITH OBSERVATION WELL
•	B-1	BORING
0	C-1	CONE PENETROMETER TEST (CPT)
_	T-1	TRENCH
•	P-1	TEST PIT
	S-1	SEISMIC REFRACTION LINE
*****	R-1	ELECTRICAL RESISTIVITY LINE
	BL-VF-T1	FUGRO WATER RESOURCES WELL

NOTE: Due to the exaggration of the map symbols, the exact location of any combination of activities is where either the boring (1st) or the CPT (2nd) is situated. Single activities are most securely located nearest the centur of the symbol.



# ACTIVITY LOCATION MAP OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION

DEPARTMENT OF THE AIR FORCE — BMO

DRAWING

II-1-1

<u>rugro national, inc.</u>

### SECTION 2.0

EXPLANATION OF BORING, TRENCH, AND TEST PIT LOGS

#### 2.0 EXPLANATION OF BORING, TRENCH, AND TEST PIT LOGS

All data from borings, trenches, and test pits are presented on standard Fugro National logs in Sections 2.0, 3.0, and 4.0. Explanations of the column headings on the logs are as follows:

A. Designations - Borings, trenches, and test pits are identified as follows:

BL-B-1

BL - abbreviation for the site (e.g., BL-Beryl)

- abbreviation for activity (e.g., B-boring,
T-trench, P-test pit)

- number of activity

- B. Sample Type Different sampling techniques were used and the symbols are explained at the bottom of the boring logs. For details of sampling techniques, see Section A4.0 of Appendix in Volume I. Horizontal lines, to scale, indicate the depth where sampling was attempted.
- C. Percent Recovery The numbers shown represent the ratio (in percent) of the soil sample recovered in the sampler to the full penetration of the sampler.
- D. N Value Corresponds to standard penetra ion resistance which is the number of blows required to drive a standard split-spoon sampler for the second and third of three 6-inch (15-cm) increments with a 140-pound (63.5-kg) hammer falling 30 inches (76 cm) (ASTM D 1586-67).

- E. Depth Corresponds to depth below ground surface in meters and feet.
- F. Lithology Graphic representation of the soil and rock types.
- G. USCS Unified Soil Classification System symbols (see Table II-2-1 for complete details).
- H. Soil Description Except in cases where samples were classified based on laboratory test data, the descriptions are based on visual classification. The procedures outlined in ASTM D 2487-69, Classification of Soils for Engineering Purposes, and D 2488-69, Description of Soils (Visual-Manual Procedure), were followed. Solid lines across the column indicate known change in strata at the depth shown.

Definitions of some of the terms and criteria to describe soils and conditions encountered during the exploration follow.

Gradation: A coarse-grained soil is well graded if it has a wide range in grain size and substantial amounts of most intermediate particle sizes.

Poorly graded indicates that the soil consists predominantly of one size (uniformly graded) or has a wide range of sizes with some intermediate sizes obviously missing (gap-graded).

Moisture: Dry - no feel of moisture

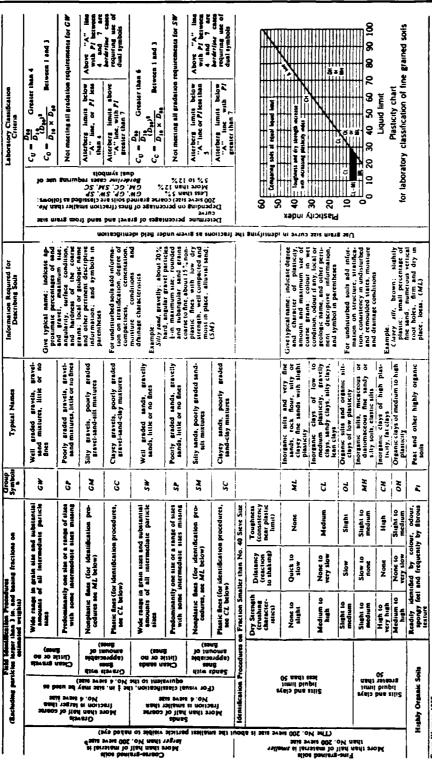
Slightly Moist - much less than normal moisture

Moist - normal moisture for soil Very Moist - much greater than normal

moisture

Wet - for soils below the water

table



From Wagner, 1957.

Manufacry classification. Soils possessing characteristics of two groups are designated by combinations of group symbols. For example GW-GC, well graded gravel-sand matters with clay binder

Mil sere sizes on that chart are U.S. standard.

Field Identification Procedure for Fire Grand Soils or Fractions

Distancy (Reactions to the principles on the ministration are the partiest, Affer tremong parties larger than No. 40 sere and, prepare a part of most soil with a volume of about once half cube, rich. Add enough waster if necessary to make it has old for but on itself.

Pace the part as the open palm of one hand and whate horizontally, striking viscocoupy against the other hand several time. A posture reaction committee in a page palm of one hand and whate horizontally, striking viscocoupy against the other hand several time. A posture reaction committee is a puested between the fineer on the sarface of the past which changes to a livery consistency and becomes allowy. When the sample is appearance to water during shaking and of its disappearance during equectage assets in descripting the craster or cumbles. The rapidity of appearance of water during shaking and of its disappearance during equectage assets in descripting the exhauster of the fines in a soil.

Very fine clean stands are take queckets and most distinct reaction whereas a paint, city has no reactions or longance still, such as a typical rock

TABLE

Ⅲ-2-1

All seve the characteristics of two groups are cases are to be performed on the minus No. 40 seve size particles, appointmently 14, in. For field characteristics.

Their convertibility of the consistency of the characteristics of the characteristics of the particles larger than No. 40 seve size, appears a particle state than 10 to the consistency of pully, adding water if recessary to show and read on the minus No. 40 seve size, appears a particle state than 10 to the consistency of pully, adding water if recessary to about one-half cube inch.

The dry competitive of the characteristic and quantity of th

UNIFIED SOIL CLASSIFICATION SYSTEM

MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - SAMSO

Grain Shape:

Consistency: Consistency descriptions of coarse-grained soils (GW, GP, GM, GC, SW, SP, SM, SC) are as follows.

Consistency	N Value (ASTM D 1586-67)
Very Loose	0 - 4
Loose	4 - 10
Medium Dense	10 - 30
Dense	30 - 50
Very Dense	>50

Consistency descriptions of fine-grained soils (ML, CL, MH, CH,) are as follows:

Consistency	Shear S (ksf) (	I	Field Guide
Very Soft	0.25	12	Sample with height equal to twice the diameter, sags under own weight
Soft	0.25- 0.50	12 - 24	Can be squeezed between thumb and forefinger
Firm	0.50- 1.00	24- 48	Can be molded easily with fingers
Stiff	1.00- 2.00	48- 96	Can be imprinted with slight pres- sure from fingers
Very Stiff	2.00- 4.00	96- 192	Can be imprinted with considerable pressure from fingers
Hard	over 4.00	over 192	Cannot be im- printed by fingers
Angular -	relative		sharp edges and e sides with aces.
Subangular -			similar to angular nat rounded edges.

Subrounded - particles exhibit nearly plane sides but have well-rounded corners and edges.

Rounded - particles have smoothly curved sides and no edges.

Plasticity: Plasticity index is the range of water content, expressed as a percentage of the weight of the oven-dried soil, through which the soil is plastic. It is defined as the liquid limit minus the plastic limit. Descriptive ranges used on the logs include:

Nonplastic (PI, 0 - 4) Slightly Plastic (PI, 4 - 15) Medium Plastic (PI, 15 - 30) Highly Plastic (PI, >30)

Cobbles and Boulders

A cobble is a rock fragment, usually rounded by weathering or abrasion, with an average diameter ranging between 3 and 12 inches (8 and 30 cm).

A boulder is a rock fragment, usually rounded by weathering or abrasion, with an average diameter of 12 inches (30 cm) or more.

- I. Remarks This column was provided on boring and trench logs for comments regarding drilling difficulty, number and size of cobbles or boulders encountered, loss of drilling fluid in the boring, trench wall stability, and other conditions encountered during drilling and excavations.
- J. Dry Density and Moisture Content The boring logs include a graphical display of laboratory test results for dry density (ASTM D 2937-71) in pounds per cubic foot and kilograms per cubic meter and moisture content (ASTM D 2216-71) in percent from representative samples taken during drilling. The symbols are explained at the bottom of the boring logs.

- K. Sieve Analysis The numbers represent the percentage by dry weight (ASTM D 422-63) of each of the following soil components:
  - GR Gravel, rock particles that will pass a 3-inch (76-mm) sieve and are retained on No. 4 (4.75 mm) sieve.
  - SA Sand, soil particles passing No. 4 sieve and retained on No. 200 (0.075 mm) sieve.
  - FI Fines, silt or clay, soil particles passing No. 200 sieve.
- L. Atterberg Limits (LL and PI) -
  - LL Liquid Limit, the water content corresponding to the arbitrary limit between the liquid and plastic states of consistency of a soil (ASTM D 423-66).
  - PL Plastic Limit, the water content corresponding to an arbitrary limit between the plastic and the semisolid state of consistency of a soil (ASTM D 424-59).
  - PI Plasticity Index, numerical difference between the liquid limit (LL) and the plastic limit (PL) indicating the range of moisture content within which a soilwater mixture is plastic.
  - NP Nonplastic.
- M. Miscellaneous Information -
  - Elevations indicated elevations on the logs are estimated from topographic maps of the study area, within an accuracy of half the contour interval.
  - Surficial
  - Geologic Unit indicates the surficial geologic unit in which the activity is located.
  - Date Drilled indicates the period from beginning to completion of the activity.
  - Drilling
  - Method signifies the type of drilling procedure used such as rotary wash.
  - Hole Diameter nominal size of boring drilled.

Water Level - indicates depth from ground surface to water table where encountered.

Trench Length - length at ground surface of final trench excavation.

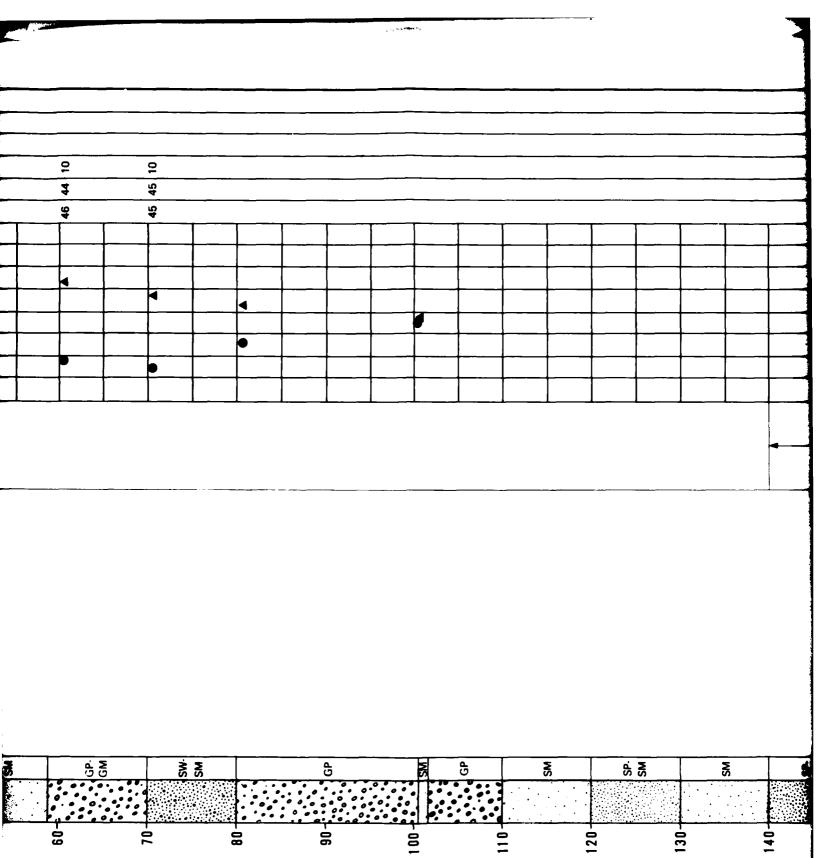
Trench

Orientation - bearing of longitudinal trench centerline.

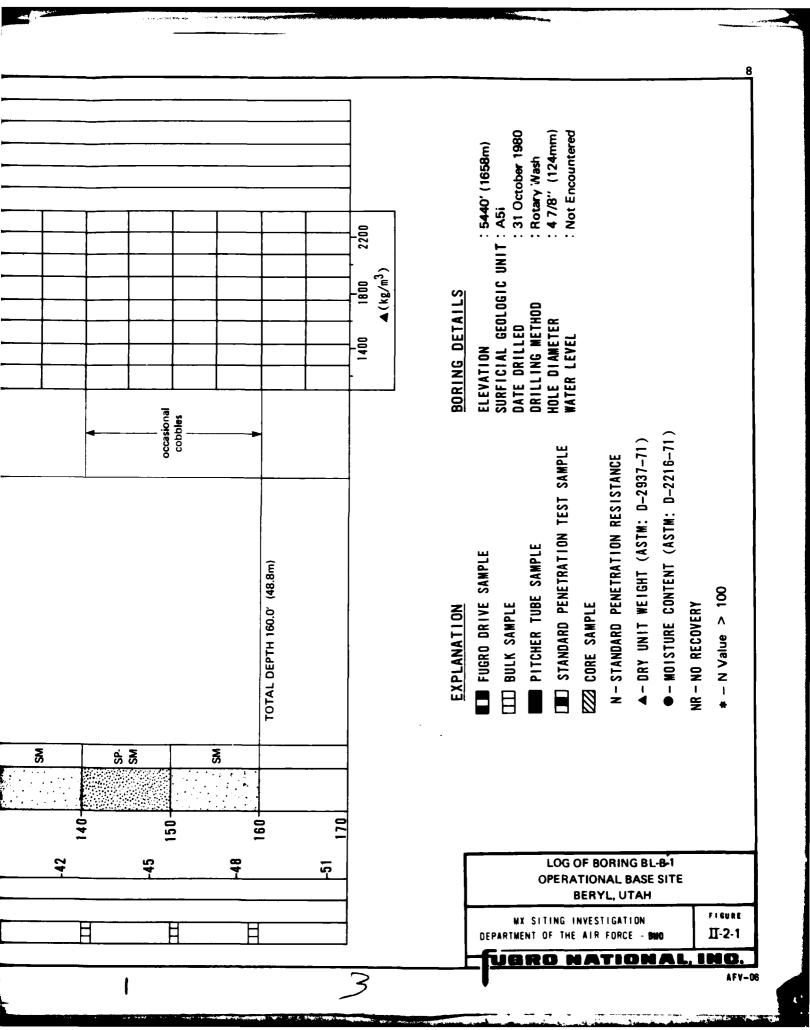
SECTION 3.0

EXPLANATION OF TRENCH LOGS

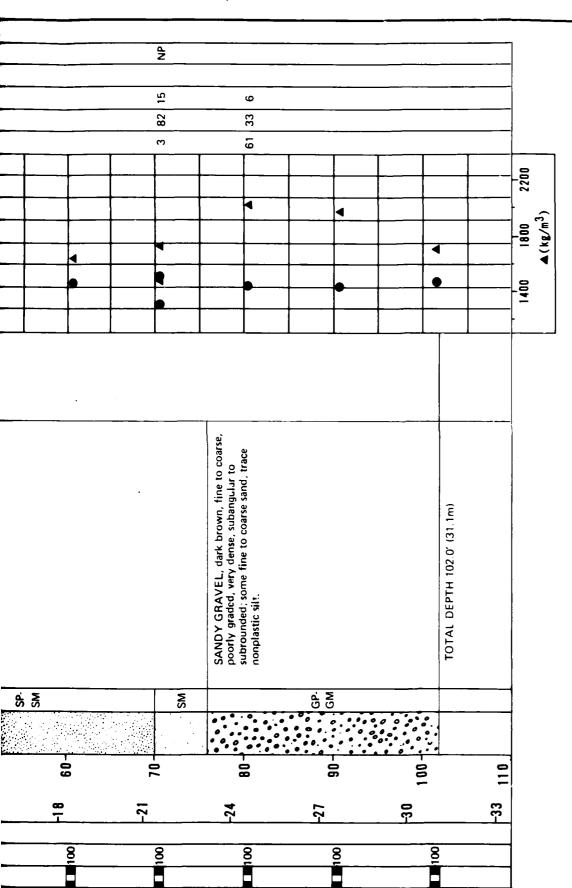
	11   11											
S	FIL		<u></u>		33		<u></u>			- 27	·	
SIEVE	SAF		61	<del></del>	62				<u> </u>	55		
SI	GR S	_	9	-8-			<u> </u>		<del></del>	<u> </u>		· ·
			T		$\overline{}$			T		1		
▲(pcf) 100 110 120 130 140	35			$\top$	_	†		1				
0 13	30		1					4				
▲(pcf)	20 25 ● (%)					•	<b>—</b>			1		1
<b>→</b>				4		+						
	15	11	•			•						
08 0	10									•		
80	5											
DESCRIPTION		AVEL and SAND:	SP-GM): dark brown, ded, dense to very	ounded; little to some nonplastic silt.	SM, SP-SM): dark Il to poorly graded, ingular to subrounded;	rse gravel; trace - 15.0').	y dense, subangular little to some non - ne to coarse gravel.					
SOIL DESCR		rbedded layers of GR/ \VEL:	IDY GRAVEL (GP, ( to coarse, poorly grav	e, subangular to subro to coarse sand; trace	ID: NVELLY SAND (SW In, fine to coarse, we e to very dense, subi	o to some fine to coa plastic silt; sand (7.5'	IY SAND (SM): dark tily graded, dense to ver ibrounded, calcareous; dc silt; none to little fil					
) I L		Interbedded layers of GRAVEL and SAND: SM GRAVEL:		dense, subangular to subrounded; little to some SW- fine to coarse sand; trace nonplastic silt.		trace to some fine to coarse gravel; trace nonplastic silt; sand (7.5' - 15.0').	SILTY SAND (SM): dark brown, fine to coarse, poorly graded, dense to very dense, subangular to subrounded, calcareous; little to some non-plastic silt; none to little fine to coarse gravel.		. SW			WS.
Nacs Nacs	.11	WS		-MS		d <sub>D</sub>	· · · · · · · · · · · · · · · · · · ·	-0	SW			
USCS NOIL	1334	WS.					ğ •	9 30-	ASS.	40-		50
THOLOGY USES	METE FEET	WS 0		10		20 GP	ğ •		WS WS		er T	50
USCS NOIL	METE FEET	WS 0		10		20 GP	ğ •		NS SW	40-		50



-27



		<u>-</u>	ā. Z		o Z								
		11	<u> </u>										
	SIEVE	1	20		4							<del></del>	
	SIEVE	SA	46		26	62					9		
	A	GR	34		<u>د</u>	98		<del>,</del>		<del></del>	33	<del></del>	
	6-	35		<u> </u>		<b></b>		<b>↓</b>	<del> </del>	<del> </del>			
	30 1	30	<del></del>	ļ		<b>_</b>		<b>↓</b>	<del> </del>	<u> </u>	↓	1 4	
	20 1	ا ا		<u> </u>		<del> </del>		ļ	<b>_</b>	<u> </u>	1		
	<b>▲</b> (pcf)	20 2 • (%)	•					•	4		•		
	▲(pcf) 100 110 120 130 140	15 2	◀	•	4			<u> </u>					
	1 06			<u> </u>					<u> </u>				
	6 -	5 1	• •	•		•			•	•			
			<u> </u>						1.		}		
	REMARKS		•			cementation, cobbles and boulders	throughout			-			
	SOIL DESCRIPTION		GRAVELLY SAND, light to dark brown, fine to coarse, poorly graded, medium dense to dense subangular to subtrounded references	little to some fine to coarse gravel; little to some non- to slightly plastic silt.			SILTY SAND (SM): light brown to dark brown, fine to coarse, poorly graded, very	little to some non- to slightly plastic silt; trace fine to coarse gravel.					
	naca		SM	ಹಿ	SS	SW.	SS	8	SM	S.			SM
	A9070H.												
	DEPTH	F333	0	•	<b>-</b>	6	7	,	<u>0</u>		-04	50-	9
	RS =	3T3M	0	~	,	ر <u>د</u> ۱	•	G	מ	ç	71	고 오	<b>∞</b>
	AVENE	N		<u>e</u>								- <del></del>	<del></del>
	ECONERY		<u>8</u> 8	<u>\$</u>	100	8		90	8	8	<u>8</u>	8	1
	34YT 3J	SAMF	-66				=	Ė		Ė		HE	
•													



# BORING DETAILS

: 5360' (1634m) SURFICIAL GEOLOGIC UNIT : A5i ELEVATION

HOLE DIAMETER

NATER LEVEL

STANDARD PENETRATION TEST SAMPLE

ZZZ CORE SAMPLE

PITCHER TUBE SAMPLE

FUGRO DRIVE SAMPLE

**EXPLANATION** 

THE BULK SAMPLE

:1 November 1980 DRILLING METHOD DATE DRILLED

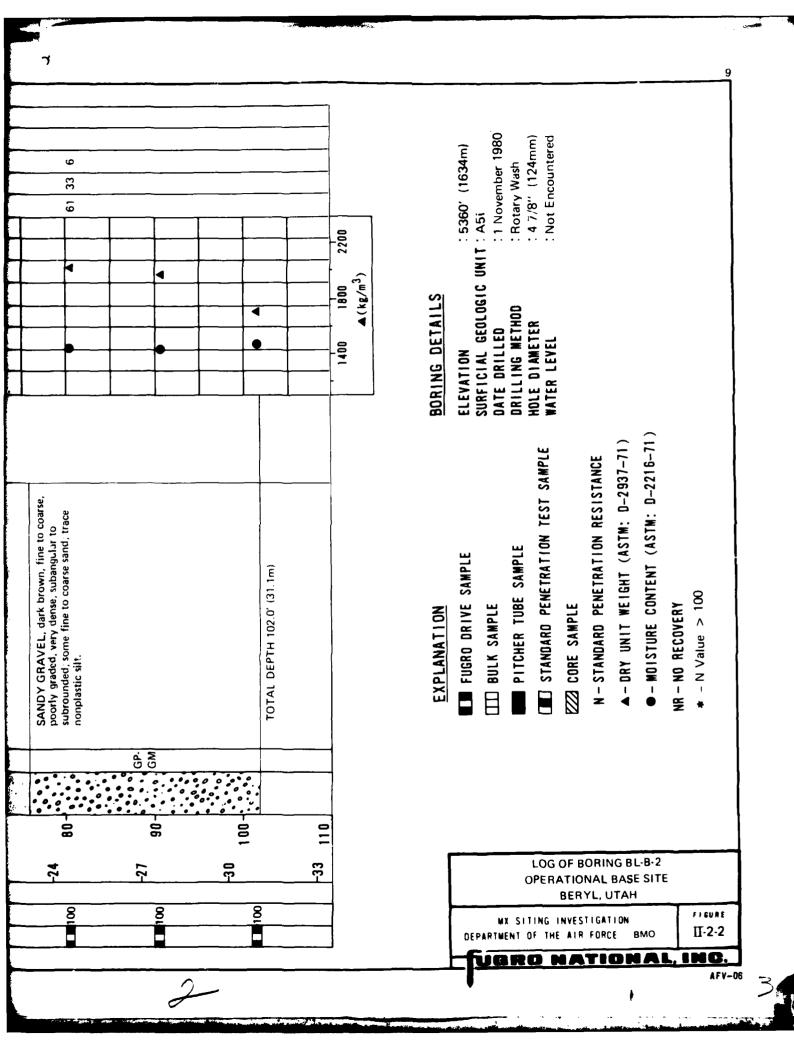
4 7/8" (124mm) Rotary Wash

Not Encountered

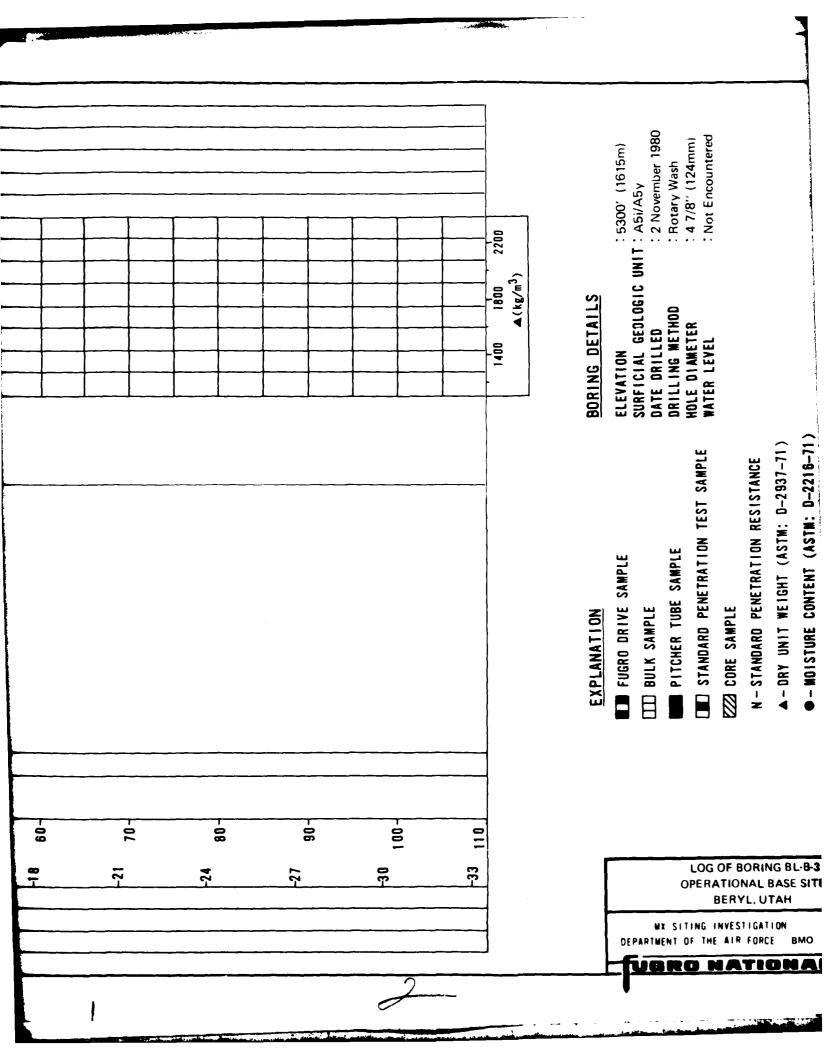
- BAR JIMIT MELONT (ACTM. B. 9497-71) N - STANDARD PENETRATION RESISTANCE

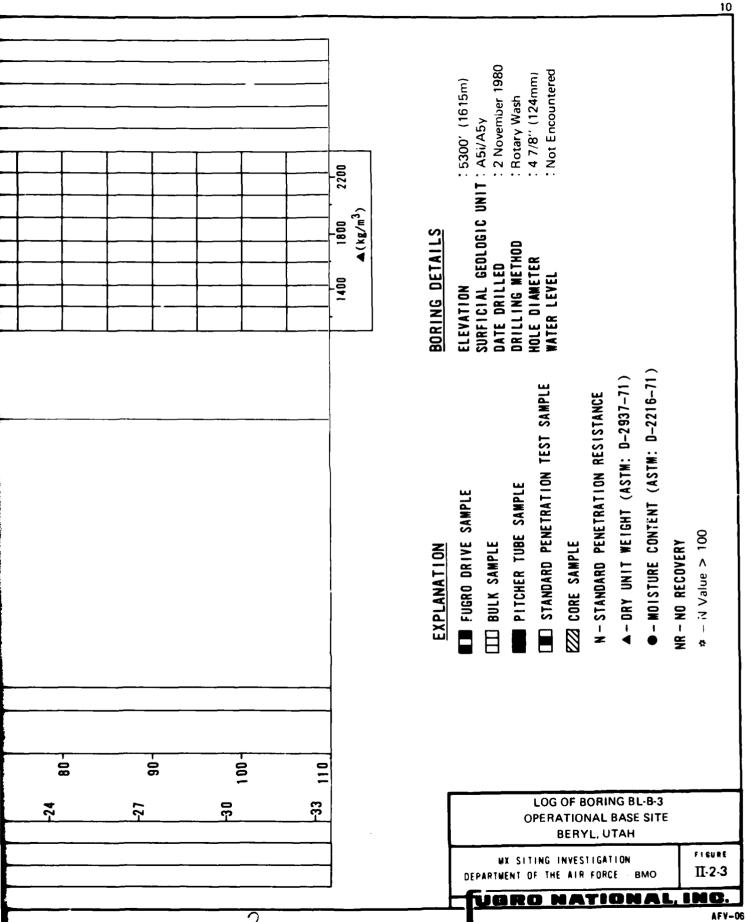
LOG OF BORING OPERATIONAL BA BERYL, UT

MX SITING INVESTIGAT

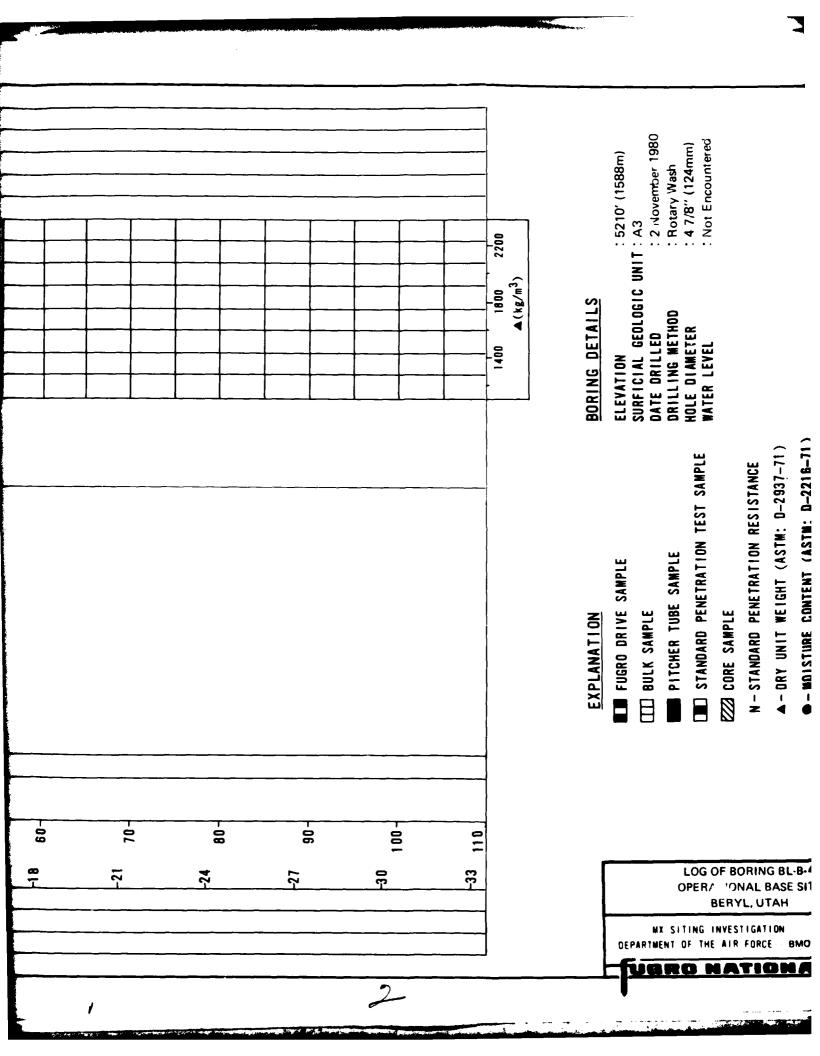


	Ы				120	7 70				∞	Z d		<u>م</u> 2			
<u></u>	11				¥ <del>4</del>	1 52				32						
VE	14				88	8			<del>-</del>	2 67	7 26					
SIEVE	S SA				94	16	<del></del>		2	32	67	<del></del>	=			
¥	GR			<del>- 1</del> -	00		<del>- 1</del>		∞ T	<del>-</del>		<del>- T</del>		·		
9-	35		+								+					
30	유		+		+	-					+	-+	<del></del>	-		
cf)	(%)	-	+-		-+			4			+	$\dashv$		-		
▲(pcf) 100 110 120 130 140	200										+			-	$\dashv$	-+
	-==		+•	-	7					4	+	-	-			-+
8-	-=		+		4			,		<del></del>	+•			<del> </del>		
8-	-20	_	+						<del>                                     </del>		+-	-		<del> </del>	_	-+
	L,	<b></b> -						<del></del>	i	<del></del>				-		L
REMARKS																
REX																
SOIL DESCRIPTION		Interbedded layer: of GRAVEL, SAND, and FINES:		brown, f dense, ar	<del></del>			subangular to subrounded, very dense, subangular to subrounded, calcareous, little to some nonplastic silt, trace fine gravel;		SANDY SILT (MH, ML): light brown to brown, hard, non- to medium plastic, calcareous; some fine subangular to subsounded good site (AD).				TOTAL DEPTH 50.0 (15.2m)		
nzcz		S GS	; ;	5	ت ا	5	₹	∑ S	SW	₹ 1	SM	9	ξ			
HOLOGY	LI1	. 0		000			o						· 96			
<b>=</b>	1331	0		0		20-		<u></u>	) )		40-		0	200		-09
283 EPH	METE	0		က		9		<b>5</b>		,	7		<u>.</u> 5			-18
AVENE	N	<u> </u>	77					<del></del>								
ECOVERY	¥ %	Š	٤	3	100	9		3 &	}	8	8		67			
	SAME			H								77				



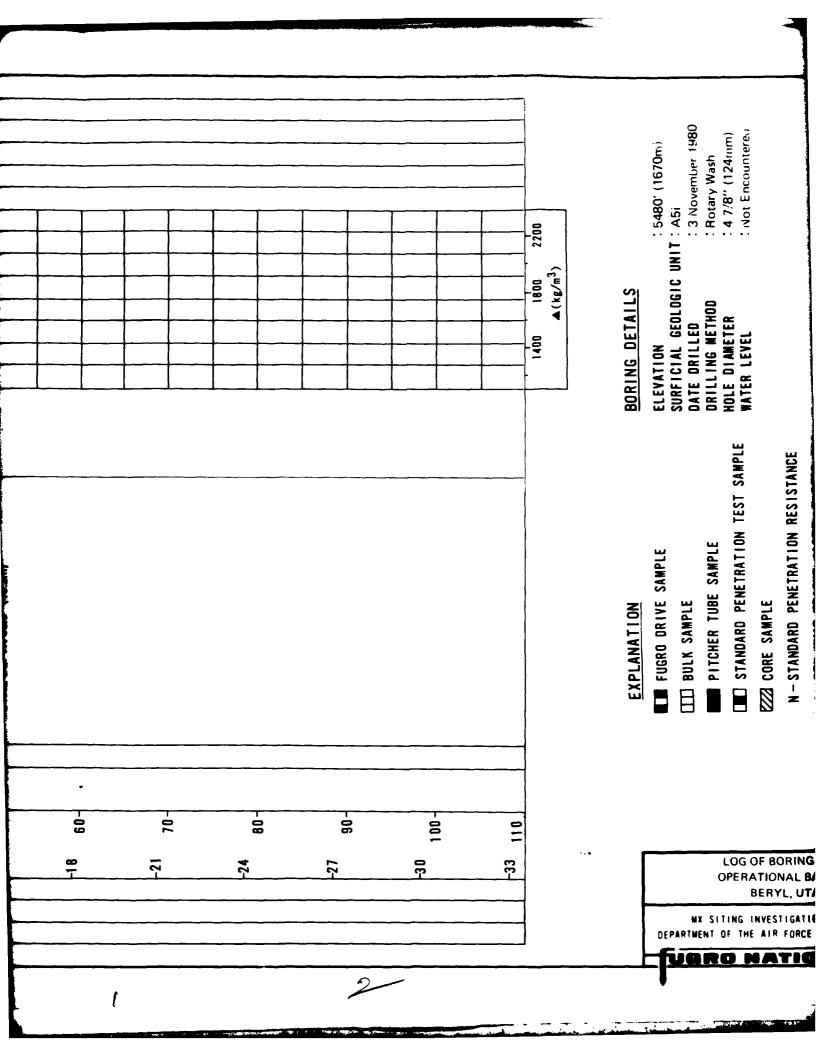


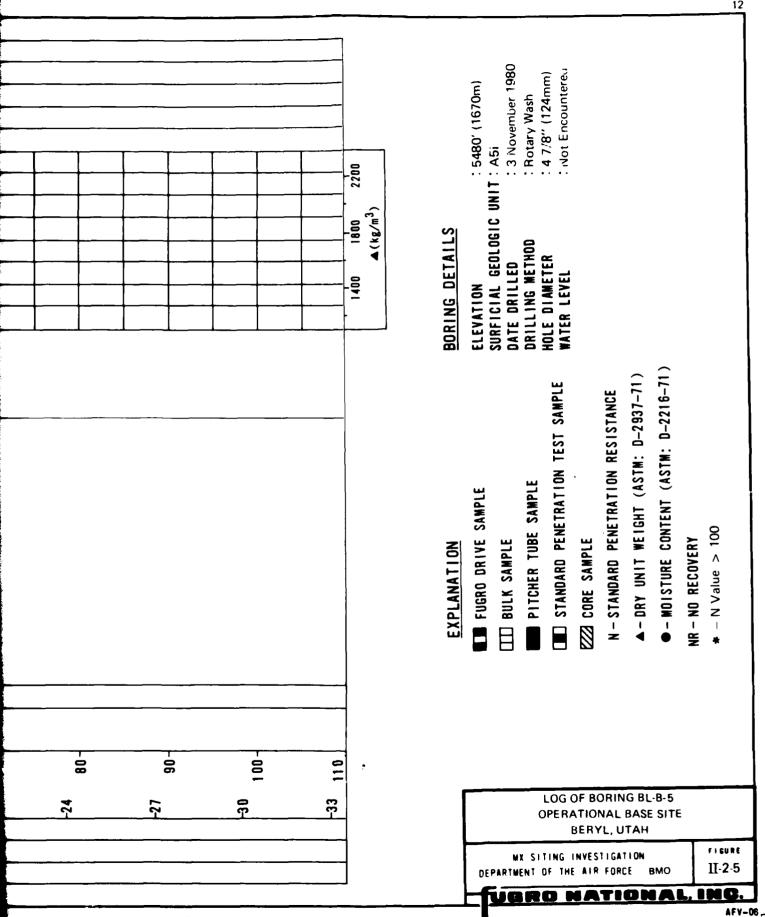
KXI/X	A9070H117	S N N N N N N N N N N N N N N N N N N N		REMARKS	8 4	8+2	15 2	20 25 0 (%)	(%)  (pcf)  (pcf	35	GR AN	SIEVE ANALYSIS GR SA FI		11 PI
1///	<i>"</i>	S OS S S S S S S S S S S S S S S S S S	calcareous; little to some fine to coarse gravel; trace to some nonplastic silt.  SILTY SAND (SM): (25.0' · 34.0'): light brown to dark brown, fine to coarse, poorly graded, dense to very dense, subangular to subrounded, calcareous; little to some nonplastic silt; none (3.0' · 5.5').		-			4			<del></del>	22	87	Z
	≥ S	SM fi S	SANDY SILT (ML): light brown to dark brown, firm to hard, nonplastic, calcareous; some fine to medium subangular to subrounded sand.				*				00 0		68	22 2
	ı										> ° <u>°</u>	29	25	ž Ž
	S	SW	, -				4	• •			73 3	50 52	19	
			30.0 (13.21)				_				<del></del>		<del></del>	



	COR Sta Dry	¥ *
--	-------------------	-----

	<b>-</b>	SANDY GRAVEL, brown to dark brown, fine to coarse, well to poorly graded, very dense, angular to subrounded, calcareous; trace to some fine to coarse sand; trace nonto signify.																			
			·											<del>,</del> _	<del></del> _						
r		= }																			_
	3 K	=																			_
	NALYSI	<u>≥</u>																		<del></del> -	
-	<u>₹</u>   8	<u>=</u>		<del>-</del> -			-	-	1			1			<del></del>	1		-1		т	_
	3+8	-		+			+		┽~	-+		╁	$\dashv$		┼-			$\dashv$		+	
	음 - 유	: <b> </b> -		+	-		+		+-	$\dashv$		+-	+		+			$\dashv$		┼	_
	52+2	( ) F		+			+		+-	-		-	$\dashv$		+-	-		┪		┼	_
	▲(pcr) 110 12( 20 25			+	-		+		+	$\dashv$		╁╌	$\dashv$		┼	-		$\dashv$		<del> </del>	_
1		. I		+-			+		+-	_		†	+		<del>                                     </del>	_		+		+	_
	,	: F		•			$\dashv$		+	$\dashv$		+-	+		+			$\dashv$		+	-
	8+5	\ \		1			+		1			+	$\neg \dagger$		+		L	$\dashv$		+	-
	REMARKS	-								cobbles	throughout		-					-			_
	SOIL DESCRIPTION			dense, angular to subrounded, calcareous; trace to some fine to coarse sand; trace non-to-slightly plastic silt; silty cand (0.01, 3.07).												<del></del>			TOTAL DEPTH 50.0" (15.2m)		
	nzcz	$\Box$	NS.		G G M						GP					GP.	₩ S				
190	רובאסרנ							0.			,				0.0						
DEPTH					-	_			20-			30	- <b>-</b>	•	-04			- 20 -	<del>_</del>		
		M C	<b>-</b>		<del>_</del>	, 			ه ـــــــ			D 		-2	<u>-</u>			Į.			٥
	IJAY M	Ì							_												_
		<del></del>	)		)																
/ERY	% BECOM		9	90	100		+ -		т	<del>-    </del>		11	_ H				<del>, _</del>	<del>-     </del>			_





GP.

70

SS

8

ė

100

SM

9

8

S SP

8

SM

ġ

8

8

8

8

dj ••• GP.

SP SP

50

8

B

_			_
20	MA	R	81

nzcz

LITHOLOGY

N AYFNE % BECOAEBA

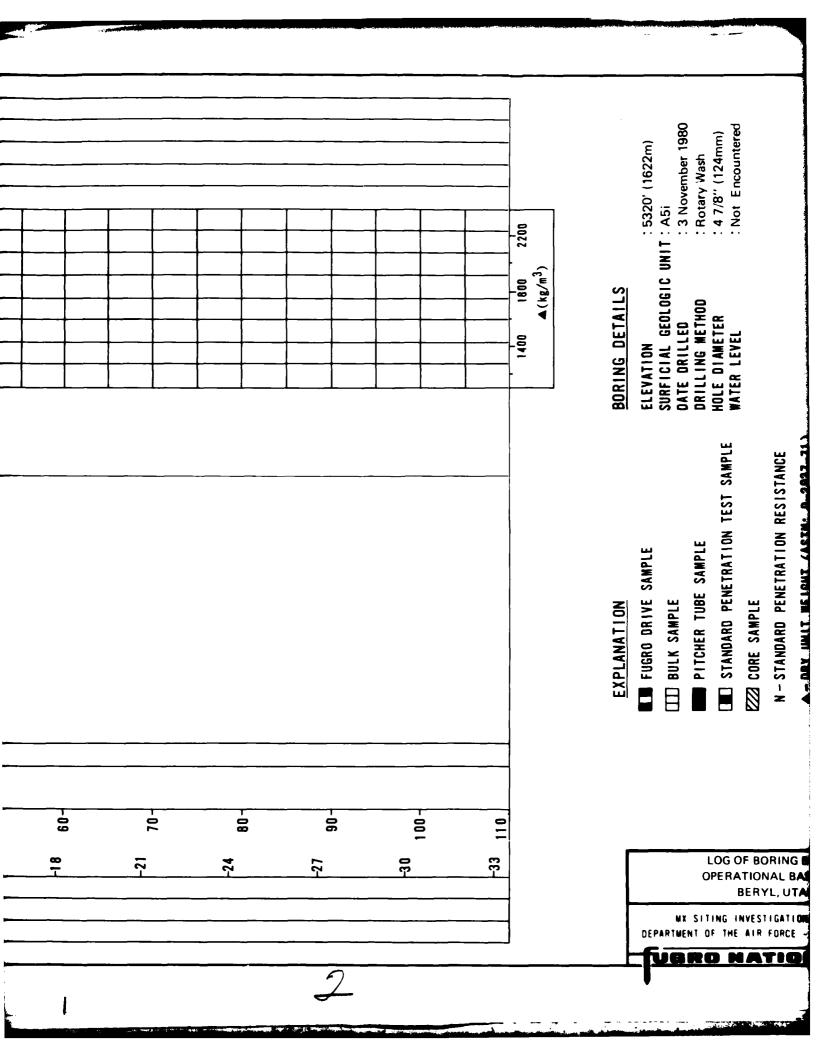
DEPTH

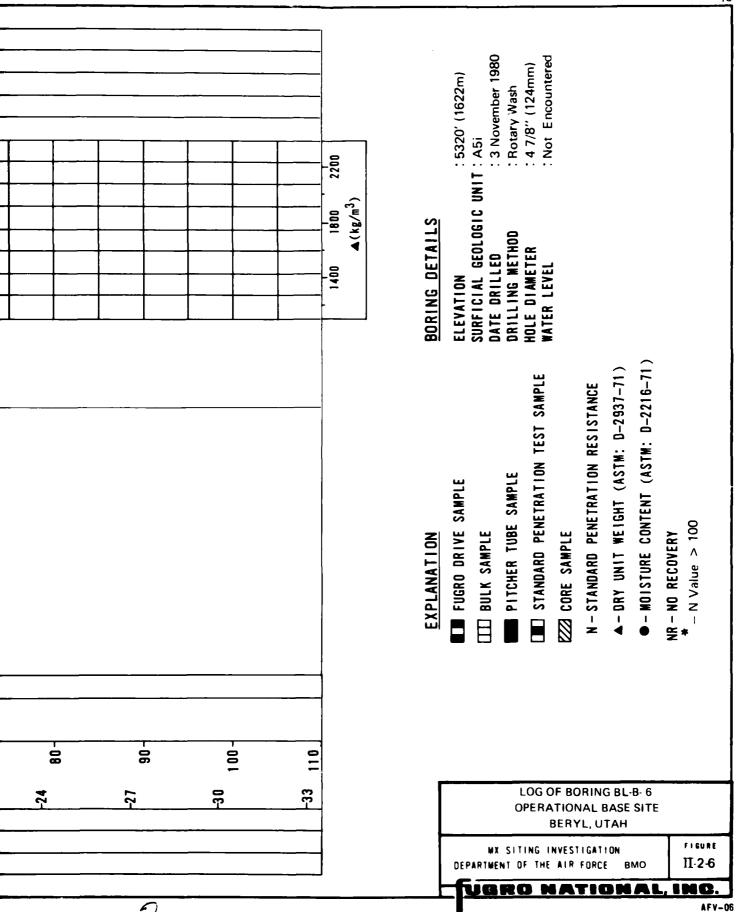
FEET

METERS

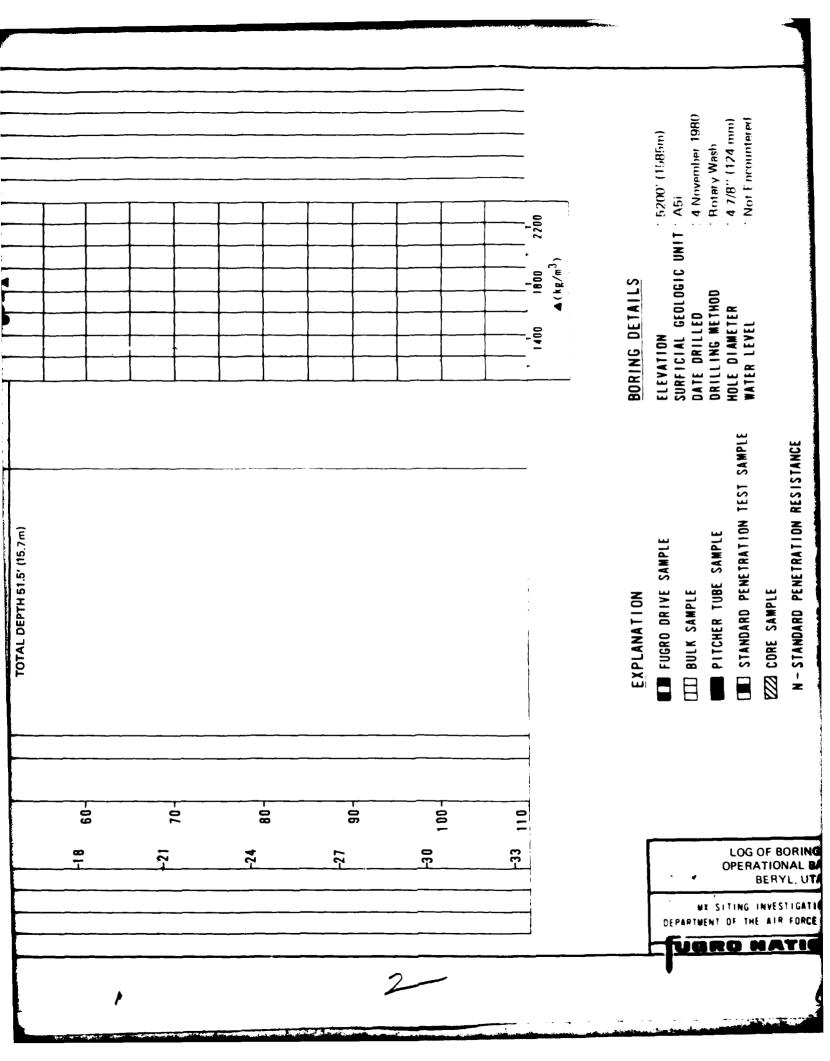
SAMPLE

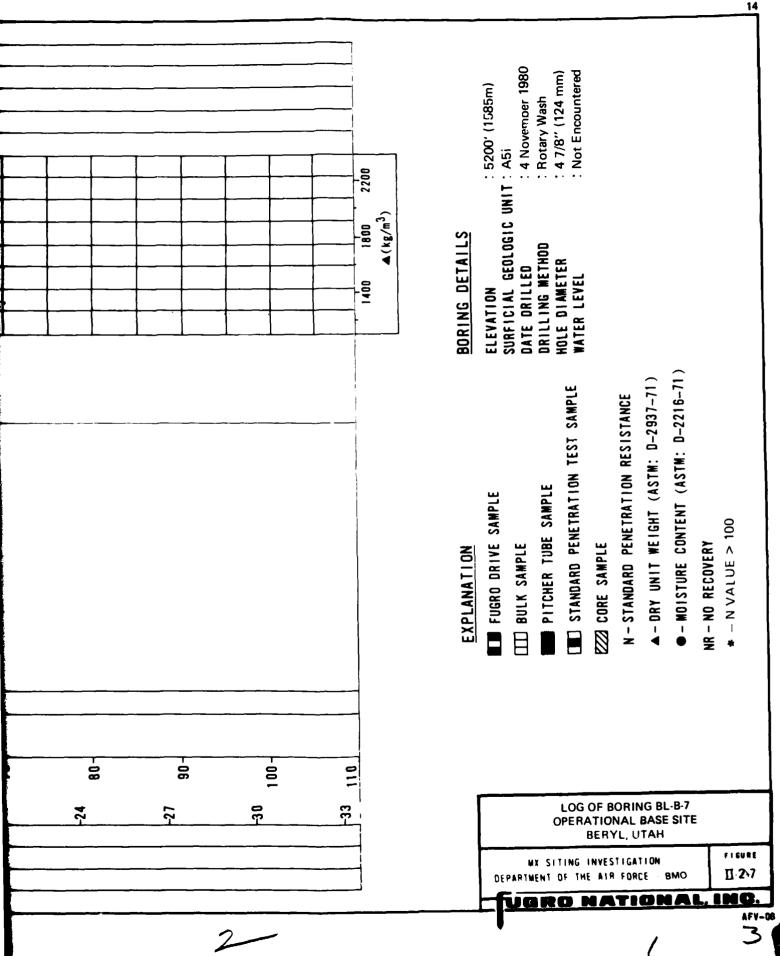
8



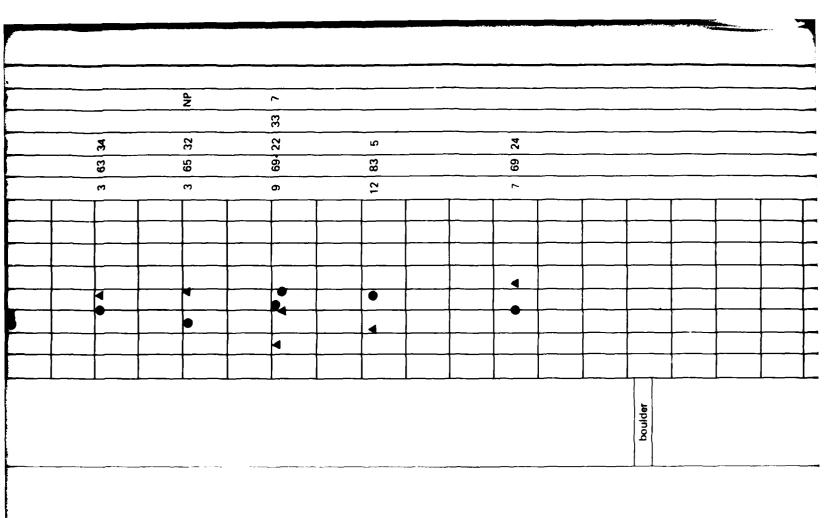


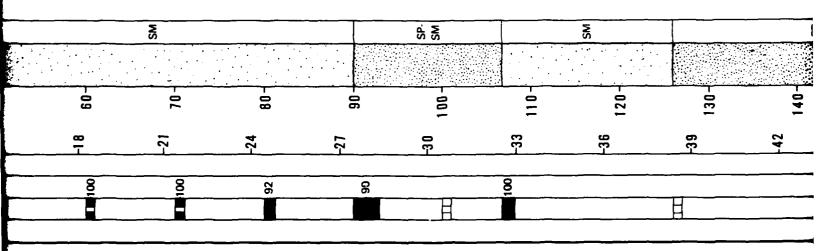
	<u>-</u>		Š	Z				<u>a</u>	<u> </u>			Ž	
S	11			-01		12		- 61	54 29			49	<del></del>
SIEVE	SA FI			49				1 17	44 5			50 4	
SI	GR	<u> </u>		- 4		- 88		4	~			-	
	35								$\prod$				
▲(pcf) 100 110 120 130 140	30 3			1					<del> </del>		<u> </u>		
f)	Lr.	ļ		$\bot$			<b>}</b>	<del> </del>	<del> </del>		<b>.</b>	_	
011 110	20 2 ● (%)		-	-	•	4	-	<b>A</b>	•4	-	•	1	
99-	12		-				-	3		-	<b>+</b>		
06 -	=	9	<b>~</b> •	+			-		<b>~</b>	-	┼		
08	-22	<del>-</del>		+	•		<del> </del>	+	+	<del></del>	+	_	
			L	1		_ <u></u>	<del></del>			<del>_</del>	<u> </u>		
REMARKS				meso Apaes	April Anues								
SOIL DESCRIPTION		Interbedded layers of SAND and FINES: SAND: GRAVELLY SAND (SP - SM): dark brown to		plastic silt.	SILTY SAND (SM): brown, fine to coarse, poorly graded, loose to very dense, subangular to subrounded, calcareous, little to some nonto slightly plastic silt.	FINES: SANDY CLAY (CL): light brown, stiff, slight- ly plastic, calcareous: some fine to medium subangular to subrounded sand.							TOTAL DEPTH 51.5′ (15.7m)
nzcz			<b>S</b> S		<u>}</u>	SP.		SM	CL		∑ S		
THOLOGY	• • •							<del>'1</del>		<u> </u>			
	F333	0				20-		30		40		50	
EPTH	r337												
SR:	373M 7337	0		- 3 10-		- 6 20-		8 F		-12 40	<u>'</u>	15 50	
EPTH	N 373M 7337	<b>.</b>	901		<u>8</u>				100		<u>,</u>	S S	

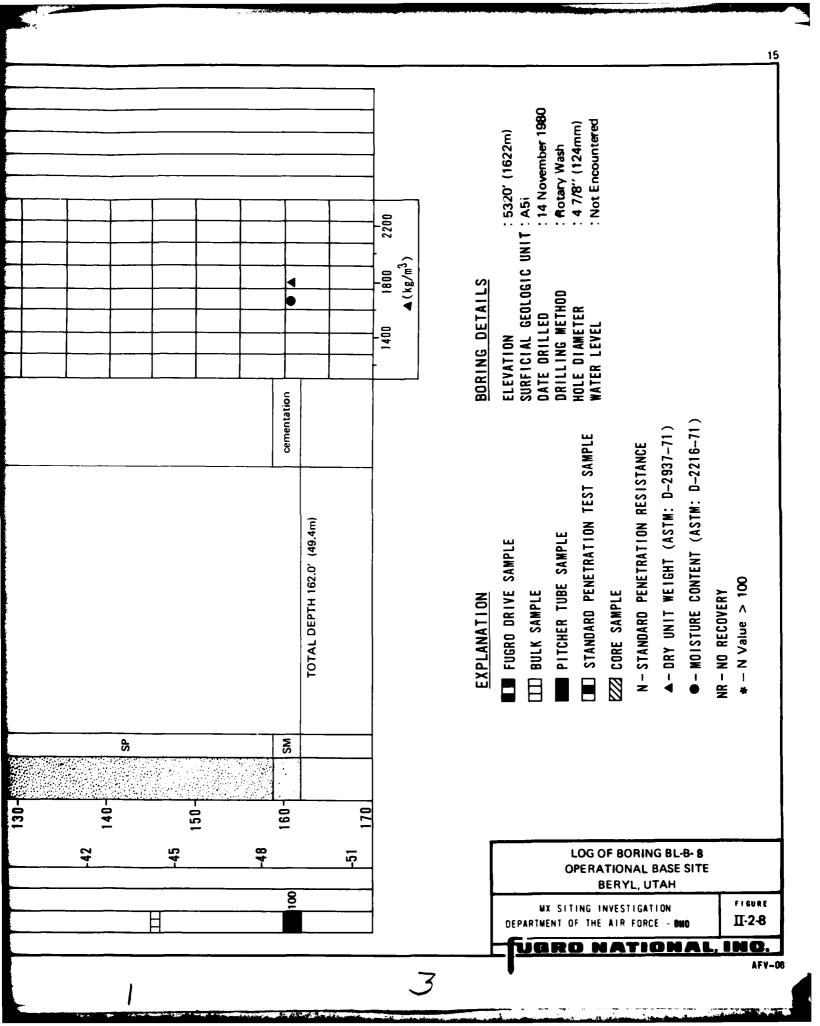




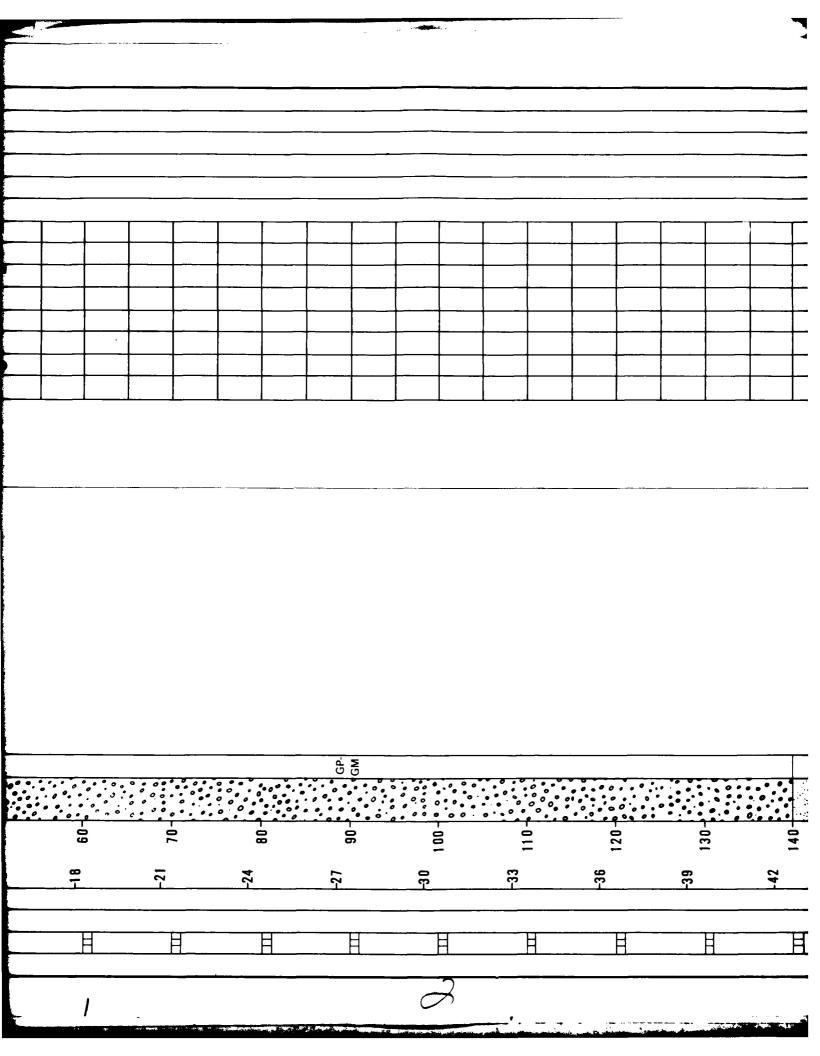
	}	LL PI						<u>Z</u>		Z						Ž	
İ	E S	FI		42		22		<u>স</u>	13	29	=			2	28	29	
	SIEVE ANALYSIS	SA		49		51		8	8	88	20			42	71	57	
i	ANA	GR		6		27		<b>∞</b>	7	8	39			53	-	4	
	01	35															
	7 0	30 3							Ţ								
	013	L I												4			
	▲(pcf) 110 120	20 25 ● (%)	4		4						4			1	_1		
	▲(pcf) 100 110 120 130 140	15 2				4		4		4			▼				
ļ			•	•					•								8
	06 0	10						•									
	80	- ro															
	REMARKS			<u>e</u>	cementation					cementation							
	SOIL DESCRIPTION		Alternating layers of GRAVELLY SAND and SILTY SAND:		to coarse, poorly graded, dense to very dense, subangular to subrounded; some fine to coarse gravel; trace to some non- to climbily plastic silt:		SILTY SAND (SM) (0.0' - 8.0', 15.0' - 30.0', 35.0' - 40.0', 45.0' - 90.0', 107.0' - 126.0'	and 159.0' - 162.0'): brown, fine to coarse, poorly graded, dense to very dense, subangular to subsunded, calcarative little to come non									
	nzcz		Ž	ž O		SM			3	,	9	SM	SM	g g	5		
49	101041	.17														-	
	1	FEE	0		•	2			20-	-	30-			40-		5.	3
<b>EPTH</b>		,	<b>I</b> —		~	•			9		6			7 -		15	
DEPTH	ERS	METI	0														
	ERS VALUI	N	100	100		8		3	<u>8</u>	8	8		<u> </u>	8		~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	

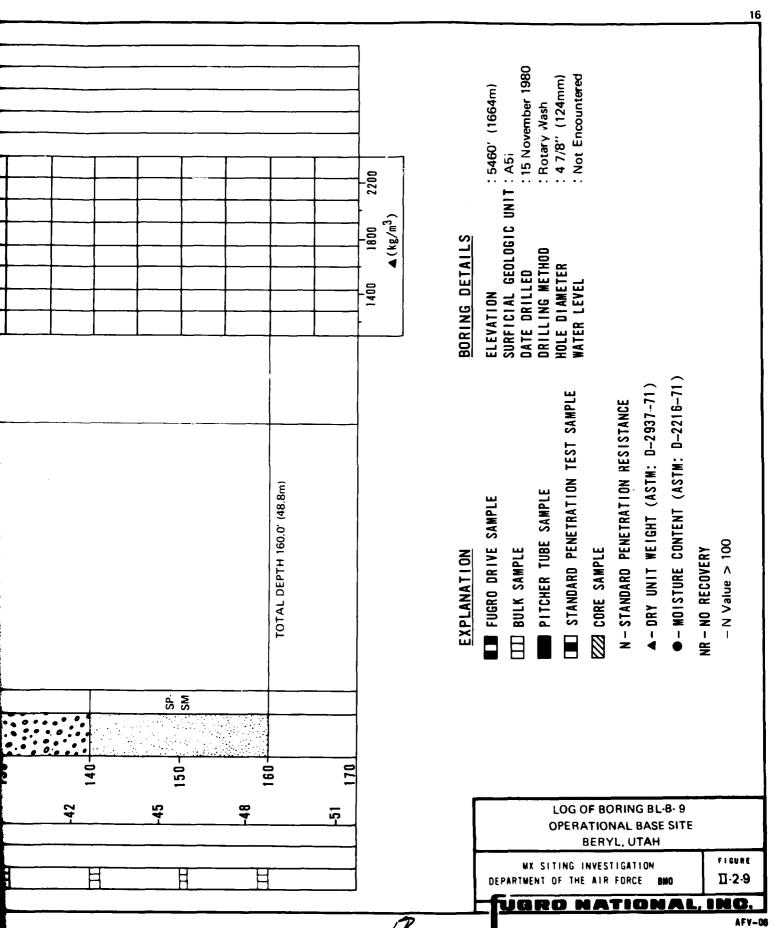




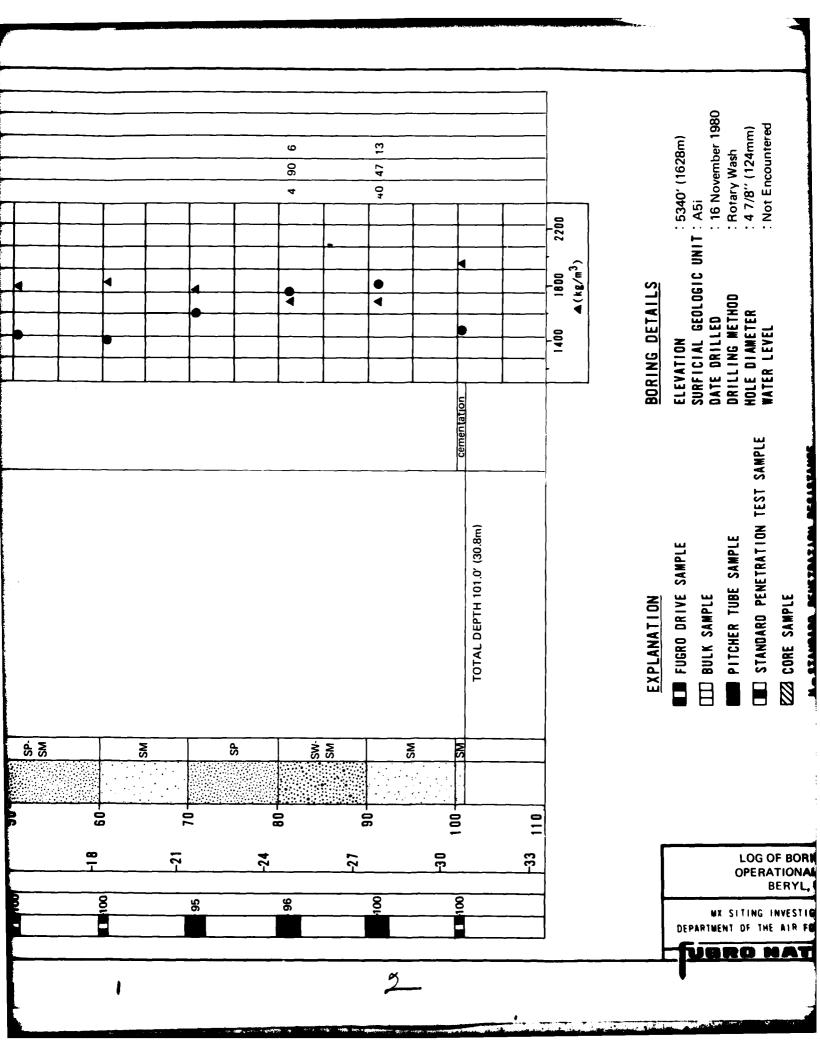


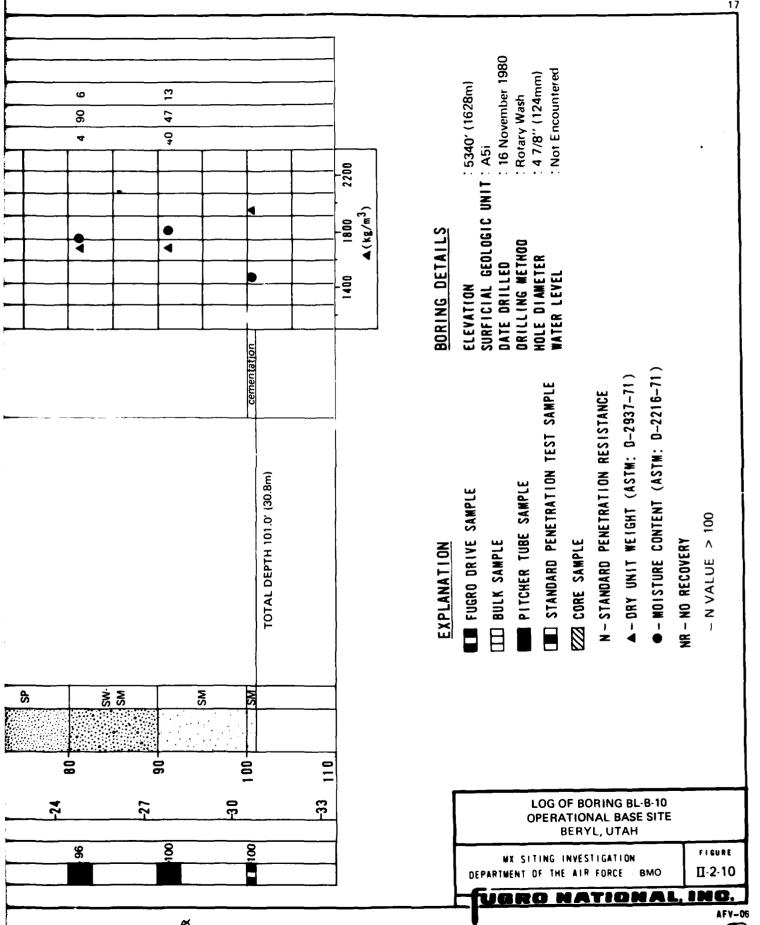
	<u>a</u>												
Γ.	- LL	 			<u> </u>							·	
YE.	ANALYSIS GR SA FI	7											
SIEVE	NALY S SA	4		8					28				
	GR AN	49	<del>                                     </del>	3	<del>-                                    </del>		т	1	-8		<del></del>	<del></del>	. 1
5	35 - 25	<b> </b>		ļ	<b>├</b> ─		<del> </del>	+-			+		-
<b>▲</b> (pcf)	로 - - -	<u> </u>		ļ -	<del>  </del>		<del> </del>	+-	+-	+-	-	-	_
£ 5	<u> </u>	ļ		ļ				+-	4	-		+-	4
<b>▲</b> (pcf)	20 25 • (%)				•		<b>-</b>	<b>-</b>		<del></del>			
=	= - - -	ļ	4	ļ.—.—			<del> </del>	+-	<del></del>	<del></del>		-	
6		ļ		L			<del> </del>	<del> </del>		<b>-</b>	<b></b> .		
- E		•	ļ				<b>•</b>	4	┸—	<u></u>	<del>- </del>	•	
Ш,		<u></u>	<u> </u>	ļ		<del></del>	<u> </u>				Ц		
REMARKS		L and	wn to cementation ose to	<u> </u>	ll to						<del></del>		
NOTION DESCRIPTION	- -	Alternating layers of SANDY GRAVEGRAVELLY SAND:	SANDY GRAVEL (GP-GM): light bronorown, fine to coarse, poorly graded, lowery dense, subangular to subrounded,	calcareous; some fine to coarse sand; tra slightly plastic silt. GRAVELLY SAND (SP-SM, SW-SM):	ight brown to brown, fine to coarse, we coorly graded, subangular to subrounded calerous; some fine to coarse gravel; tr	ionpidsuc sitt.							
=			GP. SANDY GRAVEL (GP-GM): light brown to brown, fine to coarse, poorly graded, loose to very dense, subangular to subrounded,	calcareous slightly pl	SM light brown to brown, fine to coarse, well to poorly graded, subangular to subrounded, calcareous, some fine to coarse gravel, trace	TOTAL STILL	GP.	0 00	W.S.			•* ••	
502	SU					TOO DISPLAYED STILL	WD		WS.				
S02 0000	20		W O			Dispirition .	WG .	30°-00°-00°-00°-00°-00°-00°-00°-00°-00°-	-NS	0 4		50	
\$33 \$3010 \$9010	SU		W O	, • •	MS SW			000000 F				15	
SOS OF OCA PERSON	METERS FEET LITHI US	0	W O		ws sw			ສາ L		-12 40			
OF OCK	METERS FEET LITHI US	0	W O	, • •	SW					04			



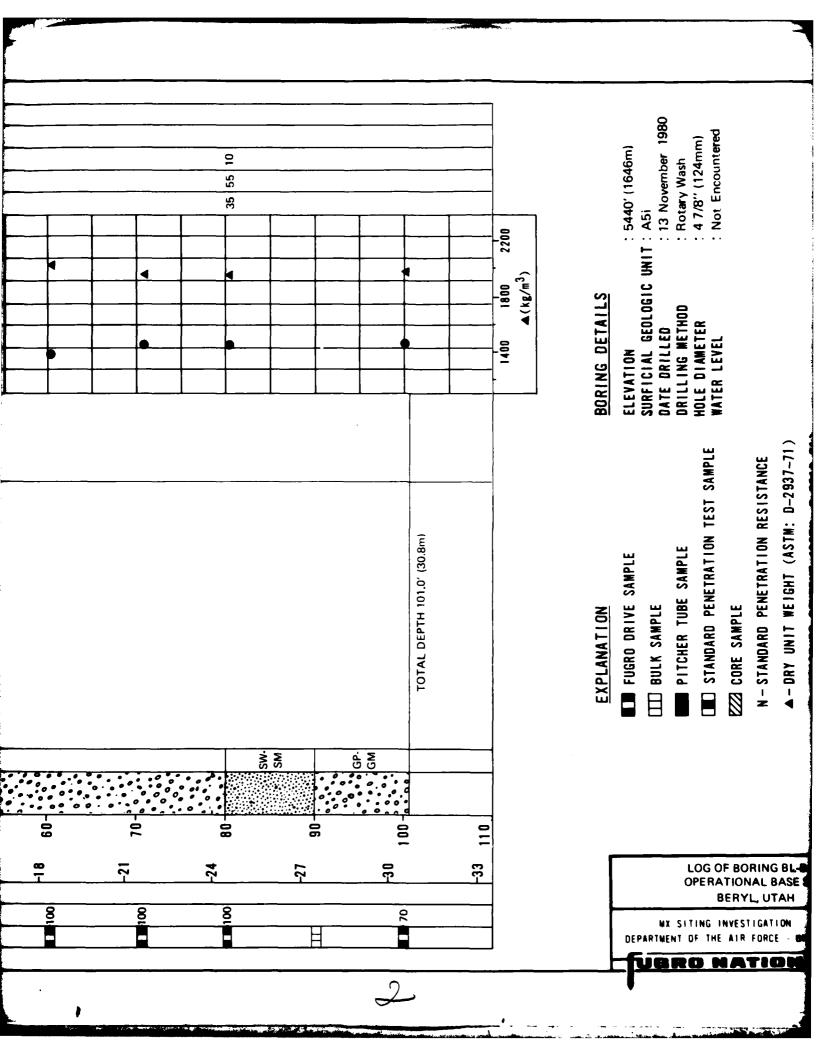


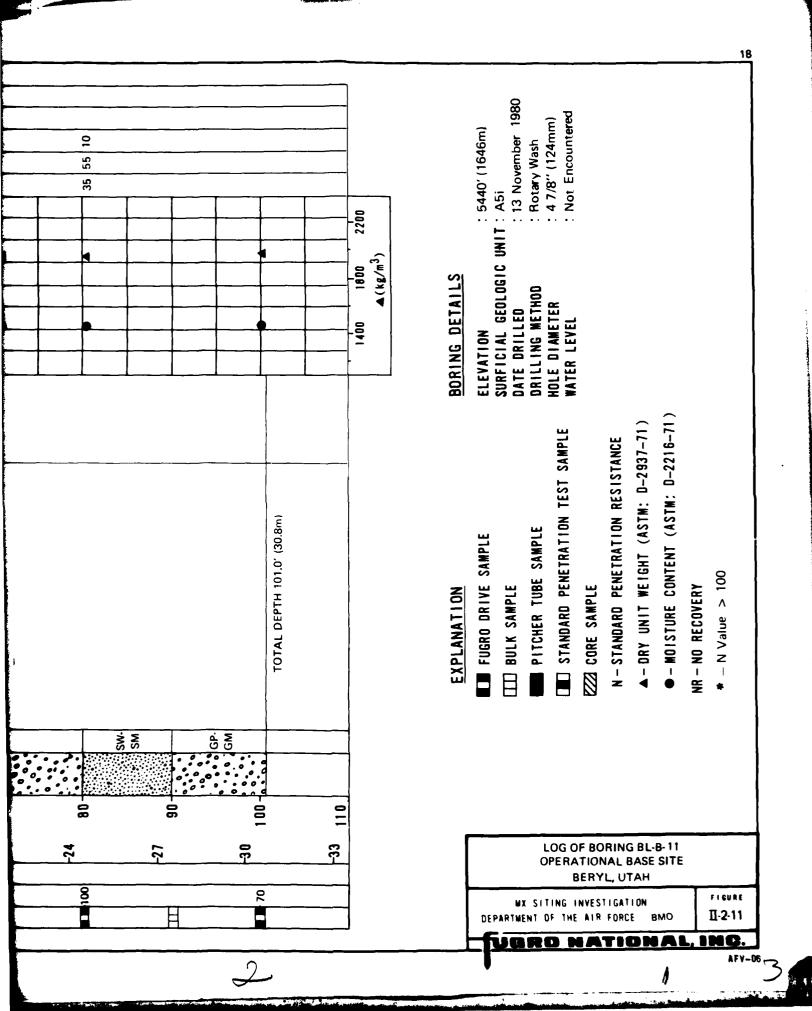
ļ	٦				Ž										
6	I LL											_			
SIEVE	SA FI		56   25 67   29		63 10						66 13				
N S	GR S.		<u>2</u> 4		27 6						21 6			<del></del> -	
		•			<u>~</u>			Ţ	Ţ	<del>,</del>	<del></del>	<u> </u>	<del></del>	<del></del>	<del></del>
1 2	35		+		-	+		<u> </u>	SSP. SSP. 1000 - 9 30 - SSP. SSP. SSP. SSP. SSP. SSP. SSP. SS	<del>- </del>	<b></b>		<del>                                     </del>	<del> </del>	
=	30		+						<b>†</b>		+	<u> </u>	<u> </u>	<b>†</b>	<b>†</b>
(£ 22 )	20 25 <b>0</b> (%)		1		4			<b>■</b>		4	4	1	4		
▲(pcf) 100 110 120 130 140	•••	4	1 9	<b>b</b>		7			4	1	1				
01 06	0 15			•					•	•					
08	10														
	- rc														
REMARKS	-	put	<u></u>			د ۵	e to carse		·						
SOIL DESCRIPTION		Interbedded layers of GRAVELLY SAND and SILTY SAND:	GRAVELLY SAND (SP,SP-SM,SW-SM,SM)	light brown, tine to coarse, well to poorly graded, dense to very dense, subangular to subrounded: trace to some fine to coarse preveil.	state to little non- to slightly plastic silt.	SM SILTY SAND (SM) (0.0* - 10.0*, 40.0* - 45.0* and 100.0* - 101.0*); brown to dark brown, fine to coarse, poorly graded, loose to dense, subangular to subrounded, calcareous; little to some nonplastic silt; none to little fine to coarse gravel.  SP SP SSP SSP SSP SSP SSP SSP SSP SSP									
		Interbedo SILTY S	GRAVELL	lignt brown graded, den	trace to littl	SILTY SAN and 100.0'	fine to coars subangular t some nonplas	gravel.							
nzcz		Interbedo	SM GRAVELL	graded, den	SW. trace to littl					ds.	SM	NS.		SP.	
THOLOGY		Interbedo		light brown graded, den						ბა	WS	WS		SS	
THOLOGY SSEC	.33J	0			-NS		SP	dS		L 05		<b>2</b>		-00	
ERS THOLOGY USCS	FEE'				-NS		SP	ds WS		L 05		<b>2</b>	<u>.</u>	-00	
THOLOGY SSEC	NET!	0	WS		-NS	NS.	SP	ds WS			-12	<b>2</b>		-00	





AFV-06





# 3.0 EXPLANATION OF TRENCH LOGS

See Section 2.0, "Explanation of Boring, Trench, and Test Pit Logs", for explanations.

BULK SAMPLE	METERS F	PTH	LITHOLOGY	nscs	CONSISTENCY	SOIL DESCRIPTION	REMARKS		S I E		- 1		
=	1		=		800			G	ı S	A]	回	LL	
	0	2 -		sc	dense	CLAYEY SAND, brewn, fine to coarse, poorly graded, slightly moiet, subengular to sub-rounded; some medium plastic day; little fine to coarse gravel.		18	3 4	1	41		
	- 1       	4 -				GRAVELLY SAND, light brown, fine to coarse, poorly greded, dry, subangular to sub-rounded, calcarsous; some gravel; trace non-plastic silt; stage III calishe (3.0'-6.0'); stage III calishe (6.0'-14.0').							
	- 2	8					vertical wet	ds					
	-3	10-		SP. SM	very dense						]		
		12-											
	-4	14-									}		
 	! !	18~				TOTAL DEPTH 14.0' (4.3m)							
1	- 5	16~											
		18-											
j	[ °	20-											

### TRENCH DETAILS

SURFACE ELEVATION : 5520' (1682m) DATE EXCAVATED : 28 OCTOBER 1980

SURFICIAL GEOLOGIC UNIT: A5 TRENCH LENGTH : 14.0' (4.3m)

: E-W

TRENCH ORIENTATION

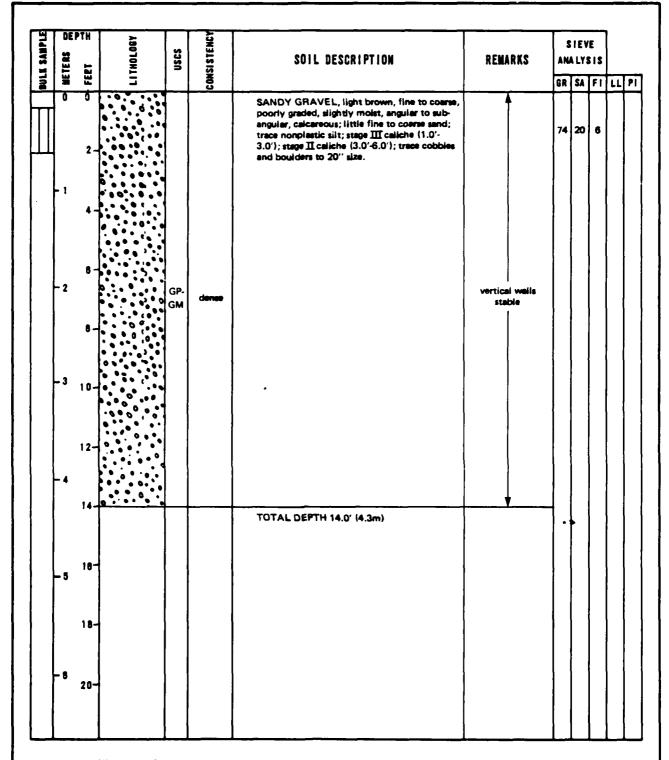
LOG OF TRENCH BL-T- 1 **OPERATIONAL BASE SITE** BERYL, UTAH

MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - BOO FIGURE II-3-1

JORO NATIONAL INC.

USAF-37

20 MAR 81



## TRENCH BETAILS

SURFACE LLEVATION

: 5880' (1792m)

DATE EXCAVATED

: 29.OCTOBER 1980

SURFICIAL GEOLOGIC UNIT: A50

TRENCH LENGTH

: 14.0' (4.3m)

TRENCH ORIENTATION

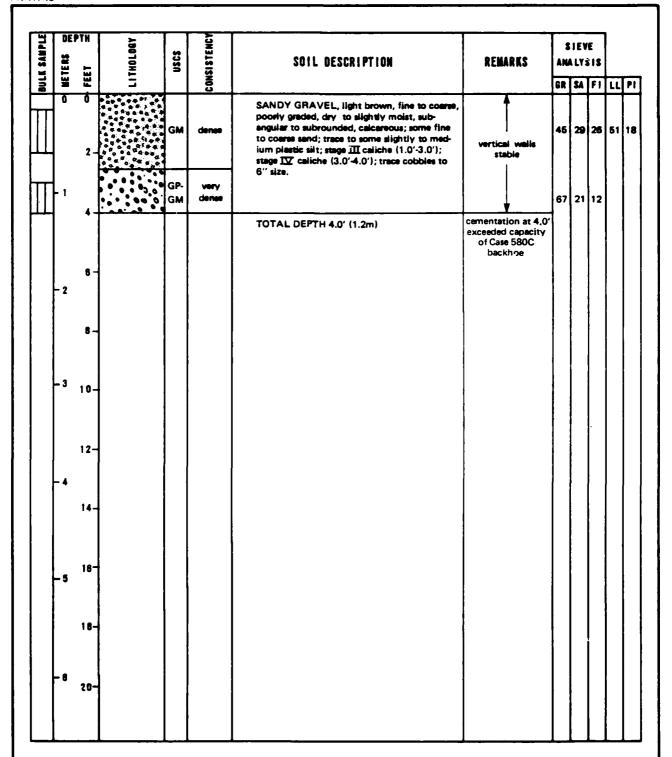
: E-W

LOG OF TRENCH BL-T- 2 **OPERATIONAL BASE SITE** BERYL, UTAH

MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - BMG F | BURE Ⅱ-3-2

20 MAR 81

USAF-37



### TRENCH DETAILS

SURFACE ELEVATION

: 5680' (1731m)

DATE EXCAVATED

TRENCH LENGTH

: 29 OCTOBER 1980

SURFICIAL GEOLOGIC UNIT: A50

: 10.0' (3.0m)

TRENCH DRIENTATION

: E-W

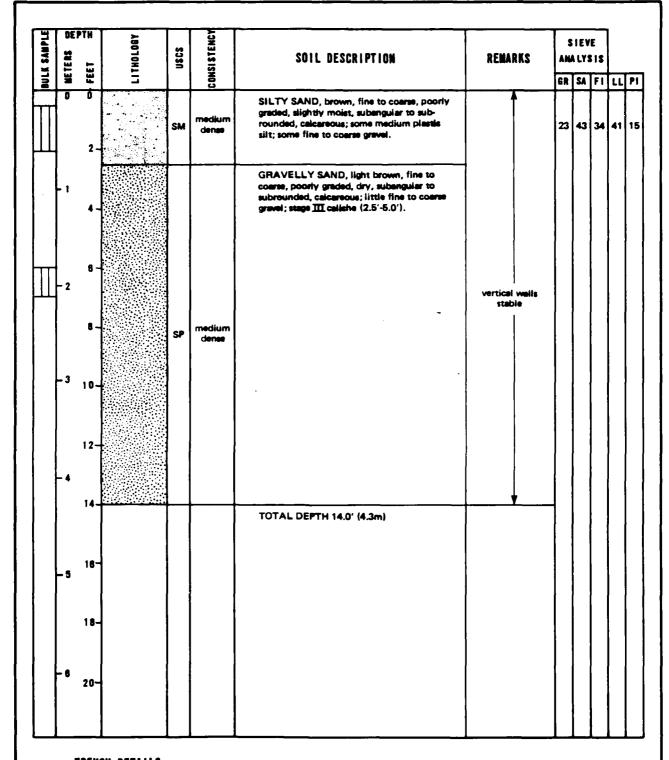
LOG OF TRENCH BL-T-3 **OPERATIONAL BASE SITE** BERYL, UTAH

MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - BMG FIGURE **II-3-3** 

UGRO NATIONAL

20 MAR 81

USAF-37



SURFACE ELEVATION

: 5620' (1682m)

DATE EXCAVATED

: 29 OCTOBER 1980

SURFICIAL GEOLOGIC UNIT: ASI

TRENCH LENGTH TRENCH ORIENTATION : 14.0' (4.3m)

: N-8

**LOG OF TRENCH BL-T-4 OPERATIONAL BASE SITE** BERYL, UTAH

MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - BMG

F 1 60 FE **II-3-4** 

20 MAR 81

BULK SAMPLE	WETERS S	PTH E	LITHOLOGY	USCS	CONSISTENCY	SOIL DESCRIPTION	REMARKS	AN	IEV ALYS	15		
3		FEET	<u> </u>		8 3			GR	SA	FI	LL	P
	0	0		SM	medium dense	SILTY SAND, brown, fine to coarse, poorly graded, slightly moist, subengular to subrounded, calcareous; some medium plastic silt; some fine to soarse gravel.						
	- 1	4-			dense	GRAVELLY SAND, light brown, fine to coarse, well graded, dry, subengular to sub-rounded, calcareous; some fine gravel; stage III callehe (2.0'-5.0'); occasional cobbles and boulders (11.0'-14.0').		35	63	2		
	- 2	6 -		sw			vertical walls stable					
	<b>–</b> 3	10-	•	<b>3W</b>	medium dense							
	-4	12-										
		14-				TOTAL DEPTH 14.0' (4.3m)	<b></b>	1				
	-5	16-										
		18-										
	- 6	20-										

SURFACE ELEVATION : 5660' (1725m)
DATE EXCAVATED : 29 OCTOBER 1980

SURFICIAL GEOLOGIC UNIT: A51

TRENCH LENGTH : 14.0' (4.3m)

TRENCH ORIENTATION : E-W

LOG OF TRENCH BL-T- 5 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMG

UBRO NATIONAL IN

BULK SAMPLE	METERS A	FEET =	LITHOLOGY	uscs	CONSISTENCY	SOIL DESCRIPTION	REMARKS	1	ALYS			
<b>1</b>	Ħ		=					GR	SA	FI	LL	PI
	- 1	2-		SM	medium dense	SILTY SAND, brown, fine to coarse, poorly graded, dry to slightly molet, subangular to subrounded, calcareous; some medium plastic silt; some fine to coarse gravel; stage II caliche (2.0'-4.0'); trace cobbles and boulders to 20" size.	vertical wells stable					
		4-				TOTAL DEPTH 4.0' (1.2m)	cementation at 4.0' exceeded capacity of Case 580C backhoe					
	- 2	8-										
	-3	10-										
		12-			,		·					
	-4	14-										
	- 5	16-										
		18-										
	- 6	20-										

SURFACE ELEVATION : 5800' (1768m)
DATE EXCAVATED : 30 OCTOBER 1980

SURFICIAL GEOLOGIC UNIT: ASI

TRENCH LENGTH : 10.0° (3.0m)

TRENCH ORIENTATION : E-W

LOG OF TRENCH BL-T-6 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - SUC

Г1 **сият** П-3-6

UBRO NATIONAL, INC.

DULK SAMPLE	BETERS R	PTH	LITHOLOGY	uscs	CONSISTENCY	SOIL DESCRIPTION	REMARKS	AN	IEV ALYS	:18	LLF
	0	2 -		SM	dense	SILTY SAND, light brown, fine to coerse, poorly graded, dry, subengular to subrounded, calcareous; some slightly plastic slit; little fine gravel; stage III caliche (1.0'-3.0').			59		
	- 1	4				GRAVELLY SAND, light brown, fine to coarse, poorly graded, dry, subangular to subrounded, calcareous; some fine to coarse gravel; stage I caliche (3.0'-10.0'); occasional cobbles to 6" size.					
	- 2	8 -		SP	medium dense		vertical walls stable				
	-3	10-									
	-4	14-				TOTAL DEPTH 14.0' (4.3m)					
	-5	16-									
	- 6	20-						i.			

SURFACE ELEVATION

: 5540' (1**689**m)

DATE EXCAVATED

: 30 OCTOBER 1980

SURFICIAL REBLOGIC UNIT: A5

TRENCH LENGTH

: 14.0' (4.3m)

TRENCH ORIENTATION

: N-S

LOG OF TRENCH BL-T-7 **OPERATIONAL BASE SITE** BERYL, UTAH

WX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - BUG FIGURE II-3-7

USAF-37

CLAYEY SAND, light brown, fine to coarse, poonly graded, dry, subengular to subrounded, calcareaus, some medium plastic city; stage  SILTY SAND, light brown, fine to coarse, poonly graded, dry, subengular to subrounded, calcareaus; little nonplastic sit; trace gravel.  SM dense  SW medium  SW medium  SW medium plastic, calcareaus; little nonplastic sit; trace gravel.  CLAYEY SILT, light gray-brown, dry, modium plastic, calcareaus; stage IV cellche (10.0-11.0).  TOTAL DEPTH 11.0: (3.4m)  cementation at 11.0 exceeded capacity of Case 580C backhos	BULK SAMPLE	WETERS 39	PTH :	LITHOLOGY	nscs	CONSI STENCY	SOIL DESCRIPTION	REMAI	RKS		IEV			
SC dense  SILTY SAND, light brown, fine to coarse, poorly graded, dry, subsequier to abbrounded, calcaveous; some medium plastic clay; stage  SILTY SAND, light brown, fine to coarse, poorly graded, dry, subsequier to subsrounded, calcaveous; little nonplastic silt; trace graved.  SM dense  SM dense  GRAVELLY SAND, light brown, fine to coarse, poorly graded, dry, subsequier to subsrounded, calcaveous; little nonplastic silt; trace graved.  CLAYEY SAND, light brown, fine to coarse, well graded, dry, subsequier to subsrounded, calcaveous; little fine graved.  CLAYEY SAND, light brown, fine to coarse, well graded, dry, subsequier to subsrounded, calcaveous; little fine graved.  CLAYEY SAND, light brown, fine to coarse, poorly graded, dry, subsequier to subsrounded, calcaveous; little fine graved.  CLAYEY SAND, light brown, fine to coarse, poorly graded, dry, subsequier to subsrounded, calcaveous; little fine graved.  CLAYEY SAND, light brown, fine to coarse, poorly graded, dry, subsequier to subsrounded, calcaveous; little fine to coarse, well graded, dry, subsequier to subsrounded, calcaveous; little fine to coarse, well graded, dry, subsequier to subsrounded, calcaveous; little fine to coarse, poorly graded, dry, subsequier to subsrounded, calcaveous; little fine to coarse, well graded, dry, subsequier to subsrounded, calcaveous; little fine to coarse, poorly graded, dry, subsequier to subsrounded, calcaveous; little fine to coarse, poorly graded, dry, subsequier to subsrounded, calcaveous; little fine to coarse, poorly graded, dry, subsequier to subsrounded, calcaveous; little fine to coarse, poorly graded, dry, subsequier to subsrounded, calcaveous; little fine to coarse, poorly graded, dry, subsequier to subsrounded, calcaveous; little fine to coarse, poorly graded, dry, subsequier to subsrounded, calcaveous; little fine to coarse, poorly graded, dry, subsequier to subsrounded, calcaveous; little fine to coarse, poorly graded, dry, subsequier to subsrounded, calcaveous; little fine to coarse, poorly graded, dr	3	<b>=</b>	<u> </u>	=		800				GR	SA	FI	LL	P
SM dense  SILTY SAND, light brown, fine to coarse, poorly graded, dry, subangular to subrounded, calcareous; little nonplastic silt; trace gravel.  GRAVELLY SAND, dark brown, fine to coarse, well graded, dry, subangular to subrounded; little fine gravel.  CLAYEY SILT, light grav-brown, dry, medium plastic, calcareous; stage IV caliche (10.0·11.0·).  TOTAL DEPTH 11.0·(3.4m)  Cementation at 11.0 exceeded capacity of Case SBOC backhoe  18-  18-  18-			2 -		sc	dense	poorly graded, dry, subangular to subrounded, calcareous; some medium plastic clay; stage			3	58	39	45	11
SM dense  GRAVELLY SAND, dark brown, fine to coarse, well greated, dry, subangular to subrounded; little fine gravel.  CLAYEY SILT, light gray-brown, dry, medium plastle, calcarreous; stags IV caliche (10.0'-11.0').  TOTAL DEPTH 11.0' (3.4m)  12-  -4  14-  18-  18-	11	- 2	6 -				poorly graded, dry, subangular to subrounded,							
Syv medium dense coarse, well greded, dry, subangular to sub-rounded; little fine gravel.  CLAYEY SILT, light gray-brown, dry, medium plastic, calesreoue; stage IV caliche (10.0'-11.0').  TOTAL DEPTH 11.0' (3.4m)  12-  -4  14-  18-  -5  18-	$\prod$		8 -		SM	dense						!		
MH   hard	П	-3			SW		coerse, well greded, dry, subengular to sub- rounded; little fine gravel.			18	79	3		
124 14- 185	$\prod$		10-		мн	hard	medium plastic, calcareous; stage IV caliche		,			:	63	2
18-		-•	12-				TOTAL DEPTH 11.0' (3.4m)	exceeded of Case	capacity 580C					
18-			14-											
	İ	- 5												
		-8	18- 20-											

SURFACE ELEVATION : 5410' (1640m)

DATE EXCAVATED : 31 OCTOBER 1980

SURFICIAL GEOLOGIC UNIT: A61

TRENCH LENGTH : 14.0' (4.3m)

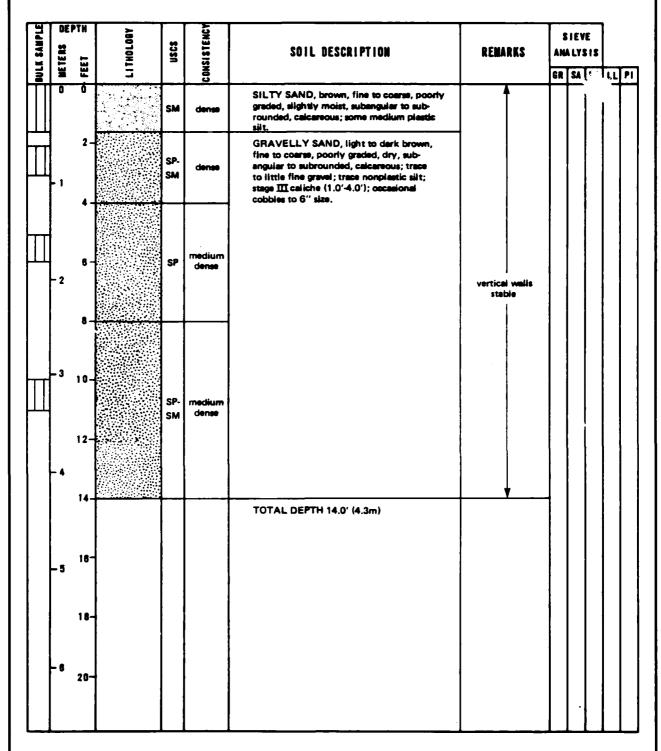
TRENCH ORIENTATION : E-W

LOG OF TRENCH BL-T- 8 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - DOOR

II-3-8

UGRO NATIONAL, INC.



SURFACE ELEVATION

: 5460' (1664m)

DATE EXCAVATED .

: 31 OCTOBER 1980

SURFICIAL GEOLOGIC UNIT: A51

NII: ADI

TRENCH LENGTH

: 14.0′ (4.3m)

TRENCH ORIENTATION : E-W

LOG OF TRENCH BL-T- 9
OPERATIONAL BASE SITE
BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - SMO

II-3-9

**UBRO NATIONAL INC** 

USAF-37

BULK SAMPLE	NETERS S	FEET =	LITHOLOGY	nscs	CONSISTENCY	SOIL DESCRIPTION	REMARKS	AN	LYS	15	LL PI
	0	0 2 -		SM	dense	SILTY SAND, dark brown, fine to coarse, poorly graded, slightly moist; subangular to subrounded, calcareous; some medium plastic silt.				38	
	-1	4-		GP- GM	dense	SANDY GRAVEL, light brown, fine to coarse, poorly graded, dry, subangular to subrounded, calcareous; some fine to coarse sand; trace nonplastic silt.	vertical walls stable				
	- 2	8				TOTAL DEPTH 6.0' (1.8m)	bedrock at 6.0' exceeded capacity of Case 580C backhoe				
	- 3	10-									
	-4	12-									
	- 5	18-									
		18-									
	F°	20-									

SURFACE ELEVATION : 5880' (1792m)

DATE EXCAVATED : 31 OCTOBER 1980

11-19日本では、中日の大学 - 1911年 - 19

SURFICIAL GEOLOGIC UNIT: I4

TRENCH LENGTH : 12.0 (3.7m)

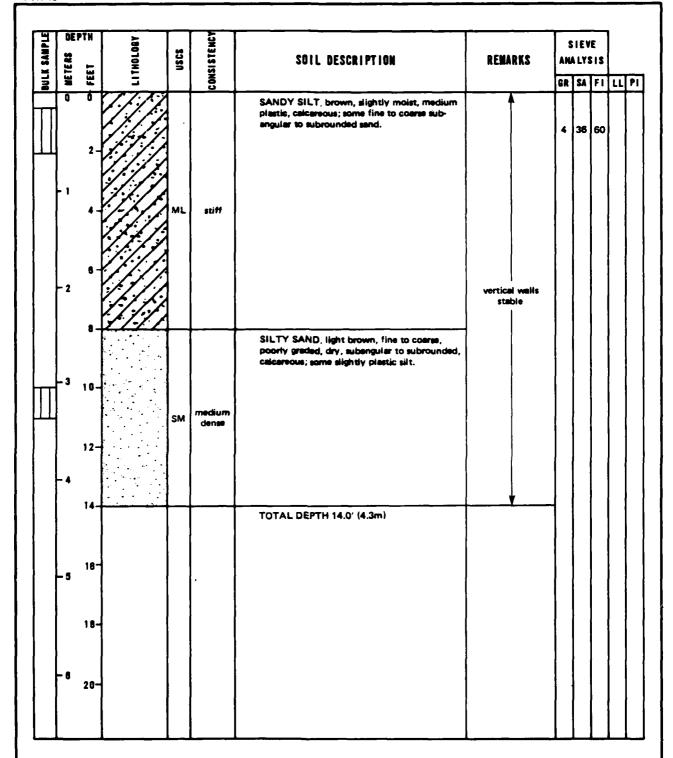
TRENCH ORIENTATION : E-V/

LOG OF TRENCH BL-T-10 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - DOOR

F1 & UTE

UGRO NATIONAL INC



SURFACE ELEVATION : 5760' (1756m) : 31 OCTOBER 1980

DATE EXCAVATED

SURFICIAL GEOLOGIC UNIT: I 4 TRENCH LENGTH : 14.0' (4.3m)

TRENCH ORIENTATION : E-W

LOG OF TRENCH BL-T-11 **OPERATIONAL BASE SITE** BERYL, UTAH

MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - BMG

FIGURE II-3-11

20 MAR 81

BULK SAMPLE	HETERS 30	PTH	LITHOLOGY	nscs	CONSISTENCY	SOIL DESCRIPTION	REMARKS	AN	IEV ALY:	\$1\$		
156		_	_ =		No.			GR	SA	FI	LL	PI
	- 1	2-			dense	SILTY SAND, light brown to brown, fine to coarse, poorly graded, dry, subengular to sub-rounded, calcareous; little to some non- to slightly plastic silt; trace to little fine gravel; stage II caliche (1.5'-12.0'); trace cobbles to 6" size (8.0'-12.0').		1	62	37		
	- 2	6 ~		SM	medium dense		vertical walls stable	15	51	34		
	-3	10-			dense							
	-4	14-				TOTAL DEPTH 12.0' (3.7m)	cementation at 12.0 exceeded capacity of Case 580C backhoe					
	-5	18-										
	- 6	18-										

SURFACE ELEVATION : 5515' (1681m)
DATE EXCAVATED : 31 OCTOBER 1980

SURFICIAL GEOLOGIC UNIT: A51

TRENCH LENGTH

: 14.0' (4.3m)

TRENCH ORIENTATION : N-S

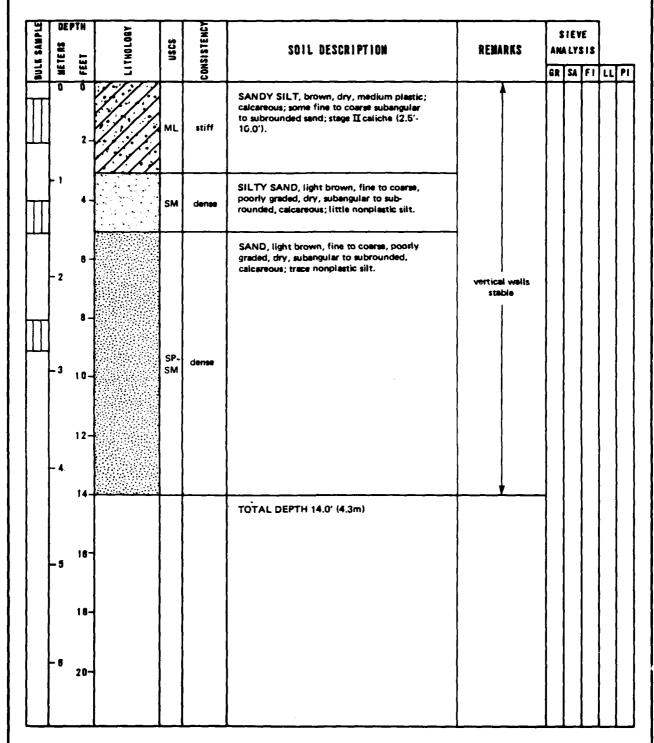
LOG OF TRENCH BL-T-12 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - 800

#160ft ∐-3-12

TUBRO NATIONAL II

USAF-37



SURFACE ELEVATION : 5420' (1652m)
DATE EXCAVATED : 31 OCTOBER 1980

SURFICIAL GEOLOGIC UNIT: A5i

TRENCH LENGTH : 14.0' (4.3m)

TRENCH ORIENTATION : E-W

LOG OF TRENCH BL-T-13 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMG

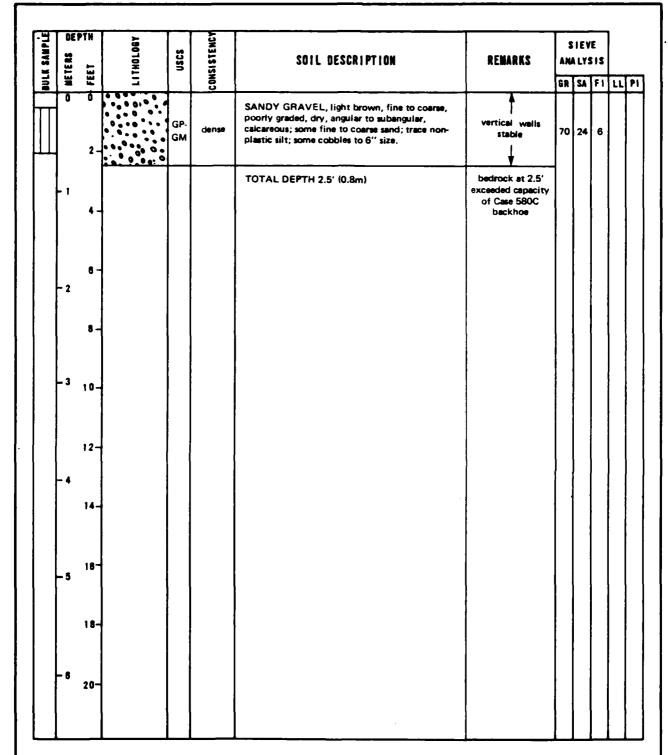
F1 6486

UGRO NATIONAL INC

USAF-37

20 MAR 81

---



SURFACE ELEVATION : 570

DATE EXCAVATED .

: 5700' (1737m) : 1 NOVEMBER 1980

SURFICIAL REOLOGIC UNIT: [4

TRENCH LENGTH : 10.0' (3.0m)

TRENCH ORIENTATION : E-W

LOG OF TRENCH BL-T-14 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMG

UBRO NATIONAL II

BULK SAMPLE	METERS 19	PTH :	LITHOLOGY	nscs	CONSISTENCY	SOIL DESCRIPTION	REMARKS		IEV ALYS			
nne		FEET	5		CON			GR	AZ	FI	LL	PI
	0	<u>0</u>		SM	dense	GRAVELLY SAND, brown to light brown, fine to coarse, poorly graded, dry, subangular, calcareous; some fine to coarse gravel; little slightly plastic silt; stage III caliche (1.0'-3.5'); stage III caliche (3.5'-4.0'); occasional cobbles to 6" size.	vertical walls stable	36	51	13		
		4 -			very dense		<del>                                     </del>					
	- 2	8 -				TOTAL DEPTH 4.0' (1.2m)	cementation at 4,0' exceeded capacity of Case 580C backhoe					
	, , ,	8 -										
	- 3	10-										
		12-					i					
									ĺ ′	ĺ		
	-4	14-						ì				
	- 5	16-										
		18-										
	- 6	20-										
						,						

SURFACE ELEVATION : 5520' (1682m) : 1 NOVEMBER 1980 DATE EXCAVATED

SURFICIAL GEOLOGIC UNIT: A51

TRENCH LENGTH

: 10.0' (3.0m) : E-W TRENCH ORIENTATION

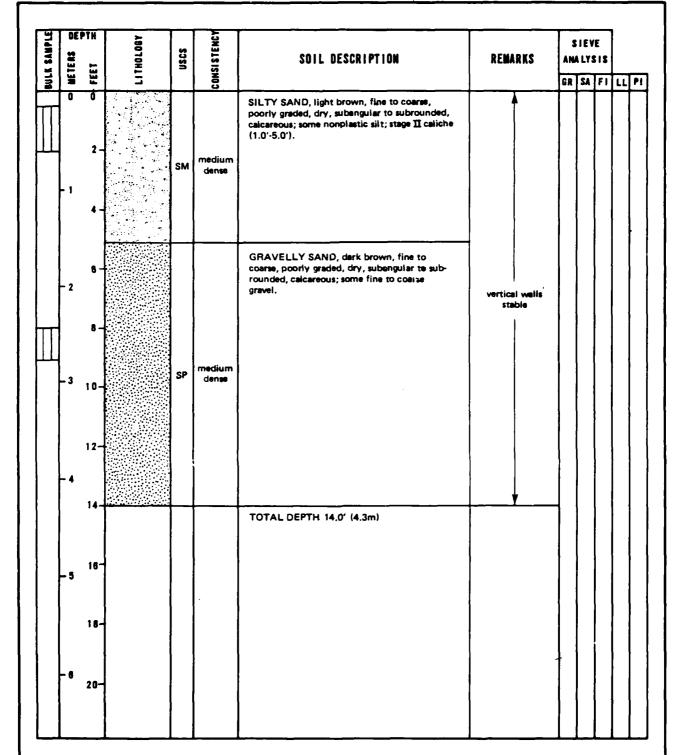
LOG OF TRENCH BL-T-15 **OPERATIONAL BASE SITE** BERYL, UTAH

MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - BMG

FIGURE **∐-3-15** 

UGRO NATIONAL INC.

20 MAR 81



SURFACE ELEVATION : 5375' (1638m)
DATE EXCAVATED : 1 NOVEMBER 1989

SURFICIAL GEOLOGIC UNIT: A5i

TRENCH LENGTH : 14.0' (4.3m)

TRENCH ORIENTATION : N-S

LOG OF TRENCH BL-T-16 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMG

F1€08€ Ⅲ-3-16

FIRED MATIONAL

BULK SAMPLE	NETERS 30	FEET H	LITHOLOGY	USCS	CONSISTENCY	SOIL DESCRIPTION	REMARKS	AN	IEV	313		Tar
	0	0 2 -			<u> </u>	SILTY CLAY, brown to dark gray, very moist to saturated, slightly plastic, calcareous.		0	3		26	
	• 1	4-		СĹ	firm		vertical wells stable weter encountered at 5.0'					
	- 2	8-										
	- •	10-				TOTAL DEPTH 10.0' (3.0m)	excavation terminated at 10,0° due to water infiltration					
	- •	12-					imitration					
		14-										
-	- 5	16-										
		18-										
	- 8	20-										

SURFACE ELEVATION : 5080' (1548m)

DATE EXCAVATES : 2 NOVEMBER 1980

SURFICIAL REGLOGIC UNIT: A1/A40 TRENCH LENGTH : 12.0 (3.7m)

TRENCH ORIENTATION : E-W

LOG OF TRENCH BL-T-17 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - 800

UGRO NATIONAL INC.

BULK SAMPLE	METERS 30	PTH	LITHOLOGY	nscs	CONSISTENCY	SOIL DESCRIPTION	REMARKS		ALYS			
		FEET	117		CON			GR	SA	FI	LL	PI
	0	2 -			firm	SILT, brown to dark olive-gray, moist to saturated, medium plastic, calcareous; some fine to medium sand; stage I caliche (1,0'-2,0'); stage III caliche (2,0'-5,0').		0	38	62	59	15
	- 2	6 -		мн	stiff		vertical walls stable water encountered at 7.5'					
	-3	8 -			firm							
٠		10-	1 - 1,1,50 + 0 - 1 - 1,00 + 0,			TOTAL DEPTH 10.0' (3.0m)	excavation terminated at 10.0	.]	i			
		12-					due to water infiltration					
	-4											
		14-										
I	- 5	16-										
		18-										
	- 6	20-										
			!									

SURFACE ELEVATION : 5090' (1551m)
DATE EXCAVATED : 2 NOVEMBER 1980

SURFICIAL GEOLOGIC UNIT: A40

TRENCH LENGTH : 12.0' (3.7m)

TRENCH ORIENTATION : E-W

LOG OF TRENCH BL-T-18 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - 800

F1 80 8E

UGRO NATIONAL IN

BULK SAMPLE	PTH E	LITHOLDBY	nscs	CONSISTENCY	SOIL DESCRIPTION	REM	IRKS		IEV			
1	2-	III	SM	medium dense	SILTY SAND, light brown, fine to medium, poorly graded, moist, subengular to subrounded, calcareous; some alightly plastic silt; stage II caliche (1.0'-3.0').			GR O		F1		PI
- 2	6-		сн	.firm	CLAY, light brown, moist, highly plastic, calcareous.		al walls ble				Ğ7	35
-4	14-				TOTAL DEPTH 14.0' (4.3m)							
-5	18-											
- 6	20-											

SURFACE ELEVATION : 5105' (1559m)
DATE EXCAVATED : 2 NOVEMBER 1980

SURFICIAL GEGLOGIC UNIT: A5y/A4o TRENCH LENGTH : 14.0' (4.3m)

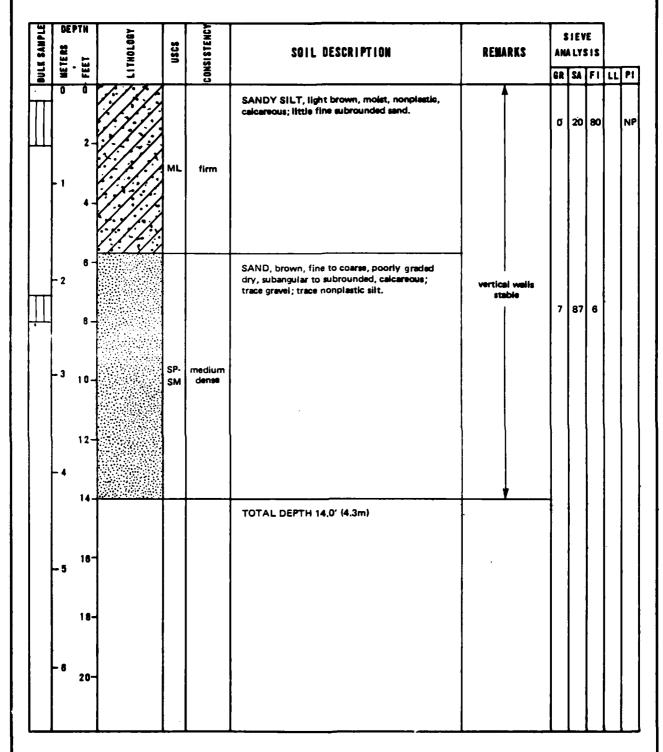
TRENCH ORIENTATION : E-W

**LOG OF TRENCH BL-T-19** OPERATIONAL BASE SITE BERYL, UTAH

WX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - BMG FIGURE

**∐-3-19** 

20 MAR 81



SURFACE ELEVATION DATE EXCAVATED

: 5140' (1567m)

: 2 NOVEMBER 1980

SURFICIAL GEOLOGIC UNIT: A5y

TRENCH LENGTH

; 14.0' (4.3m)

TRENCH ORIENTATION

: N-S

**LOG OF TRENCH BL-T-20 OPERATIONAL BASE SITE** BERYL, UTAH

MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - .

FIGURE **II-3-20** 

20 MAR 81

SILTY CLAY, brown, moist, medium plastic, calcareous; trace fine sand.  SILT, light brown, dry, slightly plastic, calcareous; trace fine subrounded sand.  O 8 92 30 6  B	DULK SAMPLE	EPTH	LITHOLOGY	uscs	CONSISTENCY	SOIL DESCRIPTION	REMARKS	AN	IEV A LY:	315		
SILTY SAND, brown, fine to coarse, poorly graded, dry, subangular to subrounded, calcareous; some nonplastic silt.		Ó		CL		SILTY CLAY, brown, moist, medium plastic, calcareous; trace fine sand.		GR	SA	FI	LL	P
SILTY SAND, brown, fine to coarse, poorly graded, dry, subangular to subrounded, calcareous; some nonplastic silt.	- 2	-		ML	firm	SILT, light brown, dry, slightly plastic, calcareous; trace fine subrounded send.		0	8	92	30	 
	-3	12-	( <i>*</i> /././	SM		graded, dry, subangular to subrounded, cal-						
	- 8	20-										

SURFACE ELEVATION : 5145' (1568m)
DATE EXCAVATED : 2 NOVEMBER 1980'

SURFICIAL GEOLOGIC UNIT: A5y/A40 TRENCH LENGTH : 14.0' (4.3m)

TRENCH ORIENTATION : N-S

LOG OF TRENCH BL-T-21 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - 800

F160RE

UGRO NATIONAL INC.

USAF-37

BULK SAMPLE	METERS 30	PTH	LITHOLOGY	uscs	CONSI STENCY	SOIL DESCRIPTION	REM	RKS	AN	IEV	:18	 l er
	6	2 -		SM	medium dense	interbedded layers of SILTY SAND and SANDY SILT:  SILTY SAND (SM): brown to light brown, fine to coarse, poorly graded, alightly moist, subangular to subrounded, calcareous; little to some nonplastic silt; trace fine to coarse gravel; stage II caliche (1.0'-5.0').  SANDY SILT (ML): light brown, slightly moist, slightly plastic, calcareous; some fine to medium subangular to subrounded sand.					F1 22	
	- 2 - 3	8-		ML	stiff			i wells ble				
	-4	12-		SM	medium dense	TOTAL DEPTH 14.0' (4.3m)	,		7	77	16	
	-5	18-										
	-6	20-										

SURFACE ELEVATION : 5180' (1579m)
DATE EXCAVATED : 2 NOVEMBER 1980

SURFICIAL EZOLOGIC UNIT: A51

TRENCH LENGTH : 14.0'(4.3m)

TRENCH ORIENTATION : E-W

LOG OF TRENCH BL-T-22 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMB

F1688€ Ⅲ-3-22

TUBBO MATIONAL

USAF-37

A Property of the Party of the

BULK SAMPLE METERS &	PTH	LITHOLOGY	USCS	CONSISTENCY	SOIL DESCRIPTION	REMARKS	AN	I LY:	818		
-1 -2 -3	6 - 8 - 10 - 12 -		SM	medium dense	SILTY SAND, brown, fine to coarse, poorly graded, slightly moist, subengular to subrounded, calcareous; some nonplastic slit; stage II caliche (2.0'-5.0').	vertical walls stable	GR	SA	FI	4	7
-5	18-				TOTAL DEPTH 14.0' (4.3m)						

SURFACE ELEVATION : 5200' (1585m)
DATE EXCAVATED : 2 NOVEMBER 1980

: E-W

SURFICIAL GEOLOGIC UNIT: A51

TRENCH LENGTH : 14.0' (4.3m)

TRENCH ORIENTATION

LOG OF TRENCH BL-T-23
OPERATIONAL BASE SITE

BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - 800

F1 60 FE

UGRO NATIONAL INC.

BULK SAMPLE	WETERS O	PTH	LITHOLOGY	uscs	CONSISTENCY	SOIL DESCRIPTION	REMARKS	AN	E SIS	 	Т.,
	0	2 -		SM- SC		SILTY SAND-CLAYEY SAND, light brown, fine to coarse, poorly graded, slightly moist, subangular to subrounded, calcareous; some slightly plastic silt-clay; stage I caliche (1.5'-5.0').			50		
	- 2	8 -		SP	medium dense	GRAVELLY SAND, brown, fine to coarse, poorly graded, dry, subangular to sub-rounded, calcareous; some gravel.	vertical walls stable				
	4	14-		ML	firm	SANDY SILT, light brown, dry, slightly plastic, calcareous; some fine subangular to subrounded sand.  TOTAL DEPTH 14.0' (4.3m)					
	- 5 - 6	18-									
	- 6	20-									

SURFACE ELEVATION

: 5175' (1577m)

DATE EXCAVATED

: 3 NOVEMBER 1980

SURFICIAL GEOLOGIC UNIT: A5y

TRENCH LENGTH

: 14.0' (4.3m)

TRENCH ORIENTATION

: E-W

LOG OF TRENCH BL-T-24
OPERATIONAL BASE SITE

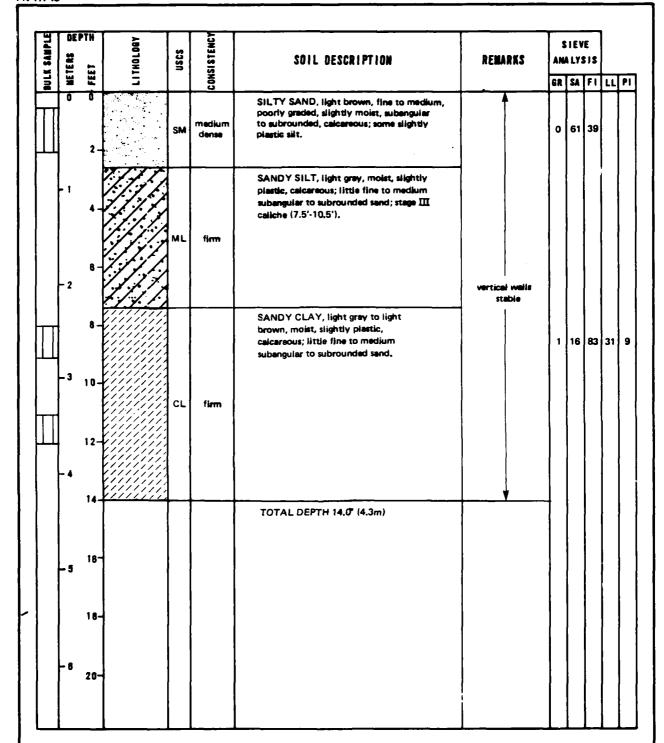
BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - 8000

F1608€ ∐-3-24

UGRO NATIONAL INC.

USAF-37



SURFACE ELEVATION

: 5150' (1570m)

DATE EXCAVATED

: 3 NOVEMBER 1980

SURFICIAL GEOLOGIC UNIT: A3/A40

TRENCH LENGTH

: 14.0' (4.3m)

TRENCH ORIENTATION

: E-W

**LOG OF TRENCH BL-T-25 OPERATIONAL BASE SITE** BERYL, UTAH

MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - BMG

FIGUEE **II-3-25** 

Jero Nationai

USAF-37

BULK SAMPLE	METERS S	HT4	LITHOLOGY	nscs	CONSISTENCY	SOIL DESCRIPTION	REMARKS	AN	LY	315	
3	13	FEET	רוו		NO 3			GR	SA	FI	LL
	0	2 -		SM	dense	Interbedded layers of GRAVELLY SAND and SILTY SAND;  GRAVELLY SAND (SP): brown, fine to coarse, poorly graded, dry, subangular to subrounded, calcareous; little gravel; stage I caliche (0,0'-8.0').  SILTY SAND (SM): brown, fine to coarse,		1	76	23	
	- 2	6-		SP	medium dense	poorly graded, dry, subangular to subrounded, calcareous; some slightly plastic silt; clayey sand (8,0'-14,0').	vertical wells stable				
	<b>-3</b>	10-		sc	dense			2	52	46	
	-4	14-				TOTAL DEPTH 14.0' (4.3m)					
	- 5	16-									
		18-									
	- 6	20-									

: 5160° (1573m) SURFACE ELEVATION : 3 NOVEMBER 1980 DATE EXCAVATED

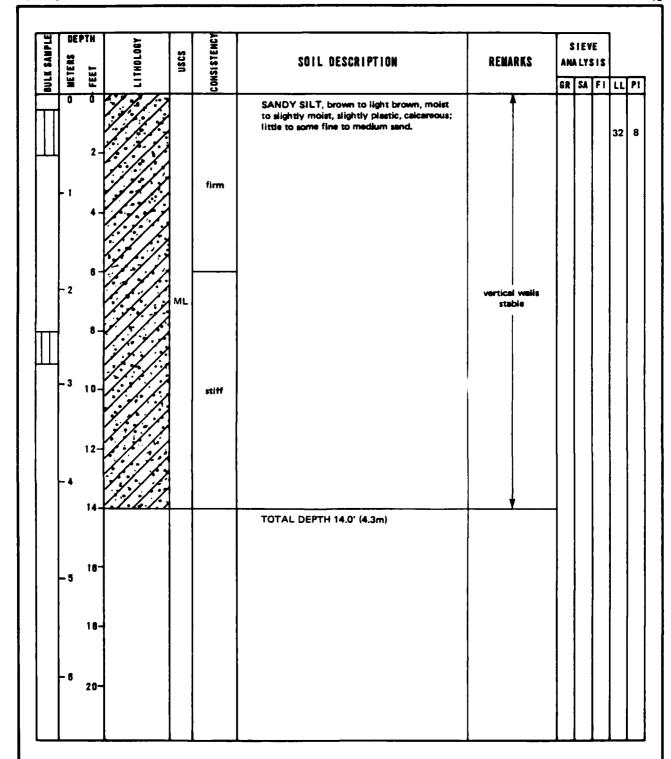
SURFICIAL GEOLOGIC UNIT: A40/A5y TRENCH LENGTH

: 14.0' (4.3m) TRENCH ORIENTATION : E·W

LOG OF TRENCH BL-T- 26 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - BOO

FIGURE **II-3-26** 



SURFACE ELEVATION

: 5175' (1577m)

DATE EXCAVATED

: 3 NOVEMBER 1980

SURFICIAL GEOLOGIC UNIT: A40/A5y

TRENCH LENGTH

: 14.0' (4.3m)

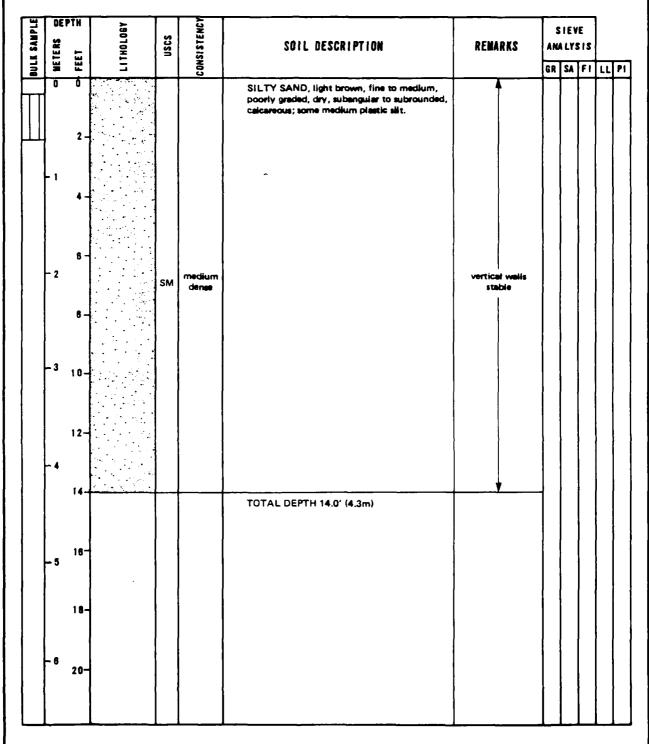
TRENCH ORIENTATION

: N-S

LOG OF TRENCH BL-T-27 **OPERATIONAL BASE SITE** BERYL, UTAH

MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - BMG

FIGURE II-3-27



SURFACE ELEVATION : 5185' (1580m) DATE EXCAVATED : 3 NOVEMBÉR 1980

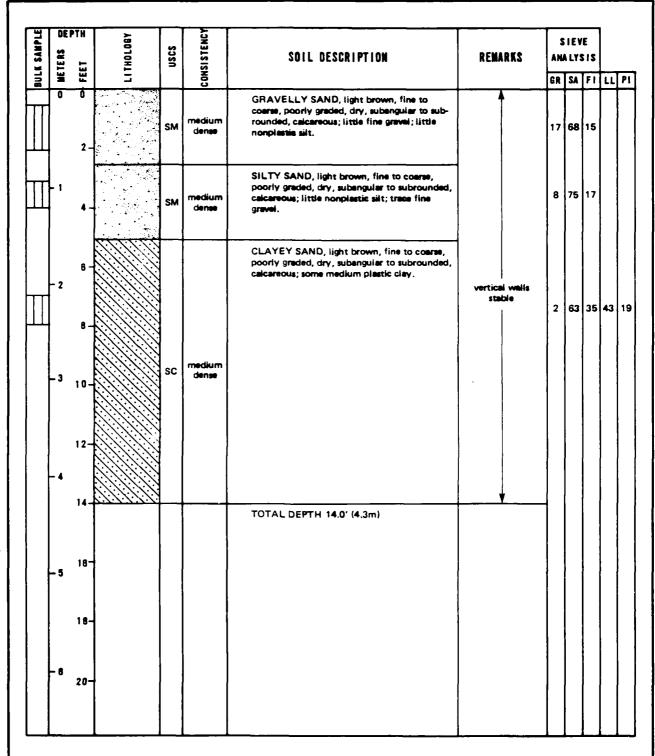
SURFICIAL GEOLOGIC UNIT: A5y/A4o TRENCH LENGTH : 14.0' (4.3m) TRENCH ORIENTATION

LOG OF TRENCH BL-T-28 **OPERATIONAL BASE SITE** BERYL, UTAH

MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - BMG

FIGURE II-3-28

20 MAR 81



SURFACE ELEVATION : 5200' (1585m)

DATE EXCAVATED : 3 NOVEMBER 1980

SURFICIAL GEOLOGIC UNIT: A5y/A4o
TRENCH LENGTH : 14.0' (4.3m)

TRENCH ORIENTATION : N-S

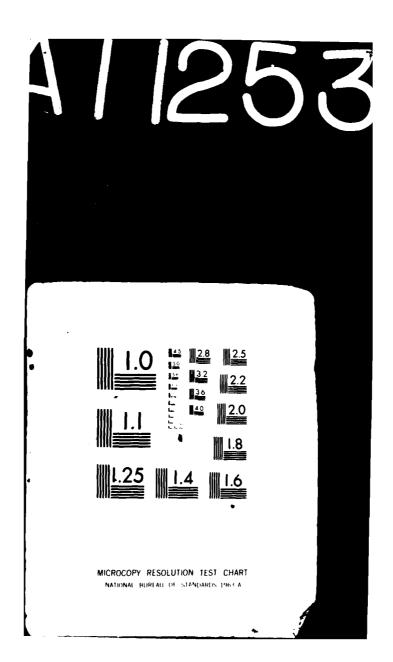
LOG OF TRENCH BL-T-29 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMG

F1608€ Ⅲ-3-29

<u>ugro mational inc.</u>

AD-A112 530 FUORO NATIONAL INC LONG BEACH CA F/6 8/13 MX SITING INVESTIGATION. PRELIMINARY GEOTECHNICAL INVESTIGATION—ETC(U) MAR 81 F04704-88-C-9006 ML FN478-45-VOL-2



DULK SAMPLE METERS 33	FEET	LITHOLOGY	USCS	CONSISTENCY	SOIL DESCRIPTION	REMARKS	AN	SIEV ALY:		Pi
	2-	_			SILTY SAND, brown, fine to coarse, poorly graded, slightly moist, subengular to subrounded, calcareous; some nonplastic sit; stage II caliche (2.0'-3.5') and (6.0'-10.0'); stage III caliche (10,0'-11.0').					NP
-2	8-		SM	dense		vertical walls stable				
-3 1	0-									
1	2-				TOTAL DEPTH 11.0° (3.4m)	cementation at 11.0 exceeded capacity of Case 580C backhoe	3			
	14-									
-5	16-									
	18-									
-6 2	20-									

SURFACE ELEVATION : 5225' (1593m)
DATE EXCAVATED : 3 NOVEMBER 1980

The second secon

SURFICIAL GEOLOGIC UNIT: A5y
TRENCH LENGTH : 14,0' (4.3m)

TRENCH ORIENTATION : N-S

LOG OF TRENCH BL-T-30 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - 9000

#100mm II-3-30

UBRO NATIONAL

BULK SAMPLE	DEPTH		uscs	CONSISTENCY	SOIL DESCRIPTION	REMARKS	AMA I	EVE .YS IS	<u>.</u>	P
	2	-		nedium dense	SILTY SAND, light brown, fine to coerse, poorly graded, slightly molst, subangular to subrounded, calcareous; some nonplastic silt; stage III caliche (4.5'-7.0') and (10.0'-12.0').					
- 2	8		SM	dense		vertical walls stable				
-3	10									
-	12 4 14				TOTAL DEPTH 12.0' (3.7m)	cementation at 12 exceeded capacit of Case 580C backhoe				
<b> </b> -!	16 5									
	<b>6</b> 20	) <del>-</del>								

SURFACE ELEVATION : 5260' (1603m)

DATE EXCAVATED : 3 NOVEMBER 1980

SURFICIAL GEOLOGIC UNIT: A54

TRENCH LENGTH : 14.0' (4.3m)

TRENCH ORIENTATION : N-S

LOG OF TRENCH BL-T-31 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - 2000

#100mm II-3-31

UBRO NATIONAL INC.

SECTION 4.0

EXPLANATION OF TEST PIT LOGS

## 4.0 EXPLANATION OF TEST PIT LOGS

See Section 2.0, "Explanation of Boring, Trench, and Test Pit Logs", for explanations.

DULK SAUPLE	ETERS 30		LITHOLOGY	SSS	C DNS; S T ENCY	SOIL DESCRIPTION	REMARKS	1 -	IEV ALY	_		
3	<u></u>	FEET	5		CON			88	SA	FI	LL	PI
	0      -	1 -			medium dense	SILTY SAND-CLAYEY SAND, brown to it brown, fine to coarse, poorly graded, slight moist to dry, subengular to subrounded, as careous; some slightly plastic slit-clay; trace gravel; stage III caliche (2.0'-5.0').	ty I-	10	44	46	23	5
	1	3 -		SM- SC	dense		vertical walls stable					
	~2	5 8 8 9		SM	dense	SILTY SAND, light brown, fine to coarse, poorly graded, dry, subengular to subround calcareous; some nonplastic silt; trace grave stage II caliche (5.0'-10.0'); trace cobbies to 10" size (7.0'-10.0').	H;					
URF		10-	ATION: 5870' OLDRIC UNIT:	(1728	lm)	TOTAL DEPTH 10.0' (3.0m)						
.URF	1018		osoulo Unil:	ADI			LOG OF TEST PIT OPERATIONAL BAS BERYL, UTAI	E S				
							SITING INVESTIGATION NT OF THE AIR FORCE -					4

20 MAR 81

BULK SABPLE	DEPT		LITHOLOGY	USCS	C ONSISTENCY	SOIL DESCRIPT	ION	REMARKS	_	IEVE ALYS IS		
BULK		o FEET	<u> </u>	_	COMS			<b>A</b>	GR	SA F	ı LL	PI
	1 <b>pa-</b>	1		CL- ML	stiff	SANDY CLAY-SANDY SILT moist, slightly plastic; some fi angular to subrounded sand.					22	4
	<b>– 1</b>	3-				GRAVELLY SAND, light bro coerse, poorly graded, dry, sul subrounded, calcareous; some gravel.	pengular to					
		5						vertical walls stable				
	- 2	7-		SP	medium dense							
	_	9 -										
	-3 1	0 -										
						TOTAL DEPTH 10.0' (3.0m)						
URF	ACE EI	GE(	TION: 5300' DLOGIC UNIT:	(1615 (A5i)	im) /A5y			<u> </u>	1		1_	<b>!</b>
						ſ		G OF TEST PIT E RATIONAL BAS BERYL, UTAH	E SI			
						Ţ		G INVESTIGATION THE AIR FORCE -	1		Π-	4-;
	B1						fuero	NATIO	N/	N.		US A

SANDY SILT, brown, slightly moist, medium plastic; some fine to coarse subangular to subrounded sand.  SR very dense   GRAVELLY SAND, light brown, fine to coarse subangular to subrounded, calcareous; some fine to coarse subangular to subrounded, calcareous; some fine to coarse gravel; trace morphastic sit; stage III callache (2.0**-4.0*); stage III callache (2.0**-6.0*); stage III callache (2.0**-6.0*); stage III callache (3.0**-6.0*); stage III callache (3.	GR SA FI	7
SP. very clense SP. very clens		
TOTAL DEPTH 4.0' (1.2m)  at 4.0' exceeded capetry of Case 580C backhoe		
3 10-		

SURFACE ELEVATION: 5680' (1731m) SURFICIAL GEOLOGIC UNIT: A5i

> LOG OF TEST PIT BL-P-3 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMO

F1808E II-4-3

UBRO NATIONAL II

U8AF-21

BULK SAMPLE	- 1	LITHOLOGY	nscs	C ONS ; S T E MCY	SOIL DESCRIP	TION	REMARKS		IEV ALY:		
	1 -	in .	SM	dense	SILTY SAND, brown, fine to graded, slightly moist, suben rounded, calcareous; some all slit; trace gravel.	pular to sub-		8R 7	8A 60		LL PI
-1	4-				GRAVELLY SAND, light britto coarse, poorly graded, dry subrounded, calcareous; some gravel; trace nonplastic silt; stage II caliche (6 occasional cobbles to 6" size	, subengular to e fine to coarse tage III caliche .0'-10.0');	vertical walls stable				
- 2	7-8-9-4		SP-	dense							
	LEVA	TION: 5540'	(1689 A5i	m)	TOTAL DEPTH 10.0' (3.0m						
							G OF TEST PIT RATIONAL BAS BERYL, UTAI	SE SI		_	
						DEPARTMENT OF	G INVESTIGATION THE AIR FORCE -	<b>9110</b>		_L	F1 601 II-4-
MAR 81				· · · · · · · · · · · · · · · · · · ·		Vero	NATIO	N.	M	J	MC US

DULK SABPLE	METERS H	FEET	I THOLOGY	nscs	CONSISTENCY	SOIL DESCRIPTION	REMARKS	AN	IEV ALY:	:18		
<b>B</b>	0	1 -		SM	medium dense	SILTY SAND, brown, fine to coarse, poorly graded, slightly moist, subengular to subrounded, calcareous; some nonplestic slit; occasional cobbles to 6" size (0.0'-7.0').		GR	SA	FI	ш	P
	-1	3 4 5 6 7		GW- GM	dense	SANDY GRAVEL, light brown, fine to coarse, well graded, dry, subangular to subrounded, calcareous; some fine to coarse sand; trace nonplastic silt; stage III caliche (2.0'-7.0').	vertical walls stable	64	27	9		
	_ 3	9-				TOTAL DEPTH 7.0' (2.1m)	cementation at 7.0' exceeded capacity of Case 580C backhoe					
IRF	ACE I	LEV/	ATION: 5530' ( OLOGIC UNIT:	1686 A5o	Sm)							L

LOG OF TEST PIT BL-P-5 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMG

II-4-5

<u>UBRO NATIONAL INC</u>

BULK SAMPLE	METERS O	PTN	LITHOLOGY	552	CONSISTENCY	SOIL DESCRIPTION	REMARKS		IEVE LYS			
	0	O FEET	=	_	. dense	SANDY GRAVEL, brown to light brown, fine to coarse, poorly graded, slightly moi to dry, subangular to subrounded, calcare	st sue;	GR	SA I		L PI	1_1
	  -  -	1 -		GP- GM	very dense	some fine to coarse send; trace nonplestic stage IV caliche (1.0'-2.0'); trace cobbles to 6" size.	silt; vertical walls o stable					
11		3 -			-	TOTAL DEPTH 2.0' (0.6m)	camentation at 2.0' exceeded capacity of Case 580C backhoe					
	- 1	4-										
		5 –										
	- 2	8 -			: [							
		7-										
	<u> </u>	9-										
	- 3	10-										
URF	ACE ICIA	ELEV	ATION: 5740'( Ologic unit:	1750 A50	m)						<u> </u>	
							LOG OF TEST PIT B OPERATIONAL BAS BERYL, UTAH	E SI				
						DEPARTME	SITING INVESTIGATION INT OF THE AIR FORCE -			I	I-4-(	•

20 MAR 81

BULK SAMPLE HETERS ==	PTH	LITHOLOGY	nscs	C ONS; S TENCY	SOIL DESCRIPTION	REMARKS	AN	IEV ALY:	18		_
2 3	0	<b>-</b>	SM	medium dense	SILTY SAND, brown, fine to coarse, poorly graded, slightly moist, subengular to sub-rounded, calcareous; some nonplastic slit.	•	GR	SA	FI	LL	•
-1	3-		GP- GM	dense	SANDY GRAVEL, light brown, fine to coarse, poorly graded, dry, subengular to subrounded, calcareous; some fine to coarse sand; trace non-plastic silt; stage III caliche (1.0'-4.5'); trace cobbles to 6" size.	vertical wells stabli.					
-	5 –				TOTAL DEPTH 4.5' (1.4m)	cementation at 4.5' exceeded capacity of Case 580C backhoe					
- 2	6 <b>-</b>										
	8 -										
	9 -										
3	10-										

SURFACE ELEVATION: 5400' (1646m) SURFICIAL GEOLOGIC UNIT: A5i

> LOG OF TEST PIT BL-P-7 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - 2010

F1 8088 1-4-7

<u>ugro national inc.</u>

USAF-21

BULK SAMPLE	METERS A	FEET =	LITHOLOGY	nscs	CONS) STENCY	SOIL DESCRIPTION	REMARKS		IEV LLY:	E 8 1 \$		
	0	1 0 FE		ML	firm	SANDY SILT, brown, molet, slightly plastic; some fine to coarse subengular to subrounded sand; trace gravel; stage IV caliche (1.5'-2.0'); trace cobbles to 10" size.	vertical walls stable			F1	LL	P
	-1	3 -	<i>Y. 7. Y. P.</i>			TOTAL DEPTH 2.0' (0.6m)	camentation at 2.0' exceeded capacity of Case 580C backhoe					
		4-										
	-	5 -										
		8-										
	- 2	7-										
	_	8 -										
		9 -										
	- 3	10-										
IRF IRF	AĈE IĈIA	ELEY/	ATION: 5480' Ologic unit:	(1670 A5i	lm)	<u> </u>	<u> </u>	<u> </u>		<u></u>		L

LOG OF TEST PIT BL-P-8 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMO

FISURE II-4-8

UBRO NATIONAL INC.

USAF-21

WLK SABPLE	ETERS 30	PTH =	Tuelet	nscs uscs	CONSISTENCY	SOIL DESCRIPTION	REMARKS		IEV ALY:			
	<u>=</u>	FEET	5	-	C 0015	_		GR	SA	FI	LL	Pf
	0	1 -		sc	dense	CLAYEY SAND, brown, fine to coarse, poorly graded, slightly moist, subengular to sub-rounded; some medium plastic clay.		4	48	48	47	26
	1	3-	Z.v.v.			SILTY SAND, light brown, fine to coarse, poorly graded, dry, subengular to subrounded, calcareous; little nonplastic silt; trace gravel; stage III caliche (3.0'-6.0').	vertical walls stable					
	_	5-		SM	dense							
	2	7-				GRAVELLY SAND, light brown, fine to coarse, well graded, dry, subangular to sub-rounded, calcareous; some fine to coarse gravel; trace nonplattic silt; stage I caliche (6.0'-10.0'); trace cobbles and boulders to 20" size.						
	  - 	8 -		SW- SM	dense		caving					
	- 3	10-				TOTAL DEPTH 10.0' (3.0m)	<b>\</b>					
	ACE	r ge	ÄTION: 5400' Olobic Unit:	(1658 : A5i	3m)		G OF TEST PIT	SE S		-	_	-
							BERYL, UTA	N		<u> </u>	П	

20 MAR 81

BULK SABPLE	ETERS 30	PTH H	LITHOLOGY	2025	ONS) S TENCY	SOIL DESCRIPTION	REMARKS		ALY:	E 818	]	
<u> </u>	<u>L</u>	FEET	5					ER	SA	FI	LL	Ī
	-1	3-		SM	medium dense	SILTY SAND, light brown, fine to coarse, poorly graded, slightly moist to dry, subengular to subrounded, calcareous; some nonplastic silt; trace fine gravel; stage I caliche (1.0'-5.0').	vertical wells	9	60	31		
	-2	5 — 6 — 7 — 8 — 9 — 10 —		SP-	medium dense	GRAVELLY SAND, light brown, fine to coarse, poorly graded, dry, subengular to subrounded, calcareous; some fine to coarse gravel; trace nonplastic silt; trace cobbles to 6" size.						
						TOTAL DEPTH 10.0' (3.0m)						

LOG OF TEST PIT BL-P-10 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMO

FI CURE II-4-10

UBRO NATIONAL INC.

USAF-21

BULK SAMPLI	METERS OF PEET	LITHGLOGY	USCS	CONSISTENCY	SOIL DESCRIPTION	REMARKS	_	IEV LYS			
3100		רונ		c ons			GR	SA	FI	H	PI
	1 -			medium dense	GRAVELLY SAND, brown to light brown, fine to coarse, poorly graded, slightly moist to dry, subengular to subrounded, calcareous; some fine to coarse graves; little nonplastic silt; stage III caliche (1.07-4.07); stage I caliche						
	. 2 -		SM	dense	(4.0'-6.5'); occasional cobbles to 10" size.		24	59	17		
	3 - - 4 - - 5 - 6 -		SP	medium dense		vertical wells stable					 
	7-		SM	medium dense	SILTY SAND, light brown, fine to coarse, poorly graded, dry, subengular to subrounded, calcareous; little nonplastic silt.						
	. B - B -		SP	medium dense	GRAVELLY SAND, light brown, fine to coarse, poorly graded, dry, subengular to subrounded, calcareous; some fine to coarse gravel.						
	10-				TOTAL DEPTH 10.0' (3.0m)						
SURFA	CE ELEV	ATION: 5360' Dlobic unit:	(1634 A5i	lm)			ئىسىد			_	
						OG OF TEST PIT I ERATIONAL BAS BERYL, UTA	SE SI				
					MX SITIN	IG INVESTIGATION			1	711 ∏-4	

BULK SAWPLE	METERS M	FEET #1	LITHOLOGY	nscs	CONS) STENCY	SOIL DESCRIPTION	REMARKS	1	IEV			
3	311		=		000			BR	SA	FI	2	P
	-1	2 -		ML	stiff	SANDY SILT, light brown, slightly moist, slightly plastic, calcareous; little fine subangular to subrounded sand.		0	13	87	32	
	L						vertical walls		Ì			l
П		5		sc	dense	CLAYEY SAND, brown, fine to coarse, poorly graded, dry, subengular to subrounded, calcareous; some slightly plastic clay; trace fine gravel.		5	60	35	28	
	_ 2	7-		SM	dense	SILTY SAND, light brown, fine to coarse, poorly graded, dry, subengular to subrounded, calcareous; some nonplastic silt; trace gravel; stage III caliche (6.0'-7.0').						
	-	9 -		SP-	dense	GRAVELLY SAND, light brown, fine to coarse, poorly graded, dry, subengular to sub-rounded, calcareous; some fine to coarse gravel; trace nonplestic silt.		41	50	9		
Ш	- 3	10-				TOTAL DEPTH 10.0' (3.0m)	<b>*</b>		]     .			

SURFICIAL GEOLOGIC UNIT: A3

LOG OF TEST PIT BL-P-12 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMO

F1 CURE

☐-4-12

<u>UGRO NATIONAL INC.</u>

20 MAR 81

BULE SABPLE	METERS S	PTH E	LITUOLOGY	RSCS MSCS	CONSISTENCI	SOIL DESCRIPTION	REMARKS		ALY:	E 8 ! \$		
3		FEET	=======================================					BR	SA	FI	LL	Ī
	1	3-		sc	dense	CLAYEY SAND, light brown, fine to coarse, poorly graded, dry, subengular to subrounded, calcareous; some slightly plastic clay; little fine gravel; stage III caliche (1.5'-6.5').	vertical walls stable	15	60	25		
		7- 8-				TOTAL DEPTH 7.0' (2.1m)	cementation at 7.0' exceeded capacity of Case 580C backhoe					
	- 3	10-										

LOG OF TEST PIT BL-P-13 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - DMO

F160RE II-4-13

UGRO NATIONAL INC.

USAF-21

BULK SAUPLE	ETERS A	PTH	LITHOLOGY	nscs	NSJ S TENCY	SOIL DESCRIPTION	REMA	RKS		IEV ALY:			
	<b>=</b>	FEET	=		3		ł		en	SA	FI	ĽĽ	ľ
	0	1 -			medium dense	SILTY SAND, light brown, fine to coarse, poorly graded, slightly moist to dry, sub-angular to subrounded, calcareous; little to some nonplastic silt; little fine to coarse gravel; stage III caliche (1.0'-3.0'); trace							
	-	2 -			dense	cobbles to 6" size.							
~	1	3 - 4 - 5 -		SM	medium dense		1	si walis atsie					
	- 2	8-											
		6-		SP- SM	medium dense	GRAVELLY SAND, light brown, fine to coarse, poorly graded, slightly moist, sub-angular to subrounded, calcareous; little fine to coarse gravel; trace nonplastic slit.			19	78	5		
	3	9-											
	,	_				TOTAL DEPTH 10.0' (3.0m)							

LOG OF TEST PIT BL-P-14 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMO

71 eure II-4-14

<u>ugro national il</u>

USAF-21

SURFACE ELEVATION: 5340' (1628m) SURFICIAL GEOLOGIC UNIT: A5;

> LOG OF TEST PIT BL-P-15 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - 800

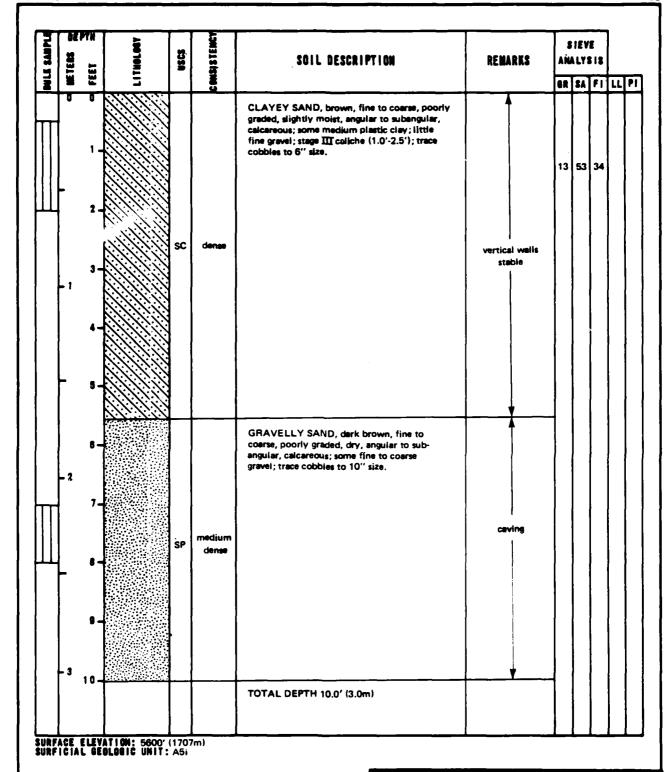
F1 CURE II-4-15

UBRO NATIONAL

USAF-21

DULK SABPLE	ETERS OF		LITHOLOGY	R CS	C BALSĮ S TEIRCY	SOIL DESCRIPTION	REMARKS		BIEV	-		
		O FEET	ın	sc	dense	CLAYEY SAND, light brown, fine to coarse, poorly graded, dry, subangular to subrounded, calcareous; some slightly plastic clay; stage III caliche (1.0'-3.5').		68	SA	FI	LL	PI
	-1	3-										
	<b>-</b>	5				GRAVELLY SAND, derk brown, fine to coerse, poorly graded, dry, subangular to subrounded, calcareous; some fine to coerse gravel.	vertical wells stable		!			
	- 2	7-		SP	dense							
	_	8-										
URF		10-	ATION: 5480'	(1670	Om)	TOTAL DEPTH 10.0' (3.0m)	•					
.unr	IVIAL	- 4E		AUI			OG OF TEST PIT PERATIONAL BA	SE S				
						DEPARTMENT	TING INVESTIGATION THE AIR FORCE	N - <b>20</b> 0			Π-4	urt I-16

20 MAR 81



LOG OF TEST PIT BL-P-17 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - CMO

#18URE

UGRO NATIONAL INC

USAF-21

DULK SABPL	METERS .	FEET HIG	LITHOLOGY	252	CONSISTENCY	SOIL DESCRIPTION	REMARKS		ALY:		!	
<u> </u>	1		17		8 5		1	en	SA	FI	u	Ţ
	0	2		SM	medium dense	SILTY SAND, brown, fine to coarse, poorly graded, slightly moist, subangular to sub-rounded, calcareous; some nonplastic silt.						
	- 2	5-		SP	dense	GRAVELLY SAND, dark brown, fine to coarse, poorly graded, dry, subangular to subrounded, calcareous; some fine gravel; stage III caliche (3.5'-6.0').	vertical wells stable					
	- 3	10		_		TOTAL DEPTH 10.0' (3.0m)						

LOG OF TEST PIT BL-P-18 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - BMG

FI CURE

20 MAR 81

DULK SAMPLE	HETERS S	FEET	LITHOLOGY	ASCS	MSJ S TEMCT	SOIL DESCRIPTION	REMARKS		IEV ALY:			
DAL	1	1 - 2 - 3 - 4 -	in William Wil	sc	dense	CLAYEY SAND, brown, fine to coarse, poorty graded, slightly moist, subangular to subrounded, calcareous; little slightly plastic clay; stage II caliche (1.5'-4.5').		GR.	SA	FI	ıı	
	-2	5 — 6 — 7 — 8 —		SP	dense	GRAVELLY SAND, dark brown, fine to coarse, poorly graded, dry, subangular to subrounded, calcareous; some fine to coarse subangular gravel; occasional cobbles to 6" size.	vertical walls stable					
	- 3	10-				TOTAL DEPTH 10.0' (3.0m)	<b>†</b>	-				

LOG OF TEST PIT BL-P-19 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMG

F190RE II-4-19

<u>ugro national inc.</u>

USAF-21

BULK SABPLE	METERS 30		LITHOLOGY	nscs	CONSISTENCY	SOIL DESCRIPTION	REMARKS		IEV			
	U	- FEET	<b>II</b>		COM	SILTY SAND, tight brown, fine to coarse, poorly graded, dry, subengular to subrounded,	1	BR	SA	FI	LL	P
		1 -				calcareous; little nonplastic silt; trace gravel.						
	-	2_			:							
				SM	medium dense		vertical walls stable					
	- 1	3-										
		4-							ı			
	_1	5 -										
				   		TOTAL DEPTH 5.0' (1.5m)	Bedrock at 5,0' exceeded capacity of Case 580 C backhoe		·			
	- 2	8-										
		7-										
		8	! !		 							ļ
			1		!							
ļ		9 -								i		
	- 3	10-										

SURFACE ELEVATION: 5440' (1658m) SURFICIAL BEOLOGIC UNIT: A5i

> LOG OF TEST PIT BL-P-20 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - SMG

F14088 II-4-20

UGRO NATIONAL

USAF-21

BULK SABPLE	TH L	LITHOLOGY	USCS	CONS; S T ENCY	SOIL DESCRIPTION	REMARKS		AL1	YE '8 11		
	FEET	[1]		200			GR	\$/	F	L	.] P
- 2	1 - 2 - 3 - 4 - 5 - 8 - 9 - 10 - 9 - 10 - 10 - 10 - 10 - 10 -		SM	medium dense	SILTY SAND, light brown, fine to medium, poorly graded, moist, subangular to sub-rounded, calcareous; some nonplastic silt; stage II caliche (2.0"-5.0").	vertical wells stable	2	50	48		
					TOTAL DEPTH 10.0' (3.0m)				l		
						1	1	1		1	1

SURFACE ELEVATION: 5200' (1584m) SURFICIAL GEOLOGIC UNIT: A5i

> LOG OF TEST PIT BL-P-21 OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - 1000

FI BURE

<u>ugro national in</u>

USAF-21

SECTION 5.0

EXPLANATION OF LABORATORY TEST RESULTS

## 5.0 EXPLANATION OF LABORATORY TEST RESULTS

Laboratory test results are presented in this section. Table II-5-1 contains a summary of laboratory test results. This table contains results of sieve analysis; plasticity data; insitu dry unit weight, moisture content, degree of saturation, and void ratio for drive and Pitcher samples; results of compaction tests; and specific gravity of solids. Other tests such as triaxial compression, unconfined compression, direct shear, consolidation, chemical, and California Bearing Ratio (CBR) are indicated on the table. Tables II-5-2 through II-5-4 and Figures II-5-1 through II-5-5 present results of triaxial compression, unconfined compression, direct shear, consolidation, chemical, and CBR tests.

All tests were performed in general accordance with the American Society for Testing and Materials (ASTM) procedures. The following list presents the ASTM designations for the tests performed during the investigation.

Type of Test	ASTM	Designations
Particle Size Analysis	D	422-63
Liquid Limit	D	423-66
Plastic Limit	D	424-59
Unit Weight	D	2937-71
Moisture Content	D	2216-71
Compaction	D	1557-70
Specific Gravity of Solids	D	854-58
Triaxial	D	2850-70
Unconfined Compression	D	2166-66
Direct Shear	· D	3080-72
Consolidation	D	2435-70
Test for Alkalinity (pH)	D	1067-70
Water Soluble Sodium	D	1428-64
Water Soluble Chloride		512-67
Water Soluble Sulphate	D	516-68
Water Soluble Calcium	D	511-72
Calcium Carbonate		1126-67
California Bearing Ratio (CBR)	D	1883-73

Explanation for the tables and figures presented in this section are as follows:

- A. Activity Number Boring, trench, or test pit sample designation.
- B. Sample Number Prefix indicates the type of sample; explanation is at the bottom of the table.
- C. Sample Interval This is the depth range measured from ground surface over which the sample was obtained.
- D. Percent Finer by Weight Presents the results of laboratory particle-size analysis (ASTM D 422-63) performed on representative soil samples at the depth indicated. The numbers represent the percent (by dry weight) of the total sample weight passing through each sieve size indicated.
- E. Atterberg Limits (ASTM D 423-66 and D 424-59) -
  - LL Liquid Limit, the water content (as percent of soil dry weight) corresponding to the arbitrary limit between the liquid and plastic states of consistency of a soil (ASTM D 423-66).
  - PL Plastic Limit, the water content corresponding to an arbitrary limit between the plastic and the semisolid state of consistency of a soil (ASTM D 424-59).
  - PI Plasticity Index, numerical difference between the liquid limit (LL) and the plastic limit (PL) indicating the range of moisture content within which a soil-water mixture is plastic.
  - NP Nonplastic.
- F. USCS Unified Soil Classification Symbols are given here; see Table II-2-1 in Section 2.0, "Boring Logs", for complete details of USCS system.

- G. In Situ Presents results of tests on drive and Pitcher samples.
  - Dry Unit Weight indicates dry unit weight of soil determined as per ASTM D 2937-71.
  - Moisture Content weight of water reported in percent of dry weight of soil sample (ASTM D 2216-71).
  - Saturation the degree of saturation in a soil sample is defined as the ratio (in percent) of the volume of water to the volume of all voids in the soil.
  - Void Ratio the numerical ratio of the volume of voids to the volume of solids in a soil specimen.
- H. Compacted Indicates results of laboratory maximum dry density and optimum moisture content test as per ASTM D 1557-70.
- I. Specific Gravity of Solids (ASTM D 854-58) Indicates the ratio of 1) the weight in air of a given volume of soil solids at a stated temperature, to 2) the weight in air of an equal volume of distilled water at a stated temperature.
- J. Triaxial The triaxial compression tests were performed in accordance with the procedures of ASTM D 2850-70. The following explanations and definitions apply.

Triaxial Compression Test - a cylindrical specimen of soil is surrounded by a fluid in a pressure chamber and subjected to an isotropic pressure. An additional compressive load is then applied, directed along the axis of the specimen called the axial load.

Consolidated-Drained (CD) Test - a triaxial compression test in which the soil was first consolidated under an all around confining stress (test chamber pressure) and was then compressed (and hence sheared) by increasing the vertical stress. "Drained" indicates that excess pore water

pressure generated by strains is permitted to dissipate by the free movement of pore water during consolidation and compression.

Consolidated-Undrained (CU) Test - a triaxial compression test in which essentially complete consolidation under the confining (chamber) pressure is followed by a shear test at constant water content.

Confining Pressure (3) - the isotropic chamber pressure applied to the soil specimen during consolidation and compression.

Maximum Deviator Stress ( $_{1}$ - $_{3}$ ) - the difference between the major and minor principal stresses in the specimen at failure. The major principal stress on the specimen is equal to the unit axial load plus the chamber pressure, and the minor principal stress on the specimen is equal to the chamber pressure.

Strain Rate - axial strain, , at a given stress level is defined as the ratio of the change in length ( L) of the specimen to the original length of the specimen ( $L_O$ ). The rate of strain was controlled during the test so that this ratio increased at equal increments for each minute of testing.

Back Pressure - pressure in excess of atmospheric applied to the pore water of a soil sample. Back pressure is usually applied to 1) increase saturation of the sample, or 2) simulate the actual in situ pressure regime.

- K. Unconfined Compression Test procedures were as described in ASTM D 2166-66. Unconfined compressive strength is defined as the load per unit area at which an unconfined prismatic or cylindrical specimen of soil will fail in a simple compression test. In these methods, unconfined compressive strength is taken as the maximum load attained per unit area or the load per unit area at 20 percent axial strain, whichever occurred first during the performance of a test.
- L. Direct Shear The procedures of ASTM D 3080-72 were followed for direct shear testing. In this test, soil under an

applied normal load is stressed to failure by moving one section of the soil container (shear box) relative to the other section. Normal stress is the value of load per unit area acting perpendicular to the plane of shearing. Maximum shear strength is defined as the maximum resistance (ksf) of a soil to shearing (tangential) stresses.

- M. Consolidation (ASTM D 2435-70) A consolidation test is a test in which a cylindrical soil specimen is laterally confined in a ring and compressed between porous plates. The term "consolidation", as used here, indicates the gradual reduction in volume of the soil mass resulting from an increase in compressive stress (axial load per unit area).
- N. Chemical The chemical tests performed on soil samples included: pH; water soluble sodium, chloride, sulphate, calcium; and calcium carbonate content. pH is an index of the acidity or alkalinity of a soil in terms of the logarithm of the reciprocal of the hydrogen ion concentration.

  ASTM test procedure designations for these chemical tests are included in the list on the first page of these Explanations.
- O. CBR California Bearing Ratio (CBR) is the ratio (in percent) of the resistance to penetration developed by a subgrade soil to that developed by a standard crushed-rock base material. The procedures for conducting a CBR test were as outlined in ASTM D 1883-73. The materials tested

for CBR were also analyzed for particle-size distribution (ASTM D 422-63) and compaction characteristics (ASTM D 1557-70). The term "percentage of maximum density" indicates the ratio (as a percentage) of the compacted sample dry unit weight to maximum dry density obtained in the laboratory from ASTM D 1557-70, "Moisture-Density Relations of Soils Using 10-Pound (4.5-kg) Hammer and 18-inch (457-mm) Drop."

D-4 I	10.2 - 11.0	3.11 - 3.35						100
D-6	15.2 - 16.0	4.63 - 4.88				1		100
D-6	20.2 - 21.0	6.16 - 6.40						
D-7	25.2 - 26.0	7.68 - 7.92					100	90
D-8	30.2 - 31.0	9.20 - 9.45						
D-9	35.1 · 35.9	10.70 - 10.94					100	90
D-10	40.2 - 41.0	12.25 - 12.50	1				100	97
D-12	50.2 - 51.0	15.30 - 15.54	-					
D-13	60.2 - 61.0	18.35 - 18.59					100	81
D-14	70.2 · 71.0	21.40 - 21.64					100	84
D-15	80.2 - 81.0	24.44 - 24.69						
D-17	100.2 - 101.0	30.54 - 30.78						
D-2	1.7 · 2.5	0.52 - 0.76					100	89
D-4	3.7 - 4.5	1.13 - 1.37						
D-6	6.2 - 7.0	1.89 - 2.13						
D-9	11.2 - 12,0	3.41 - 3.66						
D-10	15.2 - 16,0	4.63 - 4.88					100	91
D-12	25.2 · <b>26.</b> 0	7.68 - 7.92						
D-13	30.2 - 31.0	9.20 - 9.45			1			
D-14	37.2 - 38.0	11.34 - 11.58						
D-15	41.2 - 42.0	12.56 - 12.80					100	97
D-17	49.2 - 50.0	15.00 - 15.24						
D-18	60.2 - 61.0	18.35 - 18.59			1			
D-19	70.2 - 71.0	21.40 - 21.64						100
D-19	70.2 - 71.0	21.40 - 21.644				T		_
D-20	80.1 - 80.9	24.41 - 24.66				100	80	70
D-21	90.2 - 91.0	27.49 - 27.74						
D-22	101.2 - 102.0	30.84 - 31.09						
		<u> </u>						
D-3	3.2 - 4.0	0.98 - 1.22		<u></u>			L	<u> </u>
P-6	7.0 - 7.8	2.13 - 2.38			<u> </u>			L
P-8	13.5 - 14.2	4.11 - 4.33						L
P-8	13.5 - 14.2	4.11 - 4.33						
P-8	14.2 - 15.0	4.33 - 4.57						
P-9	19.0 - 19.7	5.79 - 6.00						
P-9	19.7 - 20,4	6.00 - 6.22	I					
D-10	25.2 - 26.0	7.68 - 7.92						
P-11	29.9 - 30.7	9.11 - 9.36						100

D BY WELGHT												
ER BY WEIGHT			IN-SITU					C	OMPACTE			П
U S STANDARD SIEVE NO. PARTICLE SIZE (mm) SAND SILT OR CLAY	ATTERBERG Limits (b)	USCS (c)	ORY (		MOLSTURE Control (\$)	SATURATION (\$)	VOID Ratio	MAXI Dry de		OPTIMUM Moisture (\$)	SPECIFIC GRAVITY OF SOLIDS	
4 10 40 100 200 .005 .001	LL PL PI	( )	(pcf)	(kg/m <sup>3</sup> )	물흥~	SAT (	2 × ×	(pcf)	(kg/m³)		SP. OF	Н
		SM	95.3	1527	11.2	39.6	0.77	(00.17	(100 11 /			Н
	<del></del>	SM	95.3	1527	14.1	49.7	0.77		<del></del>			Н
94 86 66 45 33		SM	100.7	1613	11.5	46.2	0.67					H
95 86 25 11 9	<del></del>	SW-SM	104,3	1671	9.1	39.9	0.62			<del></del>		H
95 88 68 47 33		SM	96.9	1552	13.3	48.4 •	0.74					H
		GP-GM	111.9	1793	13.4	71.6	0.51					П
71 54 24 13 9		SW-SM	118.8	1903	5.3	34.1	0.41					П
		SW-SM	122,9	1969	9.5	69.2	0.37					П
64 44 . 17 8 6		SW-SM	120.3	1927	9.8	66.4	0.40	<del></del>				П
67 49 27 15 12		SW-SM	118.9	1905	7.5	48.4	0.42					П
		SM	117.0	1874	9.3	57.3	0.44					П
54 43 23 13 10		GP-GM	121.6	1948	8.8	61.9	0.39					
55 45 25 13 10		SW-SM	117.2	1878	7.1	43.7	0.44					
		GP	111.8	1791	13.6	72.3	0.51					
		SM	108.4	1737	17.6	85.4	0.56					
66 53 36 24 20		SM	106.6	1708	6.0	28.1	0.58					$\Box$
	NP	SM	113,4	1817	5.9	32.9	0.49					
		SP	109.1	1748	7.2	35.9	0.55					
97 91 77 57 41	NP	SM	105.7	1693	5.2	23.7	0.59					L
70 63 22 11 8		SW-SM	110.1	1764	8.7	44.4	0.53					L
	-	SP	117.0	1874	5.8	35.8	0.44					
		SM	113.6	1820	8.4	47.0	0.48		L			L
		SP	114,5	1834	7.6	43.7	0.47					Ľ
67 53 27 13 7		SP-SM	117.7	1886	10.8	67.7	0.43					L
┡┈┼┈┼┈┼┈┼┈┼	$\longrightarrow$	SP-SM	131.4	2105	4.6	43.9	0.28					Ľ
		SP-SM	112.7	1805	11.1	60.5	0.49			$oxed{oxed}$		L
97 86 39 21 15	NP NP	SM	92.1	1475	6.1	19.8	0.83					L
	$-\downarrow$	SM	109.3	1751	11.9	59.5	0.54					L
<b>3</b> 9 <b>2</b> 9 <b>1</b> 6 <b>9 6</b>	<del></del>	GP-GM	126.5	2027	9.9	81.1	0.33					L
		GP-GM	121.7	1950	9.8	68.7	0.38		l	<b>!</b> i	L	<b>I</b>

_	_		IN-SITU			_		AMA					<del></del> -				
E	2		<u> </u>	!!	_			<u> </u>	OMPACTE		دم	9	<b>8</b> €		<b>₹</b>		
		11000	DRY	UNIT	2 L	5		MAX	MUM	38.	드논쯤	N.	SS	_	DAT	H	
(	9)	USCS (c)	WEI	GHT	MOLSTURE Content (\$)	SATURATION (\$)	<u>-</u> 2	DRY DE	NSITY	OPTIMUM Moisture (\$)	SPECIFIC GRAVITY OF SOLIOS	TRIAXIAL (d)	UNCONFINED Compression	DIRECT Shear	CONSOLIDATION	CHEMICAL	
T	PI	(-/	(pcf)	(kg/m <sup>3</sup> )		SATI (	VOID RATIO	(pcf)	(kg/m³)	90	SPE OF	<b>E</b>	3 5	금등	Sign	35	CBR
7		SM	95.3	1527	11.2	39.6	0.77										
7		SM	95.3	1527	14.1	49.7	0.77										
7		SM	100.7	1613	11.5	46.2	0.67							*			
1		SW-SM	104.3	1671	9.1	39.9	0,62										
T		SM	96.9	1552	13.3	48.4	0.74							*			
Т		GP-GM	111.9	1793	13.4	71.6	0.51										
T		SW-SM	118.8	1903	5.3	34.1	0.41									*	
T		SW-SM	122.9	1969	9.5	69.2	0.37										
1		SW-SM	120.3	1927	9.8	66.4	0.40										
7		SW-SM	118.9	1905	7.5	48.4	0.42										
1		SM	117.0	1874	9.3	57.3	0.44										
J		GP-GM	121.6	1948	8.8	61.9	0.39										
T		SW-SM	117.2	1878	7.1	43.7	0.44										
T		GP	111.8	1791	13.6	72.3	0.51								1		
7		SM	108.4	1737	17.6	85.4	0.56										
7																	
7		SM	106.6	1708	6.0	28.1	0.58							*			
7	NP	SM	113,4	1817	5.9	32.9	0.49										
7		SP	109.1	1748	7.2	35.9	0.55										
1	NP	SM	105.7	1693	5.2	23.7	0.59						*				
7		SW-SM	110.1	1764	8.7	44.4	0.53			<u> </u>							
Т		SP	117.0	1874	5.8	35.8	0.44										
T		SM	113.6	1820	8.4	47.0	0.48										
1		SP	114.5	1834	7.6	43.7	0.47										
T		SP-SM	117.7	1886	10.8	67.7	0.43										
T		SP-SM	131.4	2105	4.6	43.9	0.28						<b>-</b>				
T		SP-SM	112.7	1805	11.1	60.5	0.49						1				
1	NP	SM	92.1	1475	6.1	19.8	0.83			1							
T		SM	109.3	1751	11.9	59.5	0.54								*		
		GP-GM	126.5	2027	9.9	81.1	0.33			t							
		GP-GM	121.7	1950	9.8	68.7	0.38										
$\prod$		GP-GM	107.9	1729	11.0	53.0	0.56										
$\prod$										Ι.							
		GP	119.3	1911	6.3	41.6	0.41			$\Gamma^{}$							
$\mathbf{I}$		GP	97.6	1564	14.4	53.7	0.73										
$\prod$		CL	94.6	1515	17.1	59.2	0.78								*		
	12	CL	88.8	1423	17.8	53.6	0.90						]	]			] 1
	20	CL	93.9	1504	17.1	58.3	0.79						*				
	20	MH	71.1	1139	28.6	57.0	1.34		T		2.67						
T	一	МН	81.7	1309	14.4	36.7	1.06			]							
T		GM	115.2	1846	9.0	52.6	0.46			<b>†</b>		<u> </u>	1				
		SW-SM	113.0	1810	11.5	63.5	0.49							T			
	8	ML	75.0	1202	17.6	83.1	1.25					*					

SUMMARY OF LABORATORY TEST RESULTS
OPERATIONAL BASE SITE
BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMG

148LE II-5-1 1 OF 7

<u>ubro national, inc</u>

AFY-01

ک

10.7 - 11.5	3.26 - 3.51	I i	1 1
14.0 - 14.6	4.27 - 4.45	<del>                                     </del>	+
		<b>↓</b>	+
14.6 - 15.2	4.45 - 4.63	<b>.</b>	
19.2 - 19.9	<b>5.85 - 6.07</b>		
19.2 - 19.9	5.85 - 6.07		
19.9 - 20.7	6.07 - 6.31		" I
25.2 - 25.8	7.68 - 7.86		
29.5 - 30.3	8.99 - 9.24		
30.3 - 31.0	9.24 - 9.45		
35.0 - 35.7	10.67 - 10.88		
35.0 - 35.7	10.67 - 10.88		
40.5 - 41.2	12.34 - 12.56		
47.0 - 47.8	14.33 - 14.57		
47.8 - 48.7	14.57 - 14.84		
49.2 - 50.0	15.00 - 15.24		
0.7 - 1.5	0.21 - 0.46		
3.2 - 4.0	0.98 - 1.22		
6.2 - 7.0	1.89 - 2.13		
9.1 - 9.9	2.77 - 3.02		
	<u> </u>		1 1
1.2 - 2.0	0.37 - 0.61		

R BY W	EIGHT			<del>-</del>			_						N-SITU			C	OMPACTE			3
U S		DARD S	IEVE N	10 .	PART	ICLE		TERBE			DRY I			<u>8</u>		MAX			CIEVE	3
<b>-</b>	SAI				SIZE T OR C		LII	WITS (	(b)	USCS (c)	WEIG		MOISTURE Content (\$)	SATURATION (\$)	VOID Ratio	DRY DE	YTI 2M	OPTIMUM Moisture (\$)	SPECIFATO GRAVITY OF SOLID	3
4	10	40	100	200	. 005	.001	LL	PL	PI	(6)	(pcf)	(kg/m <sup>3</sup> )		SATI (	RAT	(pcf)	(kg/m²)	0 OM	28.9	1
										ML	99.4	1593	13.8	53.4	0.70		1			
			<u> </u>				<u>,                                     </u>	†		ML	79.6	1275	13.2	92.7	1.12					
93	81	53	34	26		1		<u> </u>	NP	SM	99,9	1600	11.3	49.6	0.69		I			
	100	98	95	89				Ī	NP	ML	97.1	1556	21.9	80.6	0.74		ļ	ļ	ļ	H
						ļ		ļ	ļ								<del> </del>			┨┥
						ļ				ML	94.8	1519	15.9	55.5	0.78 1.06		<del> </del>	<del>-</del> -	<del> </del>	H
-00		00	67	47		<b></b> -		- ;-	14	SC SC	82.0	1313 1535	15.5	39.8 50.4	0.76		<del>                                     </del>	<del>                                     </del>	<del>                                     </del>	1-1
96	95	80	57	47	<u> </u>	<b></b>	31	17	14	SM	95.8 102.7	1645	14.2	78.5	0.76		┼	<del> </del>	<del>                                     </del>	H
			<del> </del>			<del> </del>		<del> </del>		SP	117.2	1878	12.9	79.7	0.44		<del>                                     </del>	<del>                                     </del>	<del>                                     </del>	H
100	98	92	83	78		<del> </del>			NP	ML Sr	81.5	1306	16.8	42.6	1.07	<del>                                     </del>	<del>                                     </del>	<del>                                     </del>		17
-:	- 50		<u>                                   </u>	† <del>- " "</del>		<del>                                     </del>		<del> </del>	<del>                                     </del>	ML	81.6	1307	18.7	47.3	1,06	<b>†</b>	1			$\square$
100	98	94	75	52		<u> </u>			NP	ML	95.2	1525	9.8	34.2	0.77					Ш
						Ι		[		ML	99.2	1589	9.7	37.4	0.70			<u> </u>	<b></b>	1
100	99	95	83	68		1			NP	ML	99.3	1591	10.1	39.0	0.70	I	I	<b></b> _	↓	┷
						Ī				ML	104.5	1674	11,5	50.6	0.61	I	1	<u> </u>	<b>↓</b> —	╁╌
100	99	90	62	45				<u> </u>	NP	SM	90.0	1442	13.0	40.3	0.87	L	.l	<del> </del>	<del> </del>	╁╣
							L		L	SM	100.5	1610	10.3	41.2	0.68	<b>↓</b>	<b>↓</b>	<b>-</b>	2.59	<b>┼</b> ┚
81	78	67	36	25		L		ļ .	NP	SM	98.3	1575	14.2	57.5	0.64	<b></b>	<del> </del>	<del> </del>	2.59	╁╌
		ļ	ļ	ļ		<b> </b>	<b>.</b>	Ļ	ļ	SM	103.1	1652	19.5	88.6	0.57	<b>↓</b>	<del></del>	<del> </del>	<del> </del>	╂╌
					ļ	ŀ	ł	ł		SM	100.8	1615	22.4	90.1	0.67	₽	ļ	<del>                                     </del>	<del>                                     </del>	+-
69	50	37	26	19		ł	<b> </b>	<del> </del>	ļ	SM SM	106.4 102.5	1705 1642	15.6 14.2	72.2	0.58			<del>                                     </del>	<del>                                     </del>	+
77	65	47	33	25	<b></b>	}	<b></b>	<b>}</b>	<b></b>	SM	115.2	1846	8.0	59.5 46.6	0.64		- +	<del> </del>	<del>                                     </del>	1
<del></del>	05	7'	33	25	<u> </u>	<del></del>	<b>-</b>	<del> </del>	<del> </del>	3141		1040	1-0.0	70.0	0.40	<del> </del>	<del>                                     </del>	1	1	1
	<del>}</del>	<del>                                     </del>	<del>}</del>	<b>}</b>	<del> </del>	<del>}</del>	<del> </del>	<del>                                     </del>	├	SM	93.8	1503	11.9	40.3	0.80	<del>}</del>	+	1	1	T
4C	32	20	13	10	t	<del>                                     </del>	<del>                                     </del>	╁──	t	GW-GM	126.6	2028	5.3	42.9	0.33	<del>                                     </del>	1			L
-				<del>                                     </del>		†	<u> </u>	†	<del>                                     </del>	GW-GM	123.2	1974	8.4	61.5	0.37	1				
			<b>†</b>	1	<u> </u>	1	1	1	1	GW-GM	119.9	1921	8.1	54.2	0,41	1			$\perp$	L
				<u> </u>	<u> </u>	<u> </u>							1		I				<b>↓</b>	丰
						ļ			L	SM	94.6	1515	6.4	22.1	0.78	<b>_</b>		<del>                                     </del>	+	+
	<u> </u>	<b> </b>	<b></b>	ļ	L	<b>├</b>	<b>-</b>	<b>├</b>	<b> </b>	SM	98.4	1576	20.0	76.0	0.71	<del> </del>	<del></del>		+	十
		<del> </del> _	-		L	<del> </del>	<b></b> -	<b>}</b>	<b>-</b>	SM	110.3	1767	7.0	35.9	0.53		4	+	<del> </del>	十
_76	66	46	31	23_	ļ	<b> </b>		<b> </b>	<b></b>	SM	107.0	1714	5.5	26.1	0.57		+	-	+	十
70	=	22	-	1,7	<b></b> -	ļ	<del> </del>	<del> </del>	<del> </del>	GP-GM SM	128.2	2054	3.3 8.2	28.9 52.3	-			+	+	十
	59	33	21	17	<b></b>	<del> </del>	├	<del> </del>	<del> </del>	SP-SM	118.3 114.2	1894 1829	13.1		<b>+</b>		_	+	+	十
99	97	82	59	43		F	ł	ł · · -	NP	SP-SM SM	107.2	1717	7.7	74.5 36.4	0.48	_	+	+	<del>                                     </del>	十
	<del>  "</del> -	1 02	1 22	+	<del> </del>	<del> </del>	<u> </u>	<del> </del>	† <del>' ''</del>	SM	101.8	1631	12.1	49.5	0.66	-	<b>†</b>	1	1	1
	<del>                                     </del>	<del>                                     </del>	<del>                                     </del>	<del> </del>	<del> </del>	t	t	<b>†</b>	<del>                                     </del>	GP	119.1	1908	11.0	71.5	0.4		1	1		T
	<del>                                     </del>	<del> </del>	<del> </del> -	+-	<b></b>	<del>                                     </del>	<del> </del>	<del> </del>	<del>                                     </del>	GP-GM	123.2	1974	6.3	46.5			1	1		floor
61	48	15	5	2	<del> </del>	<del> </del>	t	t		SP	120.5	1930	5.2	35.2			1			$oldsymbol{\perp}$
			1						L	SP-SM	113.3	1815	1.1	42.8	0.48				<u> </u>	_1_
																				_

SU

DEPM

2

					I-SITU			C	OMPACTE	D		(g)	e 8		8		
rbe Is (		uscs	DRY L		MOISTURE CONTENT (\$)	SATURATION (\$)	) 10	MAXI DRY DE		OPTIMUM Moisture (%)	SPECIFIC GRAVITY OF SOLIDS	TRIAXIAL (d)	UNCONFINED COMPRESSION	DIRECT SHEAR	CONSOLIDATION	CHEMICAL	œ
	PI	(c)			MOIST (\$)		VOID RATIO	(pcf)	(kg/=3)	PPI (	SPE GRA OF	TRI	돌등	三器		3	85
l.	<b> </b>		(pcf)	(kg/m³)			0.70	(101)	(-0- /	├			<del>                                     </del>	-			
_		ML	99.4	1593	13.8	53.4	_		<del>                                      </del>	┼			<del>                                     </del>				
	316	ML	79.6	1275	13.2 11.3	92.7 49.6	1.12 0.69			├			t				
	NP	SM	99.9 97.1	1600 1556	21.9	80.6	0.74			├	<del> </del>	<del></del>	<b>†</b>	*			
	NP	ML	97.1	1990	21.5	80.0	0.74		<del> </del>	<del> </del>	t		<u> </u>				
_		ML	94.8	1519	15.9	55.5	0.78		<del>                                     </del>	<del>                                     </del>							
		SC	82.0	1313	15.5	39.8	1.06		<del>                                     </del>	<del> </del>							· 
7	14	SC	95.8	1535	14.2	50.4	0.76		<del> </del>	1	†		1				<u> </u>
7		SM	102.7	1645	18.6	78.5	0.64		1	<b>†</b>	1						
_	<del>                                     </del>	SP	117.2	1878	12.9	79.7	0.44	<b></b>	$t^{-}$	1					<u> </u>	<b></b>	<u> </u>
	NP	ML	81.5	1306	16.8	42.6	1.07	<u> </u>	1	1		*			L		<u> </u>
_		ML	81.6	1307	18.7	47.3	1.06			1		*	1	ļ	<b>└</b> ─	<b></b>	<b>-</b>
	NP	ML	95.2	1525	9.8	34,2	0,77			1	1		<del></del>	<del> </del>	<b>↓</b>	<b>}</b>	<b>├</b> ─-
		ML	99.2	1589	9.7	37.4	0.70			1		<u> </u>	↓	<b>!</b>	*	<b>↓</b>	├
	NP	ML	99.3	1591	10.1	39.0	0.70					<b>↓</b>	*	<b>↓</b>	<b>↓</b>	<b>├</b>	├
		ML	104.5	1674	11.5	50.6	0.61				<u> </u>	1	<b>↓</b>	<b>├</b> ──	<b>↓</b>	<del> </del>	├─-
	NP	SM	90.0	1442	13.0	40.3	0.87				<u> </u>	1 *	╁——	<del> </del>	<b>↓</b>	₩	├
		SM	100.5	1610	10.3	41.2	0.68		<u>L</u> _	<b>_</b>	<del> </del>	*	<del>-</del>	╂	╂	₩-	╫
	NP	SM	98.3	1575	14.2	57.5	0.64	<u> </u>		↓	2.59	<del>-</del>		╂	+	+	╂──
		SM	103.1	1652	19.5	88.6	0.57	<u> </u>	<b></b>		<del> </del>	╂	+	┼──	╁╾	+	╁─
	L	SM	100.8	1615	22.4	90.1	0.67	<u> </u>	↓	<b></b> -	╂	+-	+	+	<del> </del>	┼	t
		SM	106.4	1705	15.6	72.2	0.58	<u> </u>	<del></del>		╂	+-	+	+	├─	╅──	+-
		SM	102.5	1642	14.2	59.5	0.64			<del></del>	-}		+ 🚡	+	+-	*	+-
<b>—</b>		SM	115.2	1846	8.0	46.6	0.46	<b></b>		∔	╂	+-	+	+	+	<del>                                     </del>	+
_						<b> </b>	<u> </u>	↓	∔	<del></del>	<b></b> -	+-	-+	+	+-	+	┼
	<b></b>	SM	93.8	1503	11.9	40.3	0.80		╀	+	+	╁─		+-	+-	1	$\top$
	<b>-</b>	GW-GM	126.6	2028	5.3	42.9	0,33		<del></del>	+	+	+-	+	+	+-	1	$\top$
_	<b> </b>	GW-GM	123.2	1974	8.4	61.5	0.37		┼—		+	+-	+	1	+	*	$\top$
<u> </u>	<b>└</b>	GW-GM	119.9	1921	8.1	54.2	0.41	<u> </u>	+-		+	+-	+	<del>                                     </del>	+-		$\top$
	<del> </del>	<u> </u>	04.6	1535	64	20.4	+	1	+-		_	$T^-$	1	1	$\top$		
	<del> </del> -	SM SM	94.6 98.4	1515 1576	6.4 20.0	76.0	0.78		+-		+	1					oxdot
_	<del>                                     </del>	SM	110.3	1767	7.0	35.9	0.71		+			1	$\mathbf{I}^{-}$				<u> </u>
<b>-</b>	<del> </del>	SM	107.0	1714	5.5	26.1	0.5										Щ
-	<del>                                     </del>	GP-GM	128.2	2054	3.3	28.9	_	_	+-						1_		4_
_	t	SM	118.3	1894	8.2	52.3			+-	1							
_	<del>                                     </del>	SP-SM	114.2	1829	13.1	74.5	+		+	_							
	NP	SM	107.2	1717	7.7	36.4			+-	<del>    -                                  </del>		1	*				┷
	†	SM	101.8	1631	12.1	49.5			<del>                                     </del>								┷
-	<del>                                     </del>	GP	119.1	1908	11.0	71.5			<b>T</b>								┷
	t-	GP-GM	123.2	1974	6.3	46.5			T	$\top$					$\bot$	_}	
	†	SP	120.5	1930	5.2	35.2		_							4-		—
-		SP-SM	113.3	1815	7.7	42.8											

SUMMARY OF LABORATORY TEST RESULTS
OPF ATIONAL BASE SITE
BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMO

TABLE ∏-5-1 2 QF 7

UBRO NATIONAL INC.

AFY-01

2\_

3

30.8 31.5	9.39 - 9.60	<u> </u>
35.0 - 35.7	10.07 - 10.88	
35.7 - 36.3	10.88 - 11.06	
36.3 - 36.6	11.06 - 11.16	
39.0 - 39.6	11.89 - 12.07	
45.2 - 45.8	13.78 - 13.96	
49.0 - 49.7	14.94 - 15.15	
49.7 50.5	15.15 - 15.39	
50.7 51.5	15.45 - 15.70	
0.7 - 1.5	0.21 - 0.46	
4.7 - 5.5	1.43 - 1.68	
7.2 - 8.0	2.19 - 2.44	
10.2 11.0	3.11 - 3.35	
15.2 - 16.0	4.63 - 4.88	
20.0 - 20.4	6.10 - 6.22	
25.1 - 25.8	7.65 - 7.86	
25.8 26.2	7.86 - 7.99	
30.2 - 31.0	9.20 - 9.45	
35.2 36.0	10.73 - 10.97	

	l	L	L	1	1	L		SP-SM	110.9	1777	15.6	80.9	0.52
								SP-SM	114.2	1829	5.3	30.3	0.48
35	16	12						SP SM	114.4	1833	9.4	53.8	0.47
								SP-SM	89.4	1432	17.6	53.8	0.89
								SM	92.9	1488	15.9	52.6	0.81
45	26	19	5	2			NP	SM	99.9	1600	12.1	47.7	0.69
								SM	104.9	1680	12.9	57.7	0.61
84	65	54	<u> </u>		29	21	8	CI.	94.5	1514	20.8	71.8	0.78
		<u></u>	<u></u>					Cl	106.7	1709	12.9	60.1	0.58
			<u> </u>					CL	111.1	1780	11.3	<b>5</b> 8.7	0.52
								SM	108.2	1733	14.5	70.6	0.56
								SM	98.9	1584	22.3	85.5	0.70
88	66	49					NP	SM	107.2	1717	11.8	55.7	0.57
								SM	104.5	1674	13.7	60.3	0.61
			L					SM	110.9	1777	13.0	67.8	0.52
		<u></u>											
	L							SM	115.6	1852	11.4	67.4	0.46
66	48	42						SM	92.5	1482	11.1	36.5	0.82
								SM	114.1	1828	4.5	<b>25</b> .5	0.48
36	25	22						SM	106.6	1708	9.4	43.9	0.58
69	48	34					NP	SM	106.9	1713	7.3	34.3	0.58
51	21	13						SM	108.1	1732	12.3	59.5	0.56
79	46	29					NΡ	SM	100.9	1616	14.3	59.2	0.64
							,	SM	101.9	1632	14.7	62.8	0.62
32	17	11						SP-SM	113.8	1823	5.8	32.4	0.48
								SM	105.2	1685	9.3	41.6	0.60
15	8	5						GP-GM	126.2	2022	8.1	65.5	0.34
74	43	28	L	L				SM	79.4	1272	13.2	92.4	1.12
			<u></u>					SM	97.5	1562	15.1	55.9	0.73
74	47	29	i	l	<b>i</b>	l	NP	SM	91.0	1458	14.4	45.8	0.85

	ERG (b) USCS (c)			11	I-S I TU			C	OMPACTE	)		<del>G</del>	25		8		
			DRY U	JNIT GHT	MOISTURE Content (\$)	SATURATION (\$)	VOID RATIO	MAXI Dry de	MUM	OPTINUM Moisture (%)	SPECIFIC GRAVITY OF SOLIDS	TRIAXIAL (d)	UNCONFINED COMPRESSION	DIRECT SHEAR	CONSOLIDATION	CHEMICAL	CBR
	PI		(pcf)	(kg/m³)		VS	25	(pcf)	(kg/m³)	<b>6 3</b>	25 50	_=	3 5	0 8	몽	25	2
П		SM	86.8	1391	8.4	24.0	0.94						Γ			-1	
		SM	102.6	1644	16,4	68.8	0.64										
		SM	89,9	1440	10.1	31.2	0.87										
	NP	SM	109.2	1749	7.8	38.9	0.54						L		l		
	NP	GP-GM	113.1	1812	13.6	75.3	0.49										
		SP-SM	110.9	1777	15.6	80.9	0.52						*				
		SP-SM	114.2	1829	5.3	30.3	0.48							L			{
		SP-SM	114.4	1833	9.4	53.8	0.47		L				<u> </u>			*	
		SP-SM	89.4	1432	17.6	53.8	0.89						<u> </u>				
		SM	92.9	1488	15.9	52.6	0.81						*				
	NP	SM	99.9	1600	12.1	47.7	0.69						<u> </u>		}		
Ц		SM	104.9	1680	12.9	57.7	0.61				ļ	ļ	*	<del>}                                    </del>			
Ц	8	Cl.	94.5	1514	20.8	71.8	0.78		L	<b></b>		*	<b>↓</b>	<b>├</b>			∤
Ц		cı"	106.7	1709	12.9	60.1	0.58		<b> </b>	<b> </b>	<b>↓</b>	*	↓	<b>}</b> _			
Ц		CL	111.1	1780	11.3	<b>5</b> 8.7	0.52		<b> </b>	<b>↓</b>	<b> </b>	ļ	<b>├</b>	<b>├</b>	*		
Ц		SM	108.2	1733	14.5	70.6	0.56		ļ	<b> </b>	ļ		<b></b>				
Ц		SM	98.9	1584	22.3	85.5	0.70		<b>-</b>	<b> </b>		ļ	<b>↓</b>				
Н	NP	SM	107.2	1717	11.8	55.7	0.57		<b></b>	<b>↓</b>		ļ	<del> </del> _	<b>├</b> ──			
Н		SM	104.5	1674	13.7	60.3	0.61		<b></b>	<b>├</b> ──	<del>                                     </del>	<b>├</b> ──	*			<del>i</del>	-
Н		SM	110.9	1777	13.0	67.8	0.52		<b></b>	<del> </del>	<del> </del>	<b>-</b>	<del> </del>				
Н		├ <del></del>	115.0	1050			240		<b>├</b> -	<del> </del>	<del> </del>	}	╂~~~	<b>├</b> ──			
Н		SM	115.6	1852	11.4	67.4	0.46	<u> </u>	<b>├</b> ──-	<del>{</del>	<del> </del>	<del> </del>	+	<del>{</del> -			1
H		SM	92.5	1482	11.1	36.5	0.82		<del> </del> -	<b>∤</b>	<del> </del>	├	*	<del> </del>		<del>  </del>	
H		SM	114.1	1828	4.5	25.5	0.48	<b></b>	<del> </del>	<del>}</del>	<del> </del>	<del> </del>	+-	<del>                                     </del>			
Н	NO	SM	106.6	1708	9.4	43.9	0.58		<b></b> -	<del>}</del>	<del> </del>	├	+	<del>                                     </del>			
H	NP	SM	106.9	1713	7.3	34.3	0.58		<b></b>	╂	├──	├──	┿╾	+			
Н	NO	SM SM	108.1 100.9	1732	12.3	59.5 59.2	0.56		<del> </del>	┼	2.65	<del>                                     </del>	+-	+	<u> </u>		
Н	NP			1616	14.3		0.64		<del>├</del> -	+	2.03	<del> </del>	┿	+	*		-
H		SM SP-SM	101.9 113.8	1632 1823	14.7 5.8	62.8 32.4	0.62	<del></del>	<del> </del>	╂──	<del>                                     </del>	<del>                                     </del>	+	+	<u> </u>	<b>-</b>	
Н		SM	105.2	1685	9.3	41.6	0.48	<b> </b> -	<del>                                     </del>	+	<del>                                     </del>	t —	+	<del>                                     </del>	<u> </u>	<b></b>	
H		GP-GM	126.2	2022	8.1	65.5	0.60	<del> </del> -	<del>                                     </del>	1	1	t	1	<del>                                     </del>	<u> </u>		
H		SM	79.4	1272	13.2	92.4	1.12	<del>                                     </del>	<del>                                     </del>	1	1	1		T			
H		SM	97.5	1562	15.1	55.9	0.73	<del>                                     </del>	<del>                                     </del>	1	1	1	*				
H	NP	SM	91.0	1458	14.4		0.85	<b> </b>	1	<del>                                     </del>	1	1	*				
П	$\neg \neg$	SM	98.3	1575	11.5	43.7	0.71	<u> </u>	1	1					*		
Н		SM	107.0	1714	14.9	69.9	0.58				1		Τ-	*			
H	NP	SM	109.0	1746	12.4	61.4	0.55	<u> </u>	1	1	1		*			*	
	7	SM	85,4	1368	16.1	100.0	*	<b></b>		1	T -			*			
П	$\neg$	SM	100.3	1607	19.2	76.2			1	T -	T				*	]	
П		SP-SM	91.4	1464	18.2	100.0		<b></b>	1					*			
Ħ		SM	111.9	1793	15.2	81.2			T	1					1	1	<u> </u>
		SM	111.6	1788	16.8	88.9											لبل

SUMMARY OF LABORATORY TEST RESULTS
OPERATIONAL BASE SITE
BERYL, UTAH

MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - DMO

TABLE 11-5-1 3 OF 7

UBRO NATIONAL ING.

AFY-01

ć---

	Ĉ								PERCEI	IT FINE	R BY W	EIGHT	
KTIVITY	.E :R (a)	SAMPLE I	NTERVAL	STANDARD SIEVE OPENING							U S	STAN	DARO
ACTIVIT NUMBER	SAMPLE Number			BLDRS	COB	BLES		GRA	VEL			SA	ND
2 2	S E	FEET	METERS	24"	12"	6"	3"	1½"	3/4"	3/8"	4	10	4
BL·B·9	D·2	3.2 - 4.0	0.98 - 1.22					100	70	60	51	38	18
	D-3	5.2 · 6.0	1.58 - 1.83										
	D-5	13.1 - 13.9	3.99 - 4.24						100	92	68	46	21
	D-6	19.2 - 20.0	5.85 - 6.10										
	D-7	<b>25.2 · 26.</b> 0	7.68 - 7.92										
L	D-8	30.2 - 31.0	9.20 - 9.45										
	D-9	35.2 · 36.0	10.73 - 10.97				100	86	83	76	65	49	21
	D-10	40.1 - 40.9	12.22 - 12.47										
	D-12	50.1 - 50.9	15.27 - 15.51										
DI 0 10		05.12	0.15 0.27	$\vdash$									
BL-B-10	P-1 D-2	0.5 · 1.2	0.15 · 0.37	<del>                                     </del>		ļ	$\vdash$	100			0.		
	P-3	3.2 · 4.0 6.5 · 7.7	0.98 - 1.22 1.98 - 2.35	╂┈╌┤		<del></del>		100	90	87 100	81 96	68	46 53
	P 3	7.7 - 8.5	2.35 - 2.59	-		<del></del>	<del> </del>		-	100	90	86	23
	D-4	10.2 - 11.0	3.11 3.35	┢		ł	<u> </u>		100	91	73	52	24
	D-5	15.2 - 16.0	4.63 - 4.88	╂──┤		<u> </u>			100	91	/3	32	24
	D-6	20.2 21.0	6.16 6.40	<del>                                     </del>								<del> </del>	
	P·7	25.0 - 25.6	7.62 - 7.80	╉┈┤		<u> </u>			r		<u> </u>	<b>}</b> -	<u> </u>
	D-8	30.2 - 31.0	9.20 - 9.45	<del>  </del>								<del> </del>	<b>-</b>
	D-9	35.2 - 36.0	10.73 10.97	╅		<del></del>		100	96	89	79	66	26
	D-10	40.2 · 41.0	12.25 - 12.50	-	-			100		-00		- 55	
	P-11	46.6 47.1	14.20 14.36	1 1									
	D-12	50.2 · 51.0	15.30 - 15.54	<del>   </del>		<del>                                     </del>						<del>                                     </del>	<del></del>
	D-13	60.2 - 61.0	18.35 - 18.59	1			<del> </del>		<del> </del>	-		<del> </del>	<del>                                     </del>
	P-14	70.8 - 71.4	21.58 - 21.76	1		<del> </del>			<u> </u>			<del>                                     </del>	├──
	P-15	81.8 - 8.25	24.93 - 25.15							100	96	72	16
	P-16	90.9 91.8	27.71 - 27.98			<u> </u>		100	79	69	60	49	24
	D-17	100.2 - 101.0	30.54 - 30.78	t –						- 55			-
												<u> </u>	
BL B 11	D-1	0.7 - 1.5	0.21 - 0.46					100	84	76	72	67	57
	D-4	5.2 · 6.0	1.58 - 1.83					100	81	64	49	37	22
	D-6	10.2 - 11.0	3.11 - 3.35										
	D-7	15.2 - 16.0	4.63 - 4.88										
	D-8	20.2 - 21.0	6.16 - 6.40										
	D-10	29.2 - 30.0	8.90 - 9.14					100	91	68	51	38	23
	D-11	35.0 - 35.5	10.67 - 10.82										
	D-12	40.2 - 41.0	12.25 - 12.50									L	
	D-14	50.2 - 51.0	15.30 - 15.54										
	D-15	60.2 - 61.0	18.35 - 18.59										
	D-16	70.7 - 71.5	21.55 - 21.79										
	D-17	80.2 · 81.0	24.44 - 24.69					100	88	78	<b>6</b> 5	49	25
	D-19	100.2 - 101.0	30.54 30.78										

## NOTES:

(a) Sample types

(c) USCS - Unified Soil Classification System

SS - Standard split spoon

P - Pitcher

(d) \* Indicates that test has been performed and results are included in this report

0 - Fugro Drive

B, b - Bulk

(b) NP - Not Plastic

FIN	IER BY WEIGHT											11	N-SITU	COMPACTED						
	U :		DARD S	IEVE I	VE NO. PARTICLE SIZE (mm)				TERBE Mits (		uscs	DRY	DRY UNIT		SATURATION (\$)		MAXIMUM		OPTINUM MOISTURE (8)	SPECIFIC GRAVITY
		SAND SIL					LT OR CLAY		#113 (	( 0 )	(C)	WEI	WEIGHT		<b>₹</b> €	VOID RATIO	DRY DENSITY		E IS	
9/8"	4	10	40	100	200	. 005	.001	LL	PL	PI	ì í	(pcf)	(kg/m <sup>3</sup> )	MOISTURE Content (\$)	SAT	22	(pc1)	(kg m2)	등을	2 2
60	51	38	18	10	7					t	GP GM	110.6	1/72	1.1	42.2	0.48			† —	2.5
		<b>†</b>	- <del></del> -		<b></b> -	<b></b>	<del> </del>		<b></b> -	ţ	GP-GM	103.7	1661	13.1	56.7	0.63		<b>†</b>	<b>†</b>	<u> </u>
92	68	46	21	10	8		t	t		t	SW SM	113,8	1823	7.9	43.6	0.49		1		
		†	†				t	†		<b>†</b>	GP-GM	115.4	1849	4.5	26.6	0.46		1		
		<u> </u>							1	T	GP-GM	110.2	1765	6.2	31.9	0.53				
	I	T			[					T	GP-GM	109.9	1/61	4.9	24.7	0.53		I		
76	65	49	21	10	1					T	SW-SM	116.5	1866	7.6	45.8	0.45				$\Gamma$
											GP GM	116.4	1865	9.5	57.1	0.45			$I_{}$	
											GP-GM	127.9	2049	1.2	61.4	0.32		Ι		
	L																			
	L										SM	101.7	1629	1.2	29.7	0.66				
87	81	68	46	30	25				I		SM	109.3	1751	9.9	49.5	0.54		ļ	ļ	
100	96	86	53	35	29		L		<u>.</u>	L	SM	95.7	1533	17.6	62.6	0.76		L		<u> </u>
				L	L		<b> </b>	L	L	L	SM	98.1	1572	13.2	49.7	0.72		L	<b>↓</b>	↓
91	73	52	24	13	10		L	<b>.</b>	L	NP	SW-SM	113,7	1821	5.6	31.1	0.48		<b></b>	<u> </u>	<b>Ļ</b>
				ļ.					ļ	ļ	SP	109,8	1759	8.5	43.1	0.53		<b>↓</b>	<b>↓</b>	↓
							L	L	L	ļ	SP-SM	115.3	1847	10.0	58.7	0.46		ļ	<b>↓</b>	<b>↓</b>
				Ļ		,			L		SP	107.8	1/2/	13.1	63.0	0.56		↓	<b></b>	—
- 00						ļ			ļ	ļ	SP	113.6	1820	10.7	59.6	0.48			<b>_</b>	<b>├</b>
<b>8</b> 9	79	66	26	16	13			<u> </u>	ļ	<b> </b>	SM	116.8	18/1	10.3	63.0	().44		<b></b>	<b>↓</b>	<b>├</b> —
				ļ				Į.		ļ	SM	106.9	1713	14.1	66.2	0.58		<b></b>	<b>↓</b>	╂
				ļ				<u> </u>	L	<b></b>	SP SM	112.0	1/94	13.9	/4.3	0.50		<b></b>		↓
-				<b>!</b>				<b></b>		ļ	SPSM	110.6	1//2	9.6	49.7	0.52		<b></b>	<b>↓</b>	<b>├</b> —
				<b></b>			<b></b>	<b> </b>	<b></b>	ļ	SM	113.5	1818	7.7	43.1	0.48		∔	<b>∔</b>	<b>├</b> ──
***	- 20	10		<u> </u>	— <u></u>	<b>!</b>	ļ	<b> </b>	ļ	↓	SP	110.1	1/64	14.7	75.0	0.53		<b>↓</b>	<del> </del>	<b>↓</b>
100	96	12	16	8	6	<b>.</b>	<b> </b>	<b>}</b>		<b> </b>	SW SM	104.8	1679	19,4	86.3	0.61		<b></b>	<b>↓</b>	<del>}</del>
69	60	<del>- 49</del>	24	15	13		<b></b>	<b>↓</b>	<u> </u>	├	SM	105.6	1692	21.7	98.6	0.60		<del> </del> -	<b>├</b> ──	┼
	<b></b>							<b>├</b>	<b>}</b>	<del> </del>	SM	121.0	1938	10.9	75,7	0.39	<b> </b>	<del>-</del>	<del>↓</del>	┼
76	10	0.7		42			L	<b> </b>		<b>↓</b>		94.6	1515	1-10-0	77. 77	0 10		<b></b>	<del>                                     </del>	<b>├</b>
64	12 49	67 37	57 22	15	35 11	<del>                                     </del>	ļ	<b>}</b>	<b>├</b>	<del>}</del>	SM GW-GM	109.1	1515 1748	10.2 6.7	35.2 33.5	0.78	<b> </b>	<b>┼-</b>	+	┼~
5	45		- 27	13			<b>-</b>	<del> </del>	<b> </b>	<b>├</b>	GP-GM	116.2	1862	8.1	48.6	0.45		+	+	┼
				<del>                                     </del>	<b></b> -	<b>-</b>	<del> </del>	┢┈	<del> </del>	╁─	GP-GM	111.8	1791	11.2	59.6	0.45		<del>                                     </del>		1-
				<del> </del>		<del> </del>	<del> </del> -	<del> </del>	<del>                                     </del>	<del> </del>	GP-GM	119.2	1910	14.9	97.3	0.41	<del> </del>	<del>                                     </del>	1	1-
<b>68</b>	51	38	23	15	11		<u> </u>	<del> </del>	<del> </del>	t	GP-GM	117.4	1881	10.4	64.6	0.44	f ——	f	<del>                                     </del>	1-
		<del></del>		<del>  ``</del>	<u> </u>			<del>                                     </del>	<del> </del>	<del>                                     </del>	GP-GM	126.5	2027	10,1	82.2	0.33		<del>                                     </del>	<del>                                     </del>	<del>                                     </del>
		-		<del>                                     </del>	<del> </del>		<del> </del>	<del>                                     </del>	<del> </del>	<del> </del> -	GP GM	117.2	1878	10.6	65.3	0.44	<del>                                     </del>	<del>                                     </del>	<del>                                     </del>	+
				<b>-</b>			ļ	<b>†</b>	ł	ł ··	GP-GM	113.6	1820	14.1	79.1	0.48	<del> </del>	+	+	<del> </del>
				<del> </del>		···-	<u> </u>	<b>†</b> -⋯	<del> </del> -	<del> </del>	GP-GM	127.3	2039	8.3	69.6	0.32	<del> </del> -	<b>†</b>	+	<del>                                     </del>
				<del>                                     </del>			<b>-</b>	<del> </del>	<del> </del>	<del> </del>	GP GM	121.9	1953	10.5	74.3	0.38	<del> </del>	<del>                                     </del>	†	1-
78	66	49	25	13	10	<del> </del>	<b>}</b> -	<del> </del>	<del> </del>	<del> </del>	SW-SM	121.4	1945	10.4	72.7	0.39	<del> </del>	<del>                                     </del>	+	+
70	<del>'''</del>	7.7		<del>'''</del>	, 'U	<b></b>		<del> </del>	<del>                                     </del>	<del>                                     </del>	GP GM	123.4	1977	10.4	80.9	0.39	<b></b>	<del>                                     </del>	<del>1</del>	<del>                                     </del>
							<del> </del>	<del> </del>	<del> </del>	<del> </del>	(,, (,,,,,	135.7	<del>                                     </del>	<del>                                     </del>	1.5.5	<del>  ```''</del>		<del>                                     </del>	1	1-

7.6	45.8	0.45	L	I	<b>I</b> 1	. 1		
9.5	57.1	0.45						
7.2	61.4	0.32						
7.2	29.7	0.66						
9.9	49.5	0.54						
17.6	62.6	0.76						
13.2	49.7	0.72					*	
5.6	31.1	0.48						
8.5	43.1	0.53						
10.0	58.7	0.46						
13.1	63.0	0.56						
10.7	59.6	0.48						
10.3	63.0	0.44						
14.1	66.2	0.58						
13.9	74.3	0.50						
9.6	49.7	0.52						
7.7	43.1	0.48						
14.7	75.0	0.53						
19.4	86.3	0.61						
21.7	98.6	0.60						
10.9	75.7	0.39						
				Ī				
10.2	35.2	0.78						
6.7	33.5	0.54						
8.1	48.6	0.45						
11.2	59.6	0.51						
14.9	97.3	0.41					<u> </u>	
10.4	64.6	0.44						
10.1	82.2	0.33		]				

0.5 - 2.0	0.15 - 0.61			100	65	41	_2
0.5 - 2.0	0.15 - 0.61	-	-+-	100	80	65	5
3.0 - 4.0	0.91 - 1.22	+++	100	91	63	40	3
3.0 4.0	0.51 1.22		100	<del></del>			<u> </u>
0.5 · 2.0	0.15 - 0.61			100	93	85	7
4.0 - 5.0	1.22 - 1.52				100	97	
0.5 - 2.0	0.15 - 0.61				100	94	8
0.5 - 2.0	0.13 0.01			_	1.00		۳
0.5 · 2.0	0.15 - 0.61				100	99	03
4.0 - 5.0	1.22 - 1.52						
9.0 - 10.0	2.74 - 3.05				100	97	8
10.0 - 11.0	3.05 - 3.35						L
0.5 - 2.0	0.15 - 0.61	<del>- }- }</del>		<u> </u>	├	100	-
4.0 - 5.0	1.22 - 1.52						
0.5 - 2.0	0.15 - 0.61				100	98	9
0.5 - 2.0	0.15 - 0.61	-1				100	9
6.0 7.0	1.83 - 2.13				100	92	8
0.5 - 2.0	0.15 - 0.61		100	88	60	41	<u> </u>
0.5 - 2.0	0.15 0.01			400		70	
0.3 - 2.0	0.15 - 0.61	_		100	96	73	_6
0.5 · 2.0	0.15 - 0.61						
0.5 - 2.0	0.15 - 0.61	<del>-   </del>			├		┝

DARD S	IEVE N	0.	PART SIZE	ICLE (mm)		TERBE		uscs	DRY		MOISTURE Content (%)	SATURATION (\$)		MAXI	MUM	OPTIMUM Moisture (\$)	SPECIFIC GRAVITY OF SOLID:	TRIAXIAL
10		SIL	T OR C	LAY				(c)	WEI	GHT ————		(\$)	VOID RATIO	DRY DE	MSITY		EC!	<b>3</b>
40	100	200	.005	.001	4	PL	PI		(pcf)	(kg/m³)		.VS	\$≥	(pcf)	(kg/m³)	5 <b>=</b>	S 61	F
61	47	41						SC										
12	8	6		,	L	ļ		GP-GM		<b>-</b>			<u> </u>	125.5	2011	11.0		
39	31	26			51	33	18	GM		<del> </del>	ļ			115.0	1842	15.0		
21	15	12			3-	33	10_	GP-GM		<del> </del>				113.0	1042	13.0		
	,	<u> </u>					$\vdash$	G. G		<del>                                     </del>					<u> </u>			
56	40	34			41	26	15	SM						118.8	1903	11.8		
6	2	2					L	SW_	L	ļ					<b>├</b>	<u> </u>		<b> </b>
51	33	26					ļ	SM		<del> </del>	<b> </b>				<del>                                     </del>	<del> </del> -		
31	33	20				<u> </u>							<del></del>			<del>                                     </del>		
73	48	39			45	26	19	SC		<u> </u>	<u> </u>					<del>                                     </del>		
								SC		<u> </u>								
21	6	3						SW										
					63	35	28	МН		<b> </b>				<u> </u>		ļ	<b></b>	$\sqcup$
- 66	AC	20				<u> </u>	<u> </u>	CM		<del> </del>	<b>-</b>				<del>                                      </del>	<del> </del> -		
66	46	38				<b>├</b>		SM GP-GM		<del> </del>	<del> </del>	-	<del> </del> -		<del>}</del>	<del>                                     </del>	<del> </del>	
					$\vdash$		_	GI -GIII		<b></b>			<b>-</b>		<del>  -</del> -	<del>                                     </del>	<del>                                     </del>	
73	66	60		- 1				ML								<u> </u>		
										i								
86	54	37						SM										
66	47	34				<u> </u>		SM		ļ			<u> </u>		L	<u> </u>	<u> </u>	
		-						CDCM		ļ	<del></del>	_	ļ		<del></del>	-	<del> </del>	├
12	8	6	-					GP-GM		<del> </del>		<b></b>			<del>                                     </del>	<del>                                     </del>	<del>                                     </del>	$\vdash$
28	16	13				<del>                                     </del>	<u> </u>	SM	<b></b>	<del> </del>	<del>                                     </del>		<u> </u>		<del>                                     </del>	<del>                                     </del>	<del>                                     </del>	
	-				$\vdash$					f					<del>                                     </del>			
100	99	97	47	23	26	16	10	CL									2.72	
							1.									<u> </u>		<b></b>
_90	70	62			59	40	19	МН		<b> </b>	<b> </b>	L			<b></b> -	<del> </del>	<del> </del>	
87	58	46				<u> </u>		SM		<del> </del>	<b> </b>		<u> </u>	ļ	├	+	1	╂╼┯┥
_ 3/	20	70			67	32	35	CH		<del>                                     </del>	<del>                                     </del>	<u> </u>	<del></del>		<del>                                     </del>	<del>                                     </del>	<del>                                     </del>	
					<del></del> -	<del>-</del>	<del>                                     </del>	- <del>``</del> -		t	<b></b>	_			<del>                                     </del>	1	<del>                                     </del>	<b> </b>
99	94	80	19	10			NP	ML		t	t		<del>                                     </del>		<del>                                     </del>			
54	13	6						SP-SM										
																	L	<u> </u>
100	98	92			30	24	6	ML								<b>_</b>	<b>.</b>	—
						<u> </u>				<b></b>	<b> </b>		<b> </b>		<del>↓</del>	+	<del>                                     </del>	<del></del>
		L	Il	L	I	L	I	l		<u> </u>	L	L	L		<u> </u>	<del></del>	L	

				11	I-SITU			C	OMPACTE	0		Ð	9		3		
TERBE ITS (	b)	USCS (c)	DRY WE I	UNIT Ght	ISTURE Ntent (\$)	SATURATION (\$)	VOID Ratio	MAXI DRY DE	MUM NSITY	OPTIMUM Moisture (\$)	SPECIFIC GRAVITY OF SOLIDS	TRIAXIAL (d)	UNCONFINED <b>Con</b> pression	DIRECT	CONSOL IDATION	CHEMICAL	CBR
PL	PI	_	(pcf)	(kg/m³)	<b>3</b> 5	SA	Y R	(pcf)	(kg/m³)	•	2 2 2	1	2 6	00	용	ث	۳
		SC													i		
		GP-GM						125.5	2011	11.0			<u> </u>				*
				<u> </u>			┞—	145.0	4040	45.0		_	<b> </b>				-
<b>3</b> 3	18	GM		<del></del>			├─	115.0	1842	15.0							<del>  </del>
	<u> </u>	GP-GM		<del>                                     </del>	<u> </u>			├	<del> </del>	<u> </u>	<b>-</b>	,	├	<b></b> -			
26	15	SM		├			-	118.8	1903	11.8	<b>-</b>		├	<del>                                     </del>			*
-	- 2	3191		<del> </del>			├─	110.0	1303	11.0	<b></b>			<del> </del>			
	$\vdash$	SW		<del>                                     </del>	<b>-</b>		$\vdash$	<del></del>	<del>                                     </del>	<del>                                     </del>		<u> </u>					
							<del>                                     </del>	<del></del>									
		SM								1							
													<b>↓</b>	<u> </u>	<b></b>		├
26	19	SC								ļ	<b></b>	<u> </u>	ļ	<u> </u>			
		SC		<u> </u>				ļ		↓	<u> </u>	<u> </u>	<b>├</b>	<b> </b>		*	<b>├</b> ──┤
		SW		L	ļ			<u> </u>		<b>↓</b>		Ь—	<del> </del>	<del> </del>	<del>                                     </del>		╁
35	28	МН						<b></b>				—-	<b>├</b>	<b>├</b> ──	<del>                                     </del>		<del>                                     </del>
		SM		├	<b></b>	<u> </u>		<b>├</b>		<del>                                     </del>	<del> </del>	├	┼	<del>                                     </del>	<del>                                     </del>		-
$\vdash$		GP-GM		<del> </del>	<del>                                     </del>	<b></b>	├	<b>├</b> ──	<del> </del> -	┼		┢	+-	<b>-</b>	1 -	*	
		GF-GW		<del>                                     </del>	<del> </del>	<del>                                     </del>			<del>                                     </del>	<del> </del>		$\vdash$	<b>+</b>	t	1		
		ML		<del>                                     </del>		<del>                                     </del>	<del> </del>	1	1	1			1				
				<del> </del>	<b></b>	<del>                                     </del>	<u> </u>	<u> </u>									Ī
		SM								Ť					[		
		SM															<b></b>
												<u> </u>	1	<b>└</b>	<u> </u>	<u> </u>	<b>}</b>
		GP-GM									<b>!</b>	<b>!</b>	<del></del>	<b>├</b>	↓		<b>├</b> ──
				<u> </u>				<u> </u>	<u> </u>	<b>↓</b>		₩	—	<b>}</b>	<del> </del>		╁──
L.,		SM		<u> </u>	L	<u> </u>		<b></b>	↓	——	<b>↓</b>	<b>├</b>	∔	<b>├</b>	+-	├	┼──
10	10			<u> </u>	<u> </u>	<b>.</b>	<u> </u>	<b>├</b> ──	<del>├─</del> ─	<del></del>	2.72	<del> </del>	+	┼	-	*	<del>                                     </del>
16	10	CL		<b></b>	<b> </b>	├──	<del> </del>	<del> </del>	<del> </del>	╁┈	2.72	╂─	+-	<del> </del>	+-	<del>                                     </del>	<del>                                     </del>
40	19	МН	<b></b>	<del>                                     </del>	<del></del>	<del> </del> -		<b></b>	<del>                                     </del>	<del>                                     </del>	t	T	<del>                                     </del>		1		
		,			<del></del>	<del>                                     </del>	$\vdash$	$\vdash$	<del>                                     </del>	1	1	1					
		SM					$\vdash$			1							
32	35	СН						1									<b>↓</b>
									$\Box$						$oxed{oxed}$	ļ	↓
	NP	ML										1		<b>↓</b>	—	<b>├</b>	┼
		SP-SM							<u> </u>	↓	<b>↓</b>	₽-		<b>∔</b>	<b>↓</b>	<del> </del>	┼──
									┷		↓	↓	┼—	₩	┼	ļ	┼
24	6	ML				<u> </u>		ļ	<del> </del>	—	+-	╂—	+-	+	+	+	+-
				<b>├</b> ──	<b></b>	<u> </u>	<b>├</b>	<b>↓</b>	<del> </del>	+	<del> </del>	+-	+-	+	+	+-	+-
	لــــا			Ц	Щ.	<u> </u>		<u> </u>	ــــــــــــــــــــــــــــــــــــــ	ــــــــــــــــــــــــــــــــــــــ		<u> </u>				-	

SUMMARY OF LABORATORY TEST RESULTS
OPERATIONAL BASE SITE
BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - DMO

TABLE 11-5-1 8 OF 7

UBRO NATIONAL IN

AFY-01

		I 045 004	гг	<sub>I</sub>	<del></del>		
<u>B⋅1</u>	0.5 - 2.0	0.15 0.61	<b>├</b> ──		<del></del>		
b-2	8.0 9.0	2.44 2.74	$\vdash$		<del></del>		
			$\longrightarrow$		_}_		
<u>B</u> 1	0.5 2.0	0.15 - 0.61			$\rightarrow$		
b-3	10.0 11.0	3.05 · 3.35					100
<u>b-1</u>	0.5 2.0	0.15 - 0.61					
			lacksquare	—∔—			
B-1	0.5 2.0	0.15 - 0.61	lacksquare				100
b-2	3.0 - 4.0	0.91 1.22					100
b-3	7.0 8.0	2.13 - 2.44					
		L					
b 1	0.5 2.0	0.15 - 0.61					
		L					
B-1	0.5 - 2.0	0.15 - 0.61					100
b-1	0.5 - 2.0	0.15 - 0.61					
b-2	4.0 - 5.0	1.22 - 1.52					
b-1	0.5 2.0	0.15 - 0.61					100
b-2	4.0 - 5.0	1.22 1.52			100	87	59
		<u> </u>					
b-1	0.0 - 1.5	0.00 · 0.46					100
			1		+		
b-1	0.5 · 2.0	0.15 - 0.61					100
		†					
B-1	0.5 - 2.0	0.15 - 0.61			<b>—</b>	100	99
	<u> </u>		1				
b-1	0.5 - 2.0	0.15 - 0.61				100	97
		<del>                                     </del>	1 1				
B-1	0.5 · 2.0	0.15 - 0.61	1				<del>                                     </del>
b-2	5.0 · 6.0	1.52 1.83	<del>                                     </del>	-+			100
b-4	9.0 10.0	2.74 - 3.05	1			100	96
_5-	3.0 10.0	1 2.77 0.00	11.			1,00	

				•
			SM	
			SM	
43	24	19	SC	
		NP	SM	
23	18	5	SM-SC	
22	18	4	CLML	
			SP	
-				
			SM	
			GW-GM	
			ML	
47	19	28	SC	

	<u> </u>			I-SITU			C	OMPACTE	)		<del>D</del>	_ E		8		
ERG (b)	USCS (c)	DRY WEI	UNIT GHT	NOISTURE CONTENT (\$)	SATURATION (\$)	VOID RATIO	MAXI ORY DE	MUM NSITY	OPTIMUM Moisture (\$)	SPECIFIC GRAVITY OF SOLIDS	TRIAXIAL (d)	UNCONFINED COMPRESSION	DIRECT SHEAR	CONSOLIDATION	CHEMICAL	CBR
PI	1I	(pcf)	(kg/m <sup>3</sup> )	¥ 5	AS_	22	(pcf)	(kg/m³)	0 =	200		20	8	-8	-	<u>-</u>
	SM											ļ				
	SM								<u> </u>			<b> </b>	<b> </b>			
			L						<b></b>	L		<b>└</b>				
7	SM-SC		<b>↓</b>	<b> </b>	<del> </del>	├			├	<del> </del> -		<del> </del>				
┿	SM		┼	<del> </del>	<del> </del>				<del> </del>	<del>                                     </del>		1				
9	CL		<del>                                     </del>	<del> </del>	<del> </del>	<del>                                     </del>			<del></del>	<del>                                     </del>						
† <u> </u>			<del>                                     </del>													
	SM			<u> </u>								<u> </u>				
	SC					L			<u> </u>	<u> </u>	<u> </u>	↓	<b>}</b>			
<u> </u>	<del></del> _	_	-	<b></b>	<b>├</b>	<b></b> _	<b></b>	<b></b>	<b> </b>	<del></del>	<b>├</b> ─	<del> </del>			<u> </u>	
8	ML		<b>├</b> ──	<b>├</b> ──		<del> </del>			<del>                                     </del>	<del> </del>	╁	╁╌┈	1		<u> </u>	
╁	SM	<u> </u>	┼	<del> </del>	<del>                                     </del>	<del> </del> -	122.9	1969	11.4	<del>                                     </del>	t —	1				*
+	SM	<del></del>	<del>                                     </del>	<del>                                     </del>	<u> </u>	<del>                                     </del>			<u> </u>	1						
19	SC			<del>                                     </del>							L		↓	<b> </b>	<b></b>	
									L	<b></b>	↓	<del> </del>	<del> </del> -	<del> </del>	├	<b> </b> -
NP	SM		<b>↓</b>	<b></b> _	<b>}</b>	<b> </b>	<b></b> _	<b>├</b>	<b>}</b>	<b></b>		┼──	<del>                                     </del>	<del> </del>	├	
<del>↓</del>	614.60		<del> </del>	<b>├</b> ──		├		<b> </b> -	┼~──	┼	┼	+	<del>                                     </del>	<del>                                     </del>	*	<u> </u>
5	SM-SC		┼	<del> </del>	<del>                                     </del>	<del> </del>	<del> </del>	<del> </del>	┿┈	<del>                                     </del>	+-	†	†			
4	CL·ML		+	<del>                                     </del>	<del>                                     </del>	╁──	<del>├</del> ───					1				
╅	SP		<del>                                     </del>	<del>                                     </del>	<del>                                     </del>		<del>                                     </del>	<u> </u>							*	<b></b>
1				<u> </u>								↓	↓	<b>_</b>	<b>↓</b>	├
	SM							<u> </u>	<b>↓</b>	<b>-</b>	<b>}</b> —	—	<b>↓</b>	}	┼	├
<u> </u>			<b>├</b> ──	<u> </u>	<u> </u>	_	<b>↓</b>	<b>├</b>	<b>├</b>	<del>↓</del>	╂—	+-	┼	├	┼──	┼──
╄	GW-GM		<b>├</b>	<del> </del>	<del> </del>	<b></b> -	<b>├</b>	<del>├</del> ──	┼	+	+	┪—	┼	<b></b>	<del>                                      </del>	
┿	ML		┼	<del> </del> -	<del>                                     </del>	+	<del> </del>	<del> </del>	+	+	+		1			
+	<u>'''</u>		+	<del>                                     </del>	<del>                                     </del>	+-	<del> </del>	<del>                                     </del>	+-	1						
28	SC		T	<del>                                     </del>			<b>†</b>						1	<b></b>	1-	<b>↓</b>
										<del></del>	₩	┿-		┼	+	+
↓	SM		<del> </del>	ļ	<u> </u>	1	125.2	2006	9.8	+	+-	+	+-	<del> </del>	<del> </del>	╅~
∔			—	<del> </del> -	<del> </del>	┼	<del> </del>	╂	+	+	+-	+ -	1-	†	<del>                                     </del>	1
+	SM		+	<del>                                      </del>	┼	+	+	+	+	1	+-	+	_	1		
8	ML.		+	<del> </del>	┼──	┪	112.6	1804	16.4	<del></del>	1	$\top$				*
18	SC		<del>                                     </del>	<del>                                     </del>	<del>                                     </del>	+	+	1	1							
<u> </u>	SP-SM										L	<u> </u>		<del> </del>	<b>-</b>	<del>↓</del> —
										<del> </del>	╁	—	┼	+	┼	╂
	SC						I	↓	<del></del>	<del>-</del>	4—	+-	+-	+	+	+
₩-	60.514	<del></del>	┿	<del>                                     </del>	——	<del> </del>	<b>↓</b>	<del> </del>	+	<del>-</del>	+-	+-	+-	╅	+-	<u>t                                     </u>
Щ_	SP-SM			<del></del>	<b>ــــ</b> ــــــــــــــــــــــــــــــــ	Щ.,		<u></u>								

SUMMARY OF LABORATORY TEST RESULTS
OPERATIONAL BASE SITE
BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - SMO

14 BLE 11-5-1 6 OF 7

UBRO NATIONAL INC.

AFY-01

2

3

 <del>,</del>					
 					$\vdash$
 				L	
	_				L ]
 				ļ	
 				L	
 				<u> </u>	
 				<b>_</b>	L
 	L		L	L	
			I	l	l
		_			
 					-
 <del></del>			$\vdash$	<b></b> -	
 			$\vdash \vdash$	├	
 <del></del>				<b>├</b>	-
 				ļ	
	<del>                                     </del>				<del>                                     </del>
 			├	<del> </del>	$\vdash$
 			<del></del>	├	<del> </del>
 	ļ		L	L	
				L	
			<u> </u>		

	_		_	-		
	L.				i i	
<b></b>		$\vdash$		-	<del>                                     </del>	
		$\vdash$				
	<b></b>	<b>-</b>			<del></del>	—
ļ		<u> </u>				
		$\vdash$				
	L					
				-		
	-		_			
						<u></u>
		$\vdash$				
				l		

					I-SITU			C	OMPACTE		ا _ ا	(d.)	요를			_ ,	l
TERBE IITS (	RG b)	USCS (c)	ORY Wei	UNIT GHT	MOISTURE Content (\$)	SATURATION (\$)	VOID RATIO	MAXI DRY DE	MUM NSITY	OPTIMUM MOISTURE (\$)	SPECIFIC GRAVITY OF SOLIDS	TRIAXIAL (d)	UNCONFINED COMPRESSION	DIRECT	CONSOLIDATION	CHEMICAL	CBR
PL	PI		(pcf)	(kg/m <sup>3</sup> )	물 8 -	SA	25	(pcf)	(kg/m³)	5 <b>3</b>	2 2 2		22	S	- 물	ات-	
		SM	<u> </u>					125.0	2003	10.9							*
			<del>                                     </del>										<b>}</b>		<del></del>		
		SC							Ĺ				<b>↓</b>				
							<u> </u>		1000	1			<b>├</b> ──				*
		SM		Ļ			<b>├</b>	122.5	1962	11.5	——-	<del> </del>	-	<del>                                     </del>			
	Ĺ	<b>↓</b>	<b></b>	Ļ			<b>├</b> ─	<u> </u>	<del> </del>	<del>├</del> ──	<del></del>		╂	<del></del> -			
	<u> </u>	<del>                                     </del>	<b></b>	<del> </del>	<b> </b>		├	<del> </del>	├	<del> </del>	<del>                                     </del>	<del>                                     </del>	<del>                                     </del>				
		<b>}</b>	<b> </b>	<del>}</del>	<b>├</b> ──		<b>├</b> ──	<del></del>		1	<b></b>		1				
		<del> </del>	<del> </del>	├	<del> </del>	<del> </del> -	<del> </del>	<del> </del>		†	1						<u> </u>
		<del>                                     </del>	<del> </del>	<del>                                     </del>	<del>                                     </del>	<u> </u>	<del>                                     </del>	<del>                                     </del>		1							
			<del> </del>	<del>                                     </del>	<del>                                     </del>								——	<b>├</b> ──	<b>├</b> ──	<b></b> _	├
									<u> </u>	<b>↓</b>	<b>↓</b>	<b> </b>	—	}	}	├	├─
							$L_{-}$		<u> </u>	↓	<b>├</b>	}	——	<b>├</b> ──	├		$\vdash \frown$
			L	<u> </u>		<u> </u>	<u> </u>	<b>└</b>	↓	<b>-</b>	<b>├</b>	<b>├</b> ─	┼	┼──	<b>├</b> ──	<del>                                     </del>	$\vdash$
			L	<b>↓</b> _	<u> </u>	L	ļ	<b>↓</b>	ļ	<del>↓</del>	<del>↓</del>	+-	+-	+	<del> </del>		$\vdash$
	L	<b></b>	<b></b> _	<b></b>	<b></b> -	ļ	<b>↓</b>	<b></b>	<del> </del>	<del> </del>	<del>                                     </del>	<del> </del>	+	<del>                                     </del>	†	<del>                                     </del>	
	├	<del>├</del> ─~	<del> </del>	├	<del>├</del>	<del> </del> -	<b>├</b>	┼	┼	<del> </del>	+-	<del>                                     </del>	<del>                                     </del>				
	<del> </del>	<del> </del>	<b>├</b> ──	├	├	<b></b> -	┼─	┼	+	+	+	$T^{-}$	1			<u> </u>	<u> </u>
<b>-</b>	┝─	<del> </del>		╁	╆──	├	+	+	+	1					<u> </u>	<b></b>	↓
			<del> </del>	<del> </del>	<del> </del>	<del></del>	<del> </del>	<del>                                     </del>							1	1	↓
	<del>                                     </del>	<del>                                     </del>	<del>                                     </del>	<del> </del> -	<del>                                     </del>	<del>                                     </del>	+	+	1					<del>                                     </del>	↓	<b>├</b> ──	┼
			1				T					↓	Д—	—	╂	<b>↓</b>	┼─
											<u> </u>	—∔	—	┼	<del>}</del>	<del>}                                    </del>	┼─
									<b>_</b>	→—	<del>                                     </del>	+-		┼	+	+-	+-
	L	<u> </u>		<b></b> _	<b></b>		<u> </u>	<b></b>	┿	<b>_</b>		+-	+	+	+	<del>                                     </del>	+-
<u> </u>	L		ļ	↓	ļ	Ļ	↓	╄	┼──	+	+	+-		+ -	<del>                                     </del>	1	$\top$
<u> </u>	<b>├</b>	<b>├</b>	<b>_</b>	<del> </del>	<del> </del>	<b></b>	↓	<del></del>	+	+	+	+-	_	+	1		
<del> </del>	├	<b>├</b>	<b>├</b> ──	+	<del> </del>	<del> </del>	+	╂	+	+	+	1					
<b>—</b>	-	<del>                                     </del>	<del> </del>	<del>                                     </del>	<del> </del> -	┼	+	+	+-	+						<del> </del>	Д—
	<del></del>	<del>                                     </del>	<del>                                     </del>	1	<del>                                     </del>	<del>                                     </del>	+-	1							4-	<del> </del>	+-
			1	1	1	<del>                                     </del>	+					ᆚ_		-	→—-	<b>↓</b> —	<b>→</b> —
		İ					1					4-		+	+-	+	+-
												-			+-	+-	+-
										$\bot$	—	╂		+-	+	+-	+-
														+	+	+-	+
		<b></b>	<b></b>	<b>↓</b>	<u> </u>					——		+-			+-	+-	+-
	<u> </u>	<b>!</b>	L	ـــــــ	<u> </u>	<b>_</b>	1_				_}-	+-		+	+-	1	1
	<u> </u>	<del> </del>	<b></b>	<b>├</b>	<b>↓</b>	<del>   </del>		<del></del>	<del></del>		+	-	+	+	$\top$	1	$I^-$
-	<b>_</b>	<b> </b>	<b>_</b>	+-	<del>                                     </del>	┼			+	+	+-	_					$oldsymbol{\perp}$
<b>—</b>		<u> </u>	<u> </u>		┸												

SUMMARY OF LABORATORY TEST RESULTS
OPERATIONAL BASE SITE
BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - 9MO

TABLE 11-5-1 7 OF 7

UGRO NATIONAL INC.

AFY-01

2

SYMBOL	BORING NO.	SAMPLE NO.	SAMPLE I	NTERVAL	SOIL TYPE	–	DRY D	ENSITY	MOISTURE CONTENT	CONF PRESSI	INING JRE(03)	MAX DEVI STRESI
i			FEET	METERS	L	TEST	pcf	kg/m 3	(%)	ksf	kN/m <sup>2</sup>	ksf
0	BL-B-3	P-12	35.1 - 35.7	10.70 - 10.88	ML	CD	75.0	1202	17.6	2.0	96	10.5
		P-12	36.3 - 37.0	11.06 - 11.28	ML	CD	79.6	1275	13.2	8.1	388	30.9
	BL·B·4	P-5	14.0 · 14.6	4.27 - 4.45	ML	CD	81.5	1306	16.8	2.0	96	6.3
		P-5	14.6 · 15.2 4.45 · 4.63		ML	CD	81.6	1307	18.7	5.0	239	13.0
Δ	BL-B-4	P-8	29.5 - 30.3	8.99 - 9.24	SM	CD	90.0	1442	13.0	1.4	67	7.2
		P-8	30.3 - 31.0	9.24 - 9.45	SM	CD	100.5	1610	10.3	6.0	287	28.2
▽	BL-B7	P-10	35.0 35.7	10.67 - 10.88	CL	CD	94.5	1514	20.8	1.7	81	6.2
		P-10	35.7 - 36.3	10.88 - 11.06	CL	CD	106.7	1709	12.9	6.0	287	19.7
												<del></del>
•										_		
			<del></del>								$\vdash$	
											├┤	

NOTES: 
$$p = \frac{\sigma_1 + \sigma_3}{2}$$
,  $q = \frac{\sigma_1 \cdot \sigma_3}{2}$   
 $c = \frac{a}{\cos \phi}$ ,  $\phi = \sin^{-1}(\tan \alpha)$ 

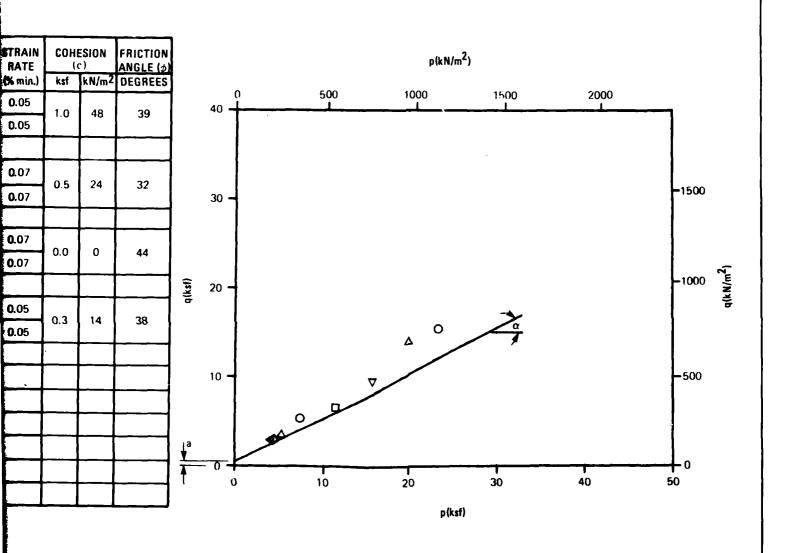
1 E		IRE(CZ)	DEVI STRESS	MUM ATOR (O <sub>1</sub> -O <sub>3</sub> )	STRAIN RATE	(6	-)	FRICTION ANGLE (4)			p(kN/m²)
	ksf	kN/m <sup>2</sup>	ksf	kN/m <sup>2</sup>	(% min.)	ksf	kN/m²	DEGREES			0 500 1000 1500 2000
	2.0	96	10.5	503	0.05	1.0	48	39	Ì	40 -	0 500 1000 1500 2000
	8.1	388	30.9	1479	0.05	1.0	70	39	1		
	2.0	96	6.3	302	0.07	0.5	24	32	1		
	5.0	239	13.0	622	0.07	0.5	24	32	1	30 -	-
	1.4	61	7.2	345	0.07	0.0	0	44			
	6.0	287	28.2	1350	0.07	0.0	Ů	44			
									q(ksf)	20 -	
	1.7	81	6.2	297	0.05		14	38	6		<b>→</b>
	6.0	287	19.7	943	0.05	0.3	) '*	36			Δ 0 σ
										10 -	▼ /
П											
П											
									↓a		
									=	<del>- 0 -</del>	0 10 20 30 40
									'		
Г									•		p(ksf)

SUMMARY OF TRIAXIAL! TEST RESUL' OPERATIONAL BA BERYL, UTA

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORG

UGRO NATIO

2\_



SUMMARY OF TRIAXIAL COMPRESSION TEST RESULTS OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE — BMO

FIGURE ∏-5-1

UGRO NATIONAL, INC.

\_2\_

3

---

_	_		_	-			_	_					_	, ···	_	_	_	<u> </u>		 		_	_		,	_
/TH813H	DIAMETER	2.40	2.10	2.10	2.10	2.10	2.40	2.40	2.10	2.00	2.10	2.10	2.00	2.40	2.10	2.10	2.10	2.40	2.10	ļ						
DEGREE OF SATURATION	(%)	23.7	58.3	39.8	39.0	59.5	46.6	36.4	80.9	52.6	57.7	60.3	36.5	34.3	59.2	55.9	45.8	61.4	49.7							
MO I STURE Content	(%)	5.2	17.1	15.5	10.1	14.2	8.0	7.7	15.6	15.9	12.9	13.7	11.1	7.3	14.3	15.1	14.4	12.4	13.2							
DENSITY.	kg/m3	1693	1504	1314	1591	1642	1846	1717	1777	1488	1680	1674	1482	1713	1616	1562	1458	1746	1572							
ORY DE	pcf	105.7	93.9	82.0	99.3	102.5	115.2	107.2	110.9	92.9	104.9	104.5	92.5	106.9	100.9	97.5	91.0	109.0	98.1					i		
KFINED Trength	kN/m2	38	278	29	177	34	115	206	62	10	24	397	187	98	11	11	34	661	24							
UNCONFINED COMP. STRENGT	ks f	8.0	5.8	1,4	3.7	0.7	2.4	4.3	1.3	0.2	0.5	8.3	3.9	1.8	1.6	1.6	0.7	13.8	0.5							
2011	IYPE	WS	CL	SC	ML	SM	SM	SM	SP-SM	SM	SM	SM	SM	WS	SM	SM	SM	SM	SM							
INTERVAL	METERS	3.41 - 3.66	4.33 - 4.57	0.76 - 1.43	6.07 - 6.31	14.57 - 14.84	15.00 - 15.24	9.20 - 9.45	3.35 - 3.60	9.14 - 9.39	9.39 - 9.60	15.15 - 15.39	1.43 - 1.68	4.63 - 4.88	7.65 - 7.86	13.93 - 14.17	15.12 - 15.33	21.40 - 21.64	2.35 - 2.59							
SAMPLE 1	FEET	11.2 - 12.0	14.2 - 15.0	2.5 - 4.7	19.9 - 20.7	47.8 - 48.7	49.2 - 50.0	30.2 - 31.0	11.0 - 11.8	30.0 - 30.8	30.8 - 31.5	49.7 - 50.5	4.7 - 5.5	15.2 - 16.0	25.1 - 25.8	45.7 - 46.5	49.6 - 50.3	70.2 - 71.0	7.7 - 8.5							
SAMPLE		D-9	P-8	P-2	P-6	P-11	D-12	D-8	P-5	P.9	P-9	P-13	D-4	6-O	P-11	P-15	P.16	D-18	P-3							
22	NU.	BL-8-2	BL-8-3	BL-8-4				BL-B-6	BL-B-7				BL-B-8						BL-8-10							

SUMMARY OF UNCONFINED COMPRESSION TEST RESULTS OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BNG

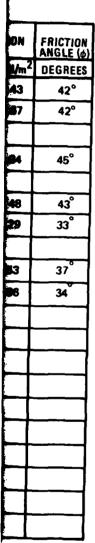
TABLE Ⅲ-5-2

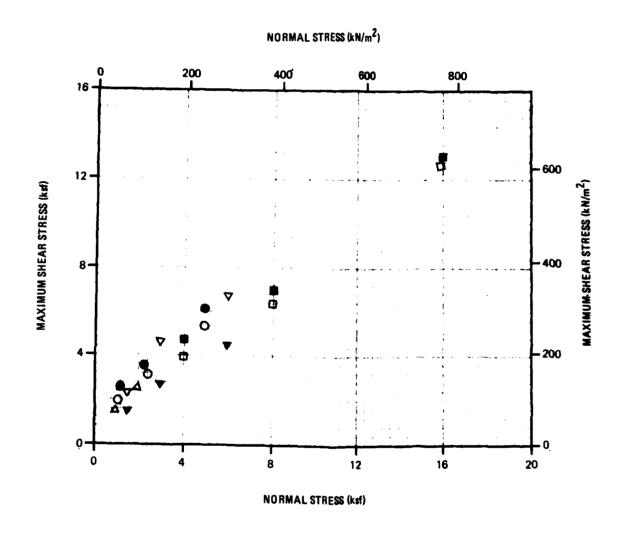
UGRO NATIONAL, INC.

SYMBOL	BORING NO.	SAMPLE NO.	SAMPLE I	NTERVAL	SOIL	TYPE OF	DRY D	ENSITY	MOISTURE CONTENT	СОН	ESION (c)	FRICTM
			FEET	METERS		TEST	pcf	kg/m <sup>3</sup>	(%)	ksf	kN/m <sup>2</sup>	DEGRE
0	BL-B-6	D-7	24.1 - 24.9	7.35 - 7.53	SP-SM	CD	114.2	1829	13.1	0.9	43	429
•									18.3	1.4	67	420
Δ	BL-B-7	D-7	19.2 - 20.0	5.85 - 6.10	SP-SM	CD	114.4	1833	9.4	0.5	24	45°
▽	BL-8-8	D-12	30.2 - 31.0	9.20 - 9.45	SP-SM	CD	113.8	1823	5.8	1.0	48	43°
▼									22.7	0.6	29	33°
0	BL-B-8	P-20	92.1 - 93.0	28.07 - 28.35	SP-SM	CD	91.4	1464	18.2	1.1	53	37
•									19.4	2.0	96	34
								<u></u> _			<del></del>	<u> </u>
											<u> </u>	
					├						-	
				<b></b>	<u> </u>			<b> </b>	<b> </b>		↓	<u> </u>

O, △, ▽ □ - Tested at natural moisture content

●, ▼, ■ - Tested in soaked condition





SUMMARY OF DIRECT SHEAR TEST RESULTS
OPERATIONAL BASE SITE
BERYL, UTAH

MX SITING INVESTIGATION

1-5-2 1 OF 5

DEPARTMENT OF THE AIR FORCE -- BMG

UGRO NATIONAL, INC

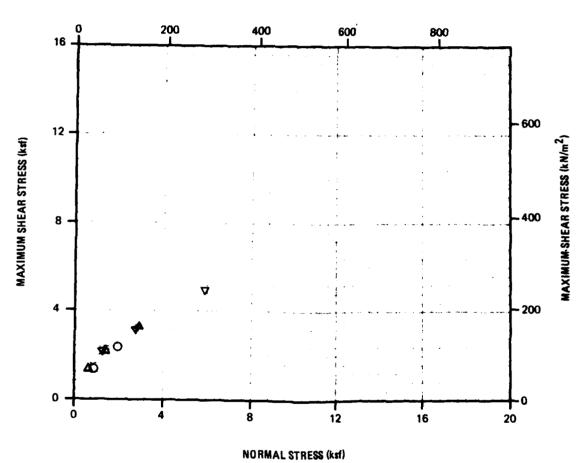
2---

SYMBOL	BORING	SAMPLE	SAMPLE I	NTERVAL		TYPE	DRY D	ENSITY	MOISTURE Content	сон	SION	FRICTION ANGLE (4)
	NO.	NO.	FEET	METERS	TYPE	OF TEST	pcf	kg/m <sup>3</sup>	(%)			DEGREES
0	BL-B-1	D-4	10.2 - 11.0	3.11 - 3.35	SW- SM	CD	104.3	1671	9.1	0.1	5	48°
Δ	BL-B-2	D-10	15.2 - 16.0	4.63 - 4.88	SW- SM	CD	110.1	1764	8.7	0.8	38	39°
<b>V</b>	BL-B-3	P-11	29.3 - 29.9	8.93 - 9.11	SW- SM	CD	113.0	1810	11.5	0.9	43	34
										ļ		

O,  $\Delta$ ,  $\nabla$  — Tested at natural moisture content







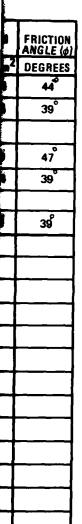
SUMMARY OF DIRECT SHEAR TEST RESULTS **OPERATIONAL BASE SITE** BERYL, UTAH

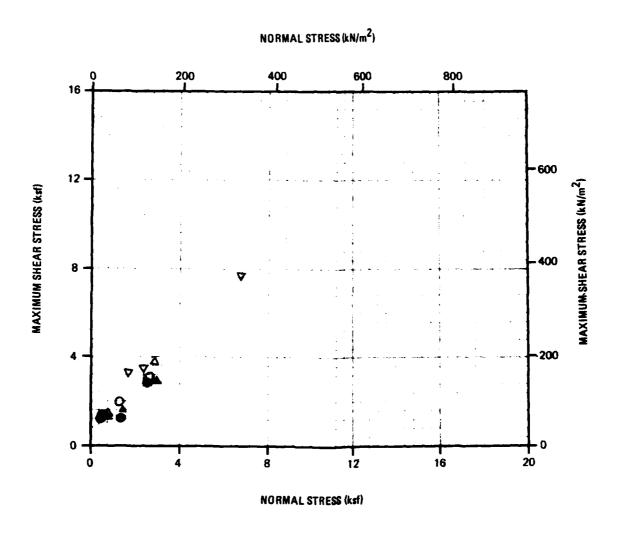
MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - BMO FIGURE 11-5-2 2 OF 5

SYMBOL	BORING NO.	SAMPLE NO.	SAMPLE II	NTERVAL	SOIL TYPE		DRY D		MOISTURE CONTENT	(	ESION c)	FRICTION ANGLE
_			FEET	METERS		TEST	pcf	kg/m <sup>3</sup>	(%)	ksf	kN/m <sup>2</sup>	DEGREE
0	BL-B-1	D-3	6.7 - 7.5	2.04 - 2.29	SM	CD	100,7	1613	11.5	0.5	24	44
•									21.0	0.3	14	39
Δ	BL-B-1	D-5	15.2 - 16.0	4.63 - 4.88	SM	CD	96.9	1552	13.3	0.4	19	47
<b>A</b>									25.6	0.5	24	39
ᢦ	BL·B-2	D-2	1.7 - 2.5	0.52 - 0.76	SM	CD	106.6	1708	6.0	1.9	91	38
					_							
			(	CONTINUED O	N NEX	T PAG	Ε					
											ļ	
			<del> </del>			-						
				ļ								
				ļ	<del> </del>				 		_	

O, △, ▽ - Tested at natural moisture content

● A ▼ - Tested in soaked condition





SUMMARY OF DIRECT SHEAR TEST RESULTS
OPERATIONAL BASE SITE
BERYL, UTAH

MX SITING INVESTIGATION

11-5-2

DEPARTMENT OF THE AIR FORCE - BMO

3 OF5

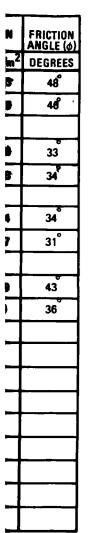
UBRO NATIONAL, INC.

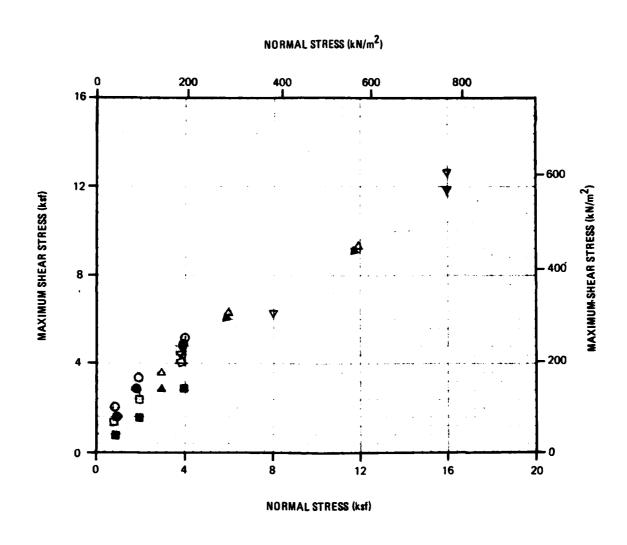
2

		· · · · · ·							, ,			
SYMBOL	BORING NO.	SAMPLE NO.	SAMPLE I	NTERVAL	SOIL TYPE	TYPE OF	DRY D		MOISTURE CONTENT		ESION c)	FRICTION ANGLE (6)
			FEET	METERS		TEST	pcf	kg/m <sup>3</sup>	(%)	ksf	kN/m <sup>2</sup>	
0	BL-B-8	P-10	20.4 - 21.2	6.22 - 6.46	SM	CD	108.1	1732	12.3	0.9	43	48
•									17.8	0.6	29	48° ′
Δ	BL-B-8	D-17	60.2 - 61.0	18.35 - 18.59	SM	CD	107.0	1714	14.9	1.8	86	33
									20.4	1.1	53	34
			·	<u> </u>				<b></b>		<u> </u>		
▽	BL-8-8	P-19	80.1 - 80.8	24.41 - 24.63	SM	CD	85.4	1368	16.1	0.7	34	34
▼									25.6	1.2	57	31°
ם	BL-B-11	D-1	0.7 - 1.5	0.21 - 0.46	SM	CD	94.6	1515	10.2	0.4	19	43
	<u> </u>				L				26.5	0.0	0	36
				<b></b>				<b></b>				
					<b> </b>			ļ				
								<b></b>				
	ļ				ļ							
				<b> </b>							<b>_</b>	
					<b> </b>			<b> </b>				
					<u> </u>			<b></b>			<u> </u>	
				ļ	ļ			<u> </u>			ļ	
					l			í	l			

O,  $\Delta$ ,  $\nabla$ ,  $\square$  — Tested at natural moisture content

●, ▲, ▼, ■ ~ Tested in soaked condition





SUMMARY OF DIRECT SHEAR TEST RESULTS
OPERATIONAL BASE SITE
BERYL, UTAH

MX SITING INVESTIGATION

FIGURE II-5-2

DEPARTMENT OF THE AIR FORCE - BMO

4 OF 5

JORO NATIONAL, INC.

2

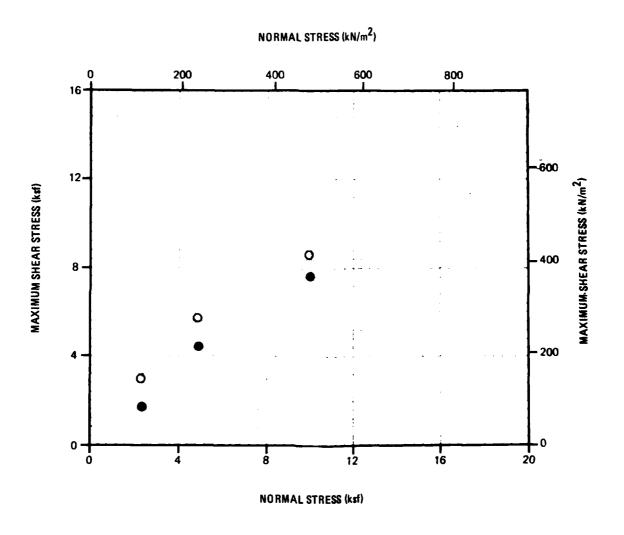
(0)

SYMBOL	BORING NO.	SAMPLE NO.	SAMPLE II	NTERVAL	SOIL TYPE	TYPE OF	DRY D		MOISTURE CONTENT	СОН	SION c)	FRICTION ANGLE (ø)
	L		FEET	METERS		TEST	pcf	kg/m <sup>3</sup>	(%)	ksf	kN/m <sup>2</sup>	DEGREES
0	BL-B-3	P-15	49.2 - 50.0	15.00 - 15.24	ML	CD	97.1	1556	21.9	0.9	43	40°
•									20.4	0.0	0	39
					<b> </b>	_					-	
					_	H						
-						$\vdash$						
<u> </u>				<u> </u>				1			}	
				<u> </u>	<del>                                     </del>	Н		<b> </b>			-	
				<u>-</u>				<del>                                     </del>			1	
				<del>                                     </del>				<del></del>	-		<del>                                     </del>	
			_									
			*									
			_									
•												

O - Tested at natural moisture content

Tested in soaked condition





SUMMARY OF DIRECT SHEAR TEST RESULTS
OPERATIONAL BASE SITE
BERYL, UTAH

MX SITING INVESTIGATION

FIGURE II-5-2

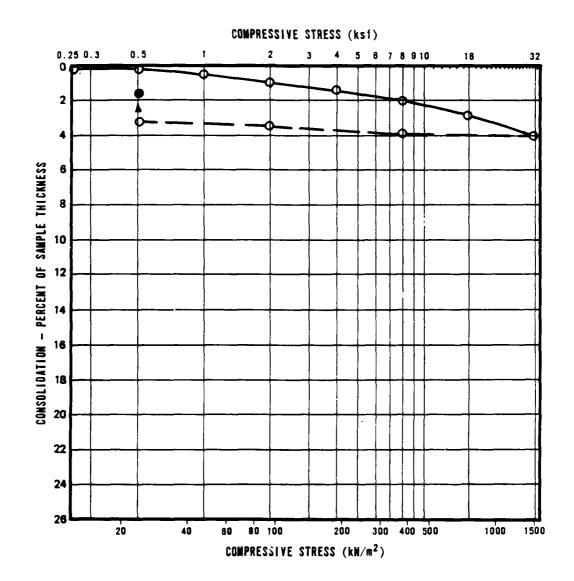
DEPARTMENT OF THE AIR FORCE - BMO

5 OF 5

<u>ugro mational, inc.</u>

2

1



SYMBOL	BORING NG.	SAMPLE NO.	SAMPLE	INTERVAL	SOIL TYPE		TIAL ENSITY	INITIAL MOISTURE CONTENT	INITIAL VOID RATIO	INITIAL DEGREE OF SATURATION
	L	l [	FEET	METERS	]	pcf	kg/m3	(%)	KALIU	(%)
0,•	BL-B-2	D-19	70.2 - 71.0	21.40 - 21.64	SM	109.3	1751	11.9	0.54	59.5
					<u></u>	<u> </u>	<u> </u>			
							1			

AT FIELD MOISTURE

AFTER ADDITION OF WATER

COMPRESSION

- - REBOUND

CONSOLIDATION TEST RESULTS
OPERATIONAL BASE SITE
BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMO

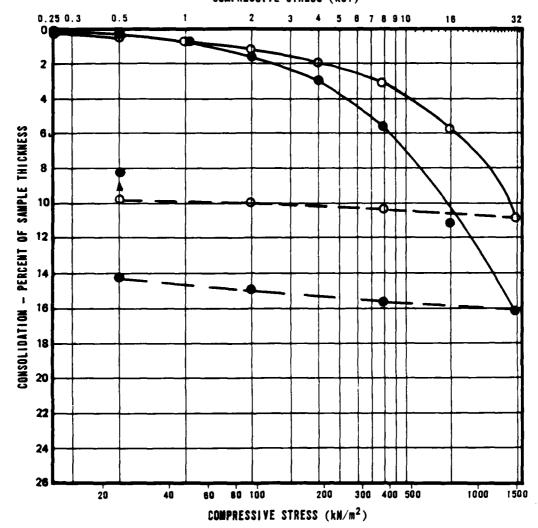
FIGURE 1.5-3

UGRO NATIONAL

USAF-09

20 MAR 81





SYMBOL	BORING NO.	SAMPLE No.	SAMPLE	NTERVAL	SOIL TYPE		TIAL Ensity	INITIAL MOISTURE CONTENT		INITIAL DEGREE OF SATURATION
			FEET METERS pof	kg/m <sup>3</sup>	(%)	טוואא	(%)			
0,0	BL-B-3	P-8	13.5 - 14.2	4.11 - 4.33	CL	94.6	1515	17.1	0.78	59.2
⊕,⊕	BL-B-3	P-8	13.5 - 14.2	4.11 - 4.33	CL	90.9	1456	16.7	0.85	53.0
						,				

.O AT FIELD MGISTURE

AFTER ADDITION OF WATER

\_\_\_\_ COMPRESSION

- - REBOUND

CONSOLIDATION TEST RESULTS
OPERATIONAL BASE SITE
BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - 800

11-5-3 2 OF 9

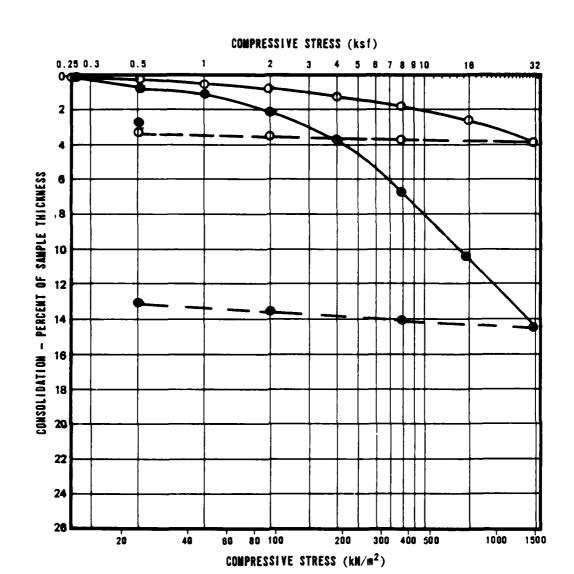
UGRO NATIONAL INC.

USAF-08

94

20 MAR 81

FN-TR-45



SYMBOL	BORING NO.	SAMPLE No.	SAMPLE	INTERVAL	SOIL		TIAL Ensity	INITIAL MOISTURE CONTENT	INITIAL VOID RATIO	INITIAL DEGREE OF SATURATION
			FEET	METERS		pcf	kg/m <sup>3</sup>	(%)	RATIO	(%)
0.0	BL-B-4	P-6	19.2 - 19.9	5.86 - 6.07	ML	99.2	1589	9.7	0.70	37.4
⊕,⊕	BL-B-4	P-6	19.2 - 19.9	5.85 - 6.07	ML	97.1	1556	14.2	0.74	51.8

. AT FIELD MOISTURE

AFTER ADDITION OF WATER

---- COMPRESSION

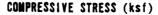
- - REBOUND

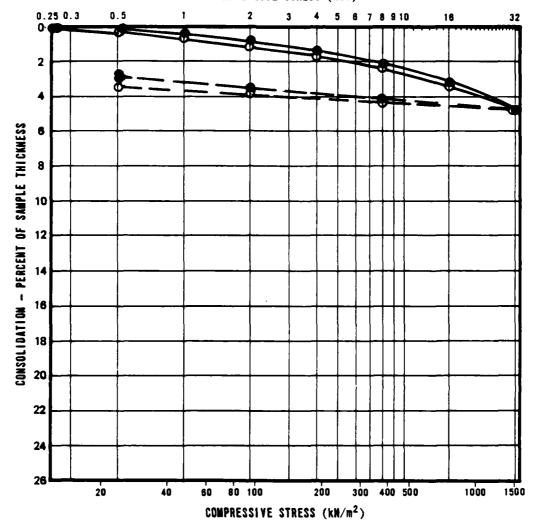
CONSOLIDATION TEST RESULTS
OPERATIONAL BASE SITE
BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - 8MG

II-5-3

UGRO NATIONAL, INC.





SYMBOL	BORING NO.	SAMPLE No.	SAMPLE	INTERVAL	SOIL	INIT Dry Di	TIAL ENSITY	INITIAL Moisture Content	INITIAL VOID RATIO	INITIAL DEGREE OF SATURATION
			FEET	METERS		pef	kg/m <sup>3</sup>	(%)	MAIIU	(%)
0,0	BL-B-4	P-9	35.0 - 35,7	10.67 - 10.88	SM	103.1	1652	19.5	0.57	88.6
⊕,⊕	BL-B-4	P-9	35.7 - 35.8	10.88 - 10.91	SM	103.7	1661	16.8	0.56	77.7

AT FIELD MOISTURE

AFTER ADDITION OF WATER

\_\_\_\_\_ COMPRESSION

- - REBOUND

CONSOLIDATION TEST RESULTS
- OPERATIONAL BASE SITE
BERYL, UTAH

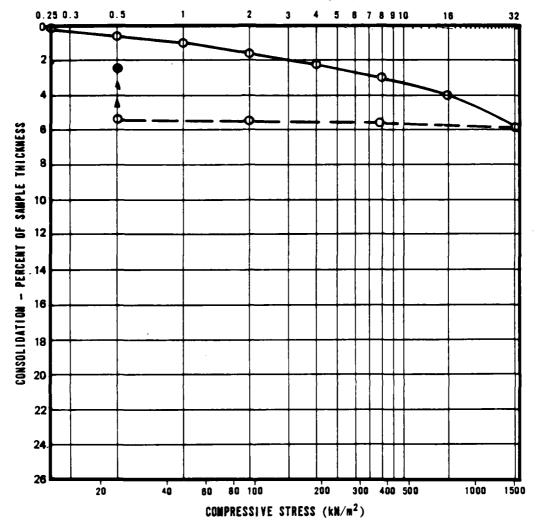
MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - 8M0

FIGURE 11-5-3 4 of 9

TUGRO NATIONAL, INC.

20 MAR 81





SYMBOL	BORING No. BL-8-6	SAMPLE No.	SAMPLE	INTERVAL	SOIL TYPE		TIAL Ensity	INITIAL MOISTURE CONTENT	INITIAL VOID RATIO	INITIAL DEGREE OF SATURATION
			FEET	METERS	1111	pef	kg/m <sup>3</sup>		KATIU	(%)
0, ●	BL-B-6	D-8	30.2 - 31.0	9.20 - 9.45	SM	101.8	1631	12.1	0.66	49.5

AT FIELD MOISTURE

AFTER ADDITION OF WATER

\_\_\_\_ COMPRESSION

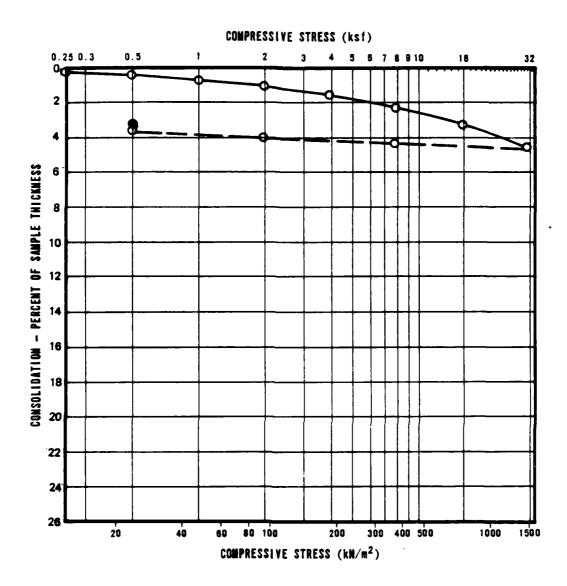
- - REBOUND

CONSOLIDATION TEST RESULTS
OPERATIONAL BASE SITE
BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMO

FIGURE 11-5-3 5 OF.9

UGRO NATIONAL, INC.



SYMBOL	BORING NO.	SAMPLE No.	SAMPLE INTERVAL		SOIL TYPE	INITIAL DRY DENSITY		INITIAL MOISTURE CONTENT	INITIAL VOID RATIO	INITIAL DEGREE OF SATURATION
			FEET	METERS		pe1	kg/m <sup>3</sup>		KALIU	(%)
0,0	BL-8-7	P-10	36.3 - <b>36</b> .6	11.06 - 11.15	CL	111.1	1780	11.3	0.52	58.7

. O AT FIELD MOISTURE

AFTER ADDITION OF WATER

COMPRESSION

- - REBOUND

CONSOLIDATION TEST RESULTS
OPERATIONAL BASE SITE
BERYL, UTAH

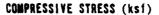
WX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - 900

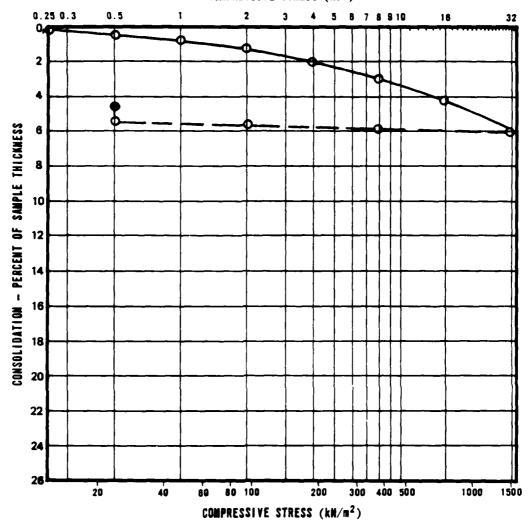
FIGURE II-5-3-6 OF 9

UGRO NA

USAF-09

20 MAR 81





SYMBOL	BORING No.	SAMPLE NO.	SAMPLE	INTERVAL	SOIL		TIAL Ensity	INITIAL MOISTURE CONTENT		INITIAL DEGREE OF SATURATION
<u> </u>			FEET	METERS	1	pcf	kg/m3	(%)	KAIIU	(%)
0.	BL-B-8	P-11	25.8 - 26.2	7.86 - 7.98	SM	101.9	1632	14.7	0.62	62.8
<b></b>		<del>  </del>			<del> </del>			<b></b>		

O AT FIELD MOISTURE

AFTER ADDITION OF WATER

---- COMPRESSION

**— —** REBOUND

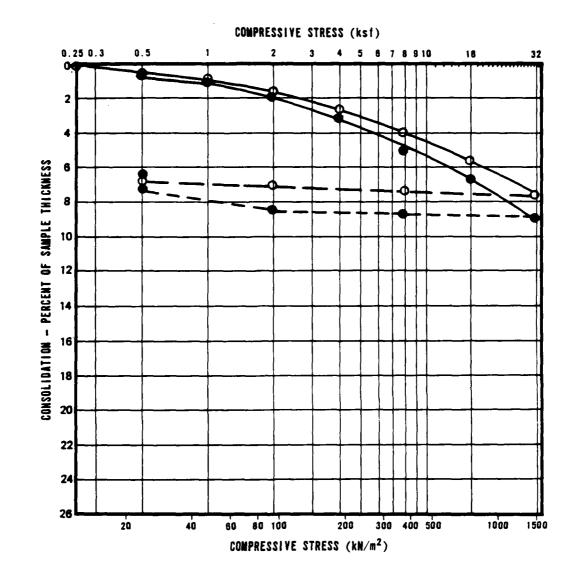
CONSOLIDATION TEST RESULTS
OPERATIONAL BASE SITE
BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMO

II-5-3

UGRO NATIONAL INC

20 MAR 81



SYMBOL	BORING No.	SAMPLE No.	SAMPLE	INTERVAL	SOIL TYPE		TIAL Ensity	INITIAL MOISTURE CONTENT		INITIAL DEGREE OF SATURATION
			FEET	METERS		pef	kg/m <sup>3</sup>	(%)	RATIO	(%)
0, •	BL-8-8	P-16	50.3 - 50.9	15.33 - 15.51	SM	98.3	1575	11.5	0.71	43.7
⊕, ●	BL-B-8	P-16	50.3 - 50.9	15.33 - 15.51	SM	102.1	1636	10.1	0.65	42.0

AT FIELD MOISTURE

AFTER ADDITION OF WATER

\_\_\_\_ COMPRESSION

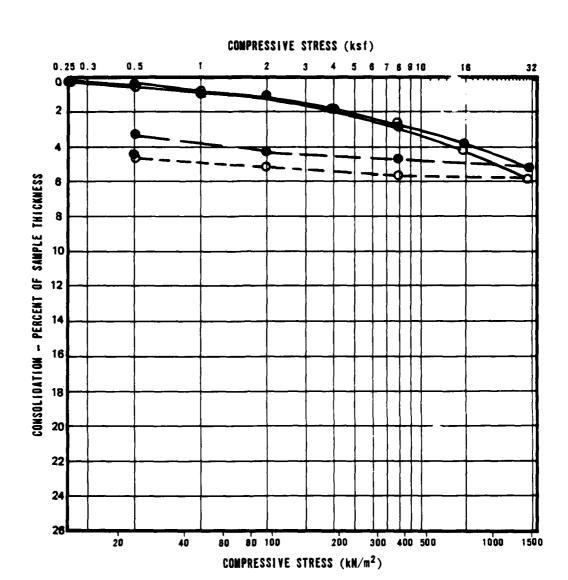
--- REBOUND

CONSOLIDATION TEST RESULTS
OPERATIONAL BASE SITE
BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMO

FIGURE 11-5-3 8 OF 9

UGRO NATIONAL, INC.



SYMBO	BORING NO.	SAMPLE NO.	SAMPLE	INTERVAL	SOIL Type		TIAL Ensity	INITIAL MOISTURE CONTENT	INITIAL VOID RATIO	INITIAL DEGREE OF SATURATION
			FEET	METERS		pcf	kg/m <sup>3</sup>	(%)	KALIU	(%)
0,0	BL-B-8	P-19	80.1 - 80.8	24.41 - 24.63	SM	100.3	1607	19.2	0.68	76.2
⊕,⊕	BL-B-8	P-19	80.1 - 80.8	24.41 - 24.63	SM	103.2	1653	17.7	0.63	75.9
							I -			

• O AT FIELD MOISTURE

AFTER ADDITION OF WATER

\_\_\_\_ COMPRESSION

\_ \_ REBOUND

CONSOLIDATION TEST RESULTS
OPERATIONAL BASE SITE
BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMG

FIGURE II-5-3 9 OF 9

TUBRO NATIONAL INC.

USAF-09

20 MAR 81

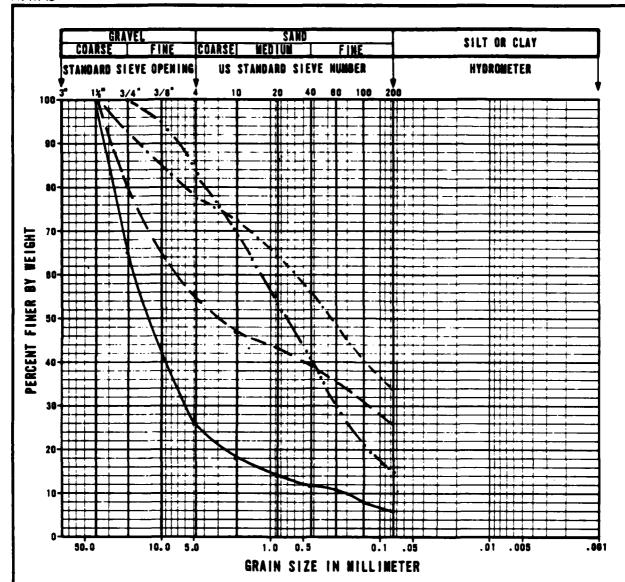
	<del></del> -				,		,		,							 		 	
CALCIUM	CARBONATE	mg/kg	125	***	906	1140	368	336	261	214	289	7175	138	172					
u,	CALCIUM	mg/kg	13	271	69	121	96	12	817	81	99	2420	12	75					
NATER SOLUBLE	SULPHATE	ng/kg	36	22	31	100	1440	`34	37	63	48	157	37	36					
M	CHLORIDE	Bg/kg	78	121	31	454	291	47	.82	367	26	197	48	47					
	\$00 I UM	ng/kg	722	09	29	187	183	400	96	809	32	1780	100	180					
	콥		9.4	8.6	9.1	8.7	8.4	9.4	8.6	9.2	9.0	8.2	8.4	9.4					
	SOT TYPE		SW-SM	ML	.WS	GW-GM	SP-SM	SM	MS	၁ၭ	GP-GM	่ว	SM-SC	S.					
147021	MICHTAL	METERS	7.68 - 7.92	0.15 - 0.46	15.00 - 15.24	2.77 - 3.02	5.82 - 6.07	21.40 - 21.64	10.73 - 10.97	1.22 - 1.52	1.22 - 1.52	0.15 - 0.61	0.15 - 0.61	1.22 - 1.52					
	SAMPLE INICHTAL	FEET	25.2 - 26.0	0.5 - 1.5	49.2 - 50.0	9.1 - 9.9	19.1 - 19.9	70.2 - 71.0	36.2 - 36.0	4.0 - 5.0	4.0 - 5.0	0.5 - 0.2	0.5 - 2.0	4.0 - 5.0					
	SAMPLE		D-7	P-1	D-12	D-4	0-7	D-18	6-Q	<b>p-2</b>	P-2	B-1	B-1	<b>b-2</b>					
	ACTIVITY S		BL-B-1	BL-8-4		BL-8-5	BL-B-7	BL-B-8	BL-8-10	BL-T-8	BL-T-10	BL-T-17	BL-P-1	BL-P-2					

SUMMARY OF CHEMICAL TEST RESULTS
OPERATIONAL BASE SITE
BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - SMO

TABLE II-5-3

UGRO NATIONAL IN



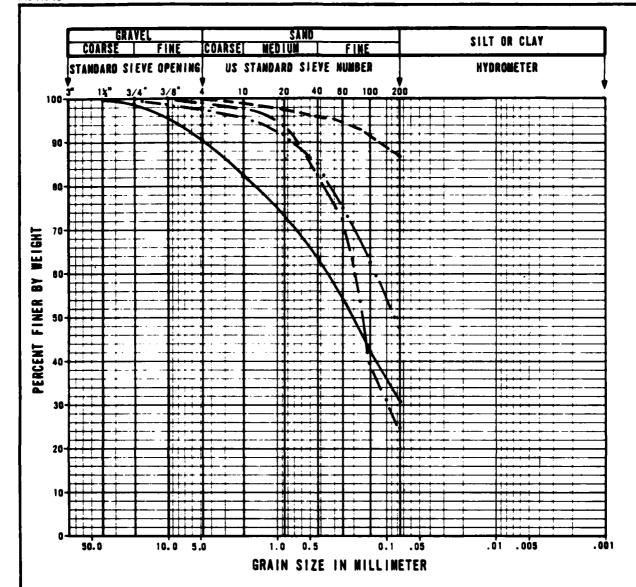
SYMBOL	COMPOSITE Sample	ACTIVITY	SAMPLE	INTERVAL	SOIL
21 MDQL	NUMBER	NUMBER	FEET	METERS	TYPE
_	Α	BL-T-2	0.5 - 2.0	0.15 - 0.61	GP- GM
	В	BL·T·3	0.5 - 2.0	0.15 - 0.61	GM
	С	BL-T-4	0.5 - 2.0	0.15 - 0.61	SM
	D	BL-T-29	0.5 - 2.0	0.15 - 0.61	SM

GRAIN SIZE CURVES, CBR TESTS
OPERATIONAL BASE SITE
BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - 8000

∏-5-4 1 OF 2

UGRO NATIONAL INC.



SYMBOL	COMPOSITE Sample	ACTIVITY	SAMPLE	INTERVAL	SOIL
21 MOAL	NUMBER	NUMBER	FEET	METERS	TYPE
	E	BL-P-10	0.5 - 2.0	0.15 - 0.61	SM
	F	BL-P-12	0.5 - 2.0	0.15 - 0.61	ML
	G	BL-P-15	0.5 - 2.0	0.15 - 0.61	SM
	Н	BL-P-21	0.5 - 2.0	0.15 - 0.61	SM

GRAIN SIZE CURVES, CBR TESTS
OPERATIONAL BASE SITE
BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - 800

FIGURE 11-5-4 2 OF 2

<u>ubro national inc.</u>

COMPOSITE SAMPLE	SOIL	PERCENT PASS ING	ATTE	ATTERBERG LIMITS	SPECIFIC	MAXIN DRY DEN	MAXINUM DRY DENSITY	OPT INUM MOISTURE		COMPACTED RY DENSITY	COMPACTED	PERCENT OF MAXIMUM	CB8
		<b>#</b> 200	11	PI	GKAVIIT	pcf	kg/m3	(\$)	pcf	kg/m3	(\$)	DRY DENSITY	(\$)
									123.0	1970	11.3	0.88	171
_									116.5	1866	11.5	92.8	83
	d Z	9				125.5	2011	11.0	112.0	1794	12.4	89.2	11
	5												
_													
									111.5	1786	16.8	97.0	.45
									108.4	1737	16.6	94.3	39
_	₽	<b>7</b> 9	61	18		115.0	1842	15.0	100.0	1602	15.7	87.0	0
$\vdash$									118.1	1892	11.8	99.4	16
_									113.1	.1812	11.9	95.2	10
	SM	ह	Ş	16		118.8	1903	11.8	107.5	1722	12.7	90.5	9
					•								
$\vdash$									118.9	1905	11.4	2.96	41
									113.0	1810	11.7	61.6	16
	NS.	15				122.9	1969	11.4	106.4	1706	11.5	9.98	3
_													

CALIFORNIA BEARING RATIO (CBR)
TEST RESULTS
OPERATIONAL BASE SITE
BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - 500

140LE 1-5-4 1 OF 2

UERO NATIONAL INC.

USAF -08

20 MAR 81

	_	_			_	_	_	_	_	_	_	_	_		_	_	_	_		_	_
	(\$)	99	22	8		i	4	7	7			28	43	14			<b>5</b> 6	13	•		
PERCENT OF MAX ( WUR	DRY DENSITY	97.2	93.1	89.1			6.96	91.7	85.7			87.8	94.6	88.6			95.3	90.4	84.2		
COMPACTED	(\$)	10.4	10.4	10.5			17.3	17.6	17.2			11.1	11.1	11.1			11.6	11.9	12.3		
COMPACTED DRY DENSITY	kg/m3	1950	9981	1788			1748	1655	1546			1958	1895	1773			1871	1775	1653		
	pcf	121.7	116.5	111.6			109.1	103.3	96.5			122.2	118.3	110.7			116.8	110.8	103.2		
OPT INUM WOISTURE	(\$)			9.8					16.4					10.9					11.5		
MAXIMUM DRY DENSITY	kg/m3	2006				1804				2003				1962							
	pcf			125.2					112.6					125.0					122.5		
SPECIFIC	<b>BKAVIIT</b>				·																
ATTERBERG LIMITS	H								œ												
ATTE	11								32												
PERCENT PASSING	#200			31					87					25					48		
1108	111			SM					ML					SM				. —	SM		
COMPOSITE Sample	NUMBER		ш		īF				9			I									

CALIFORNIA BEARING RATIO (CBR)
TEST RESULTS
OPERATIONAL BASE SITE
BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - 600

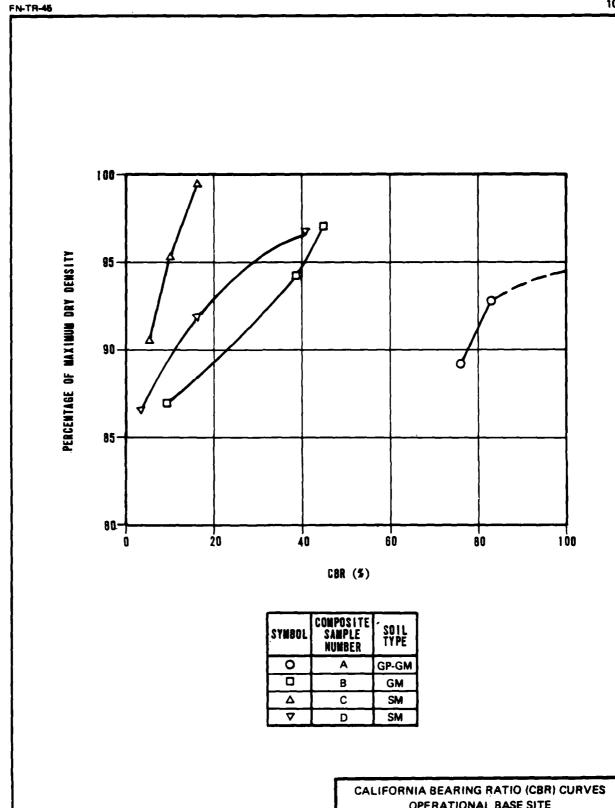
148LE 11-5-4 2 OF 2

**UGRO NATIONAL INC** 

20 MAR 81

USAF -08





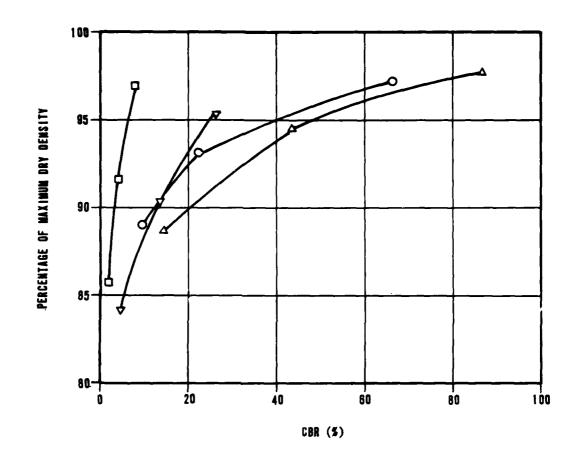
**OPERATIONAL BASE SITE** BERYL, UTAH

MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - BMG FIGURE II-5-5 1 OF 2

20 MAR 81







SYMBOL	COMPOSITE Sample Number	SOIL Type				
0	E	SM				
0	F	ML				
Δ	G	SM				
▽	Н	SM				

CALIFORNIA BEARING RATIO (CBR) CURVES
OPERATIONAL BASE SITE
BERYL, UTAH

WX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - 800

TI-5-5 2 OF 2

UGRO NATIONAL II

USA F-07

20 MAR 81

SECTION 6.0

EXPLANATION OF CONE PENETROMETER TEST RESULTS

.

1

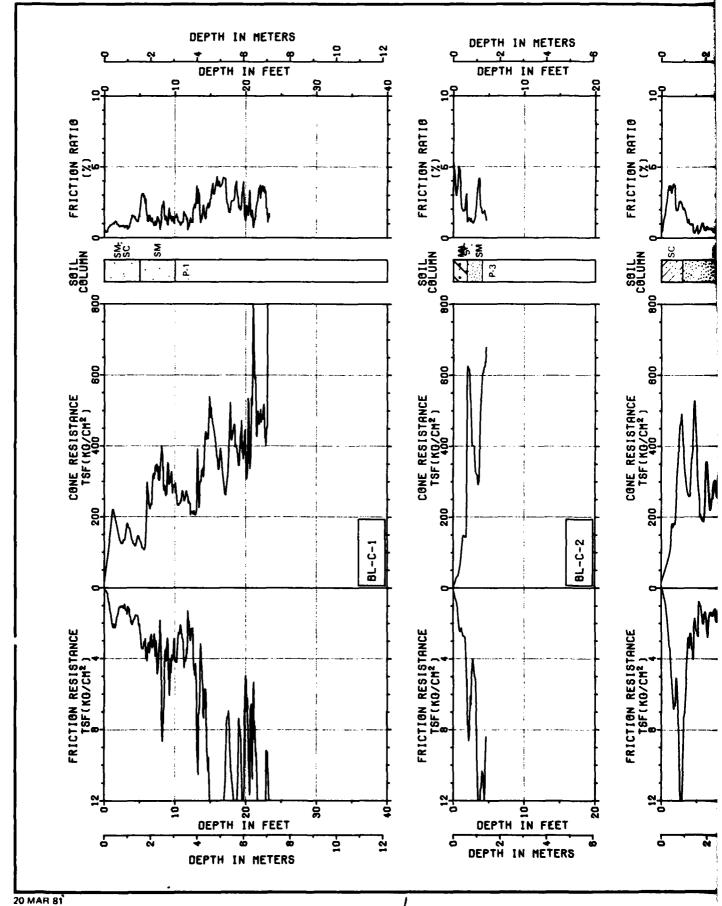
. .

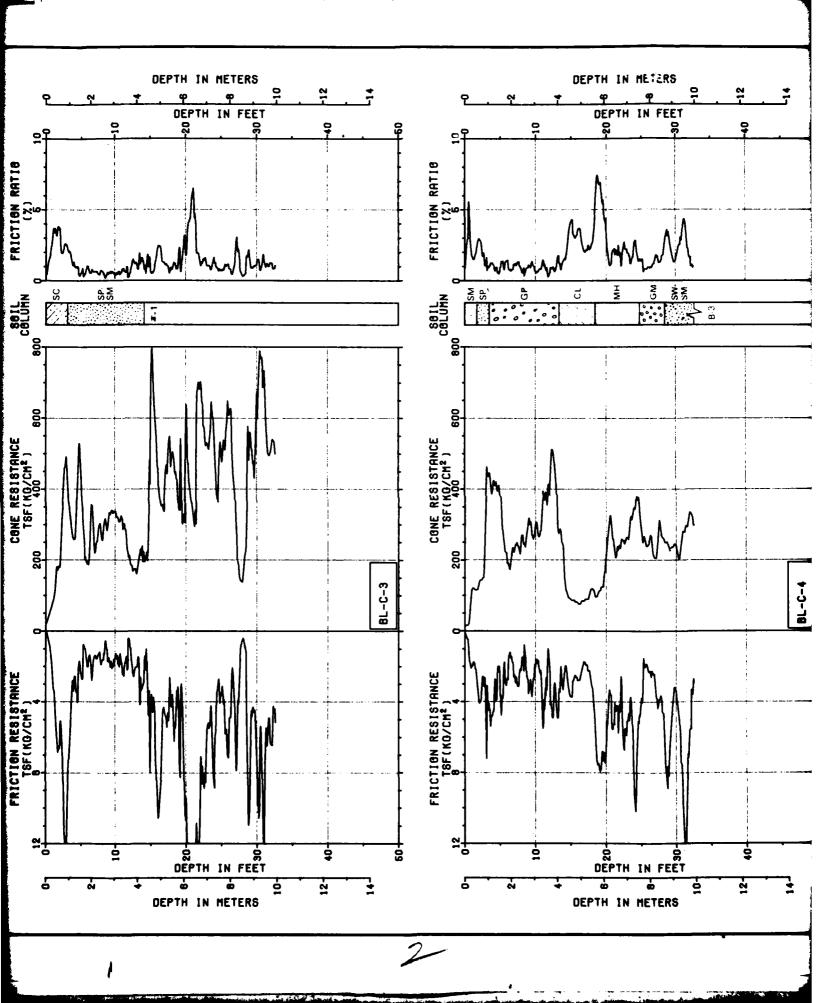
## 6.0 EXPLANATION OF CONE PENETROMETER TEST RESULTS

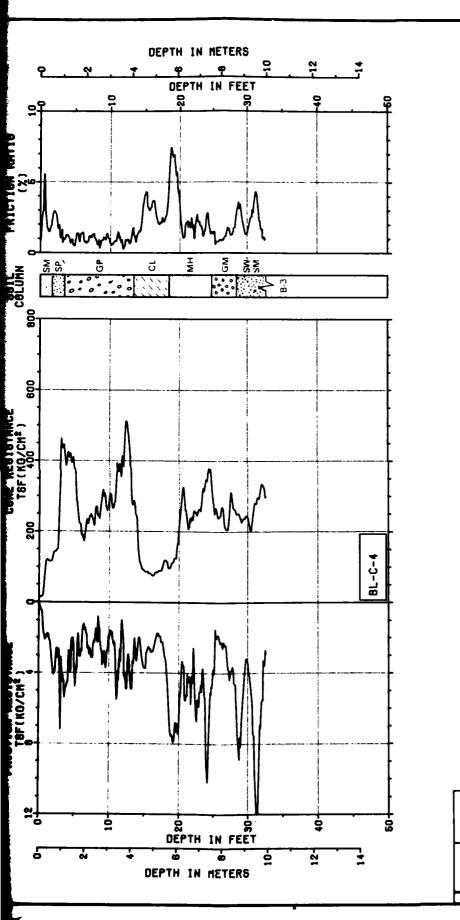
The results of all cone penetrometer tests are presented in this section. Explanations of the test results are as follows:

- A. Friction Resistance The resistance to penetration developed by the friction sleeve, equal to the vertical force applied to the sleeve divided by its surface area. This resistance is the sum of friction and adhesion.
- B. Cone Resistance The resistance to penetration developed by the cone, equal to the vertical force applied to the cone, divided by its horizontally projected area.
- C. Friction Ratio The ratio of friction resistance to cone resistance.
- D. Designation Each cone penetrometer test is identified by a number: for example BL-C-1.
  - BL abbreviation for the site (e.g., BL-Beryl)
  - C abbreviation for the CPT
  - 1 number of the test
- E. Soil Column A graphical presentation of the soil type versus depth at each cone penetrometer test location where either a boring, trench, or test pit was performed. The Unified Soil Classification Symbol for each different soil type is listed immediately to the right of the soil column.

Immediately below the soil column, the activity number for the corresponding boring, trench, or test pit at each CPT location is given.



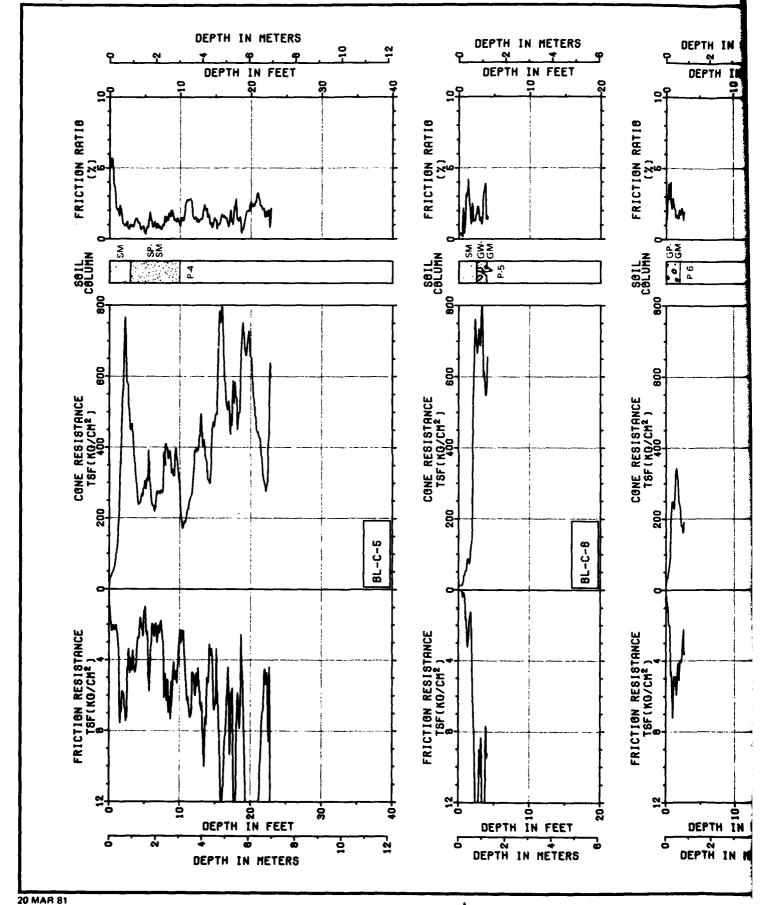


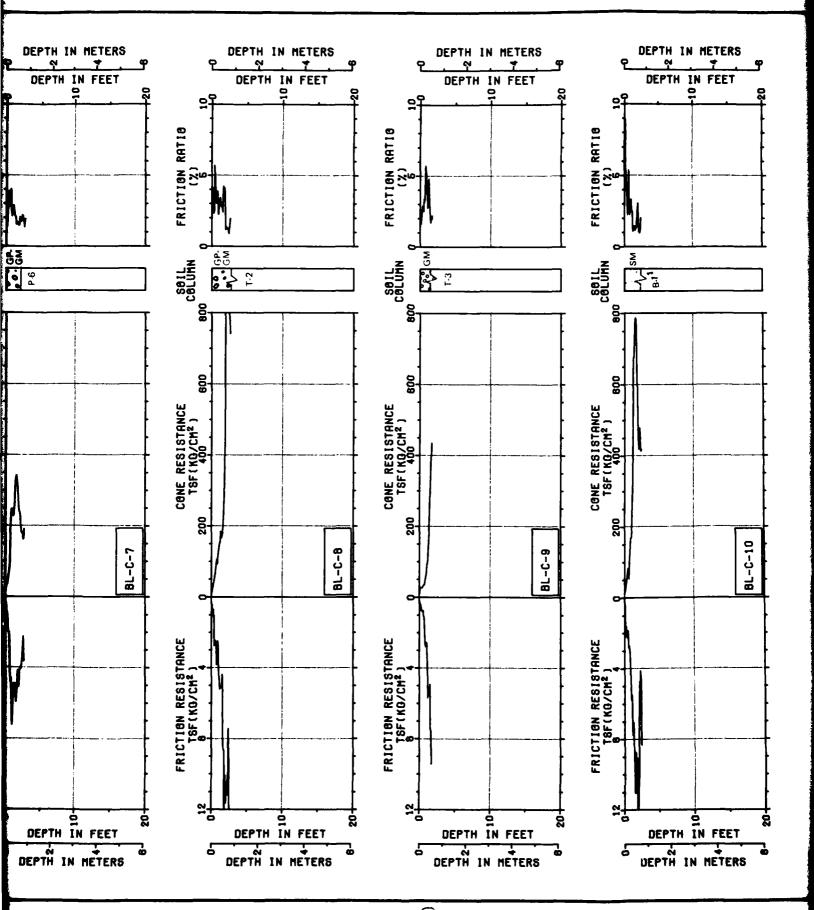


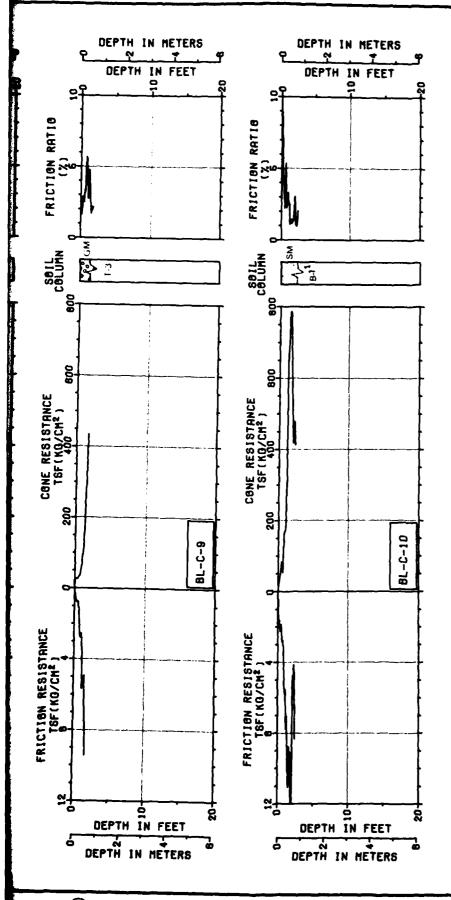
MX SITING INVESTIGATION

FIGURE

DEPARTMENT OF THE AIR FORCE BMO 11-6-1 1 OF 18





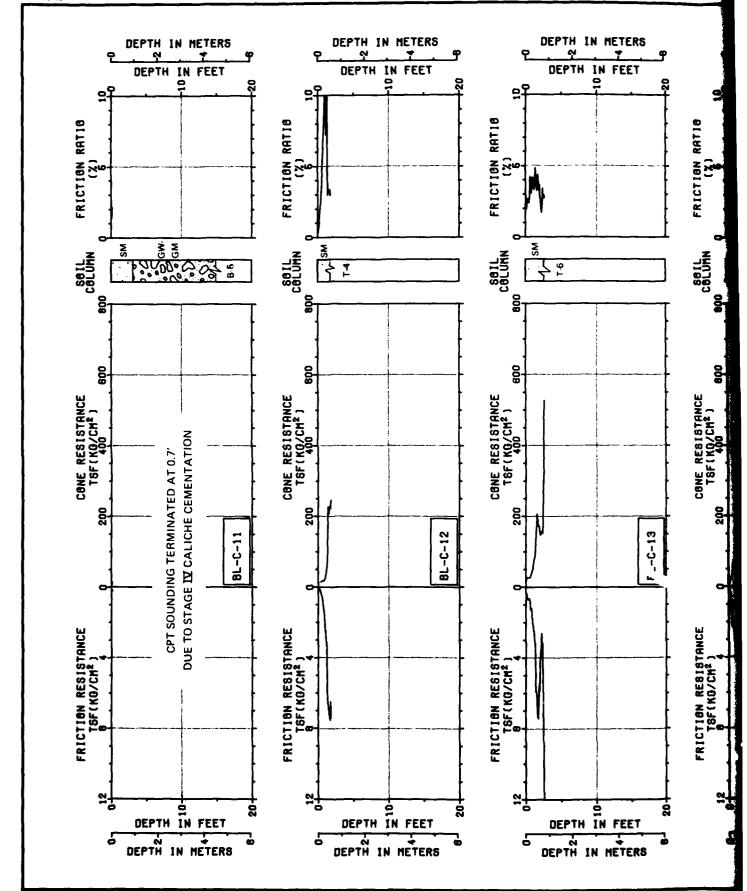


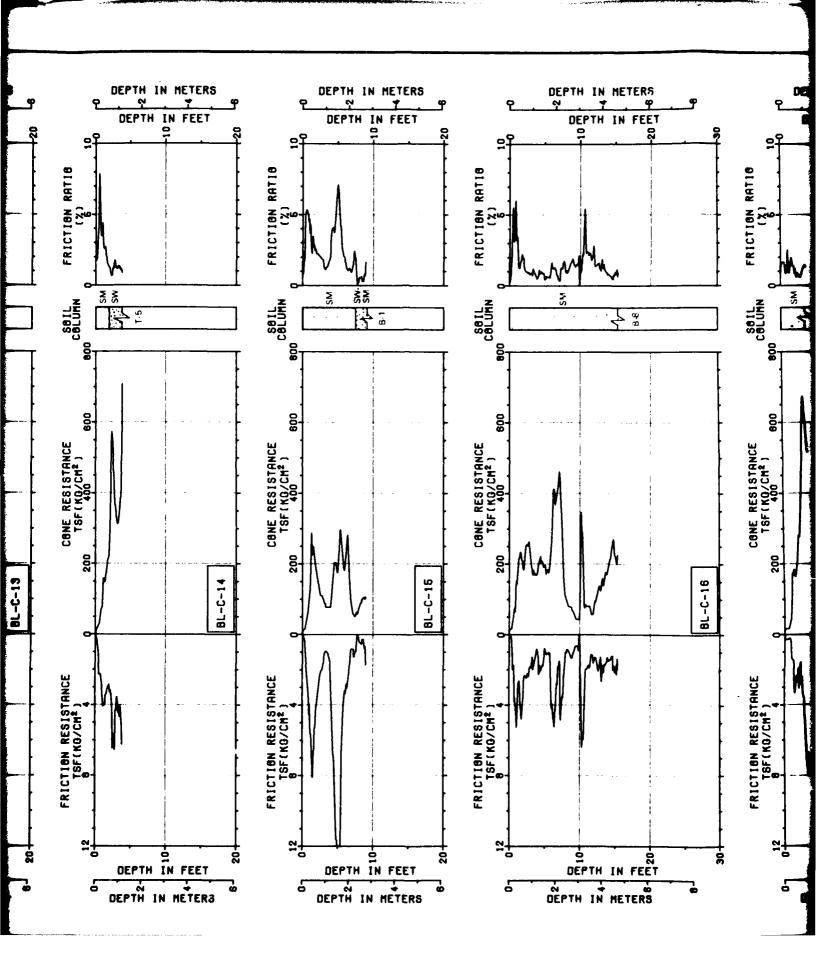
MX SITING INVESTIGATION

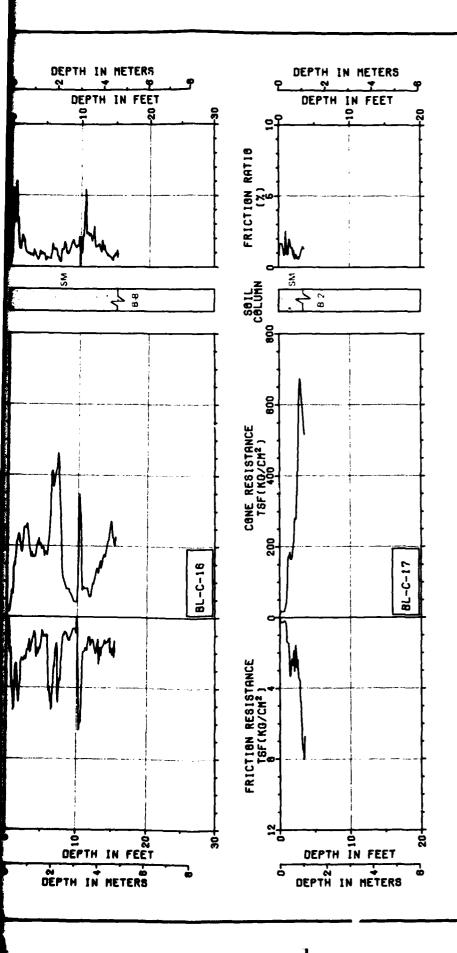
FIGURE

DEPARTMENT OF THE AIR FORCE - BMO

II-6-1 2 OF 15







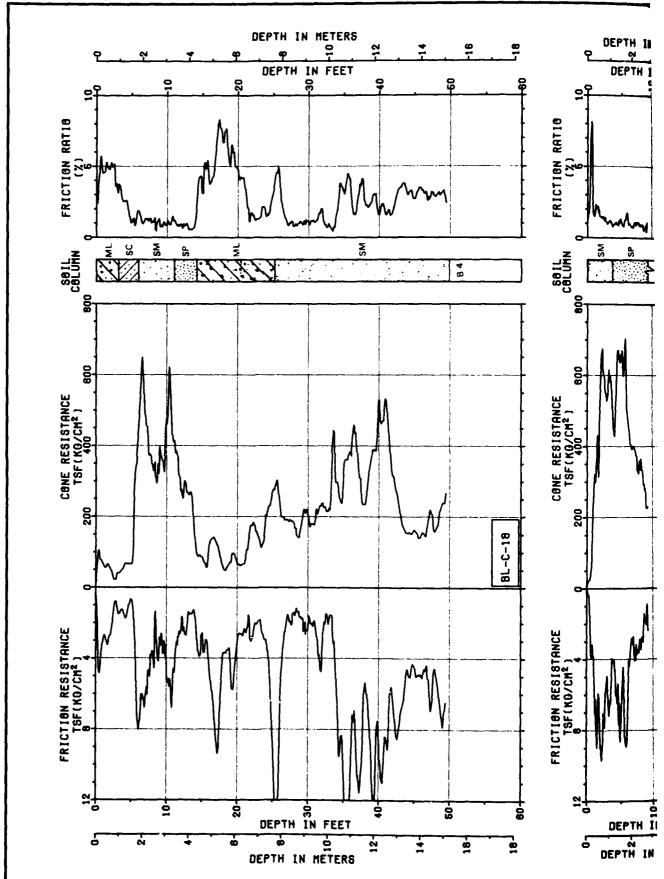
MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMO

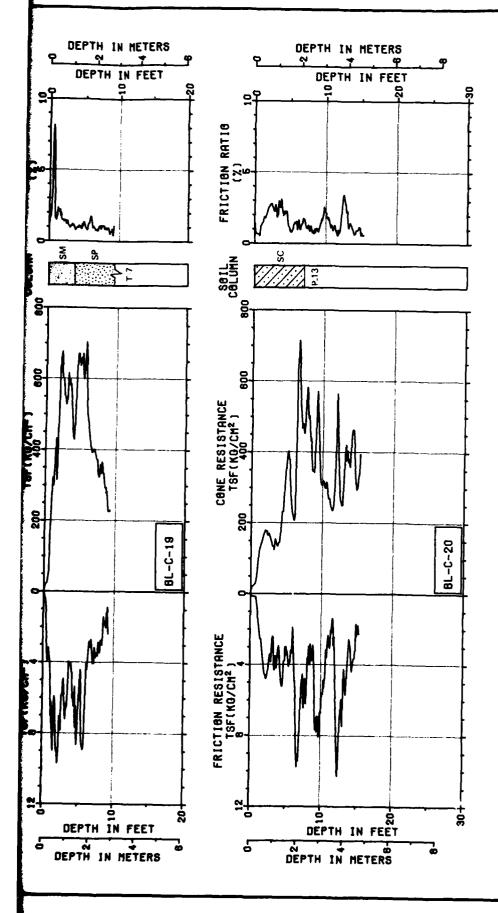
## 150 F 18

TUGRO MATIONAL IN

\_3

\$ X



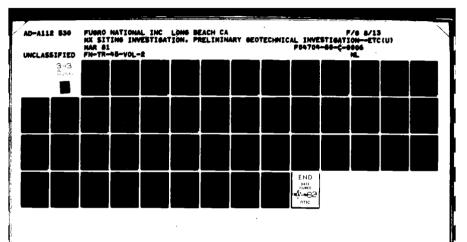


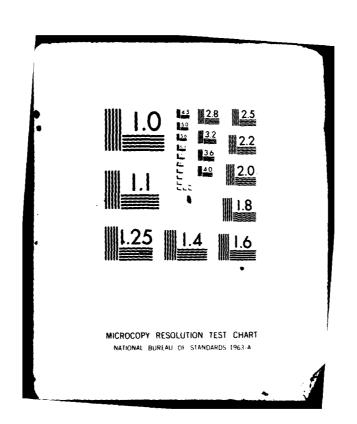
MX SITING INVESTIGATION

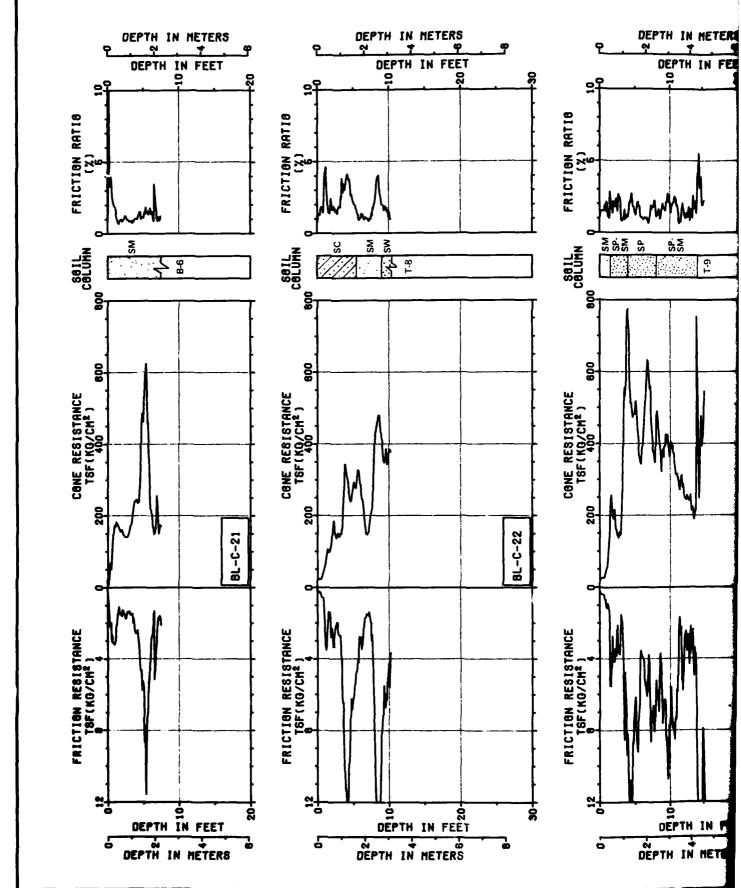
II-6-1 4 09 18

DEPARTMENT OF THE AIR FORCE - BMC

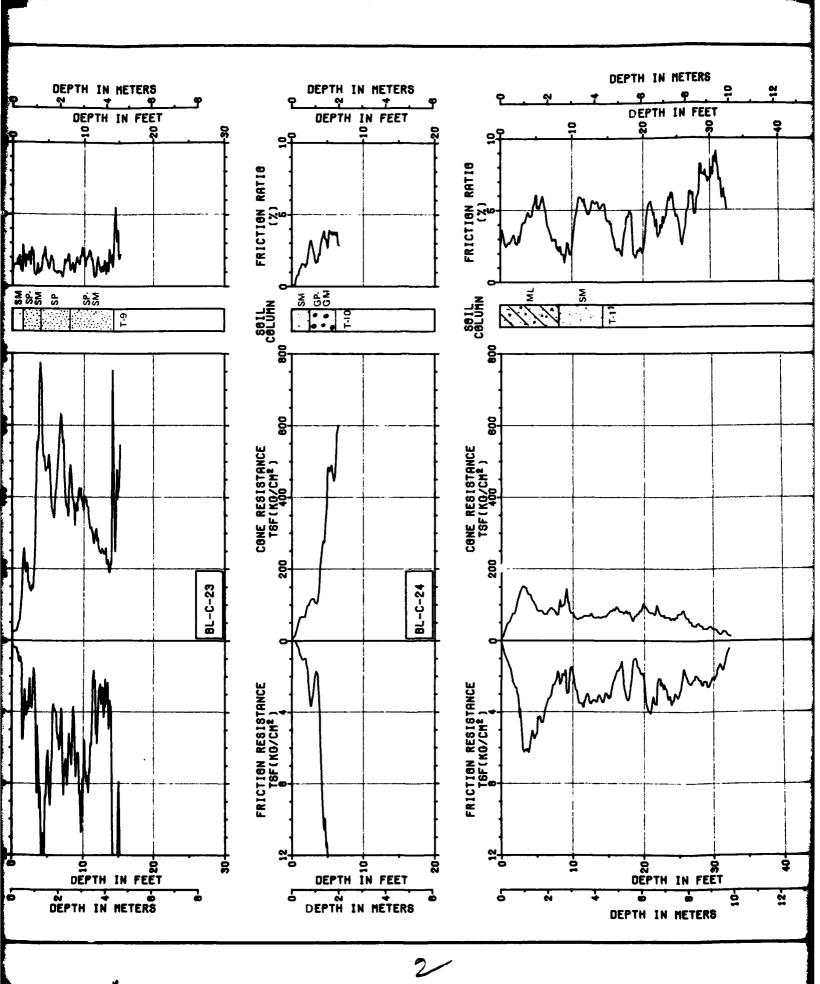
UGRO NATIONAL, INC.

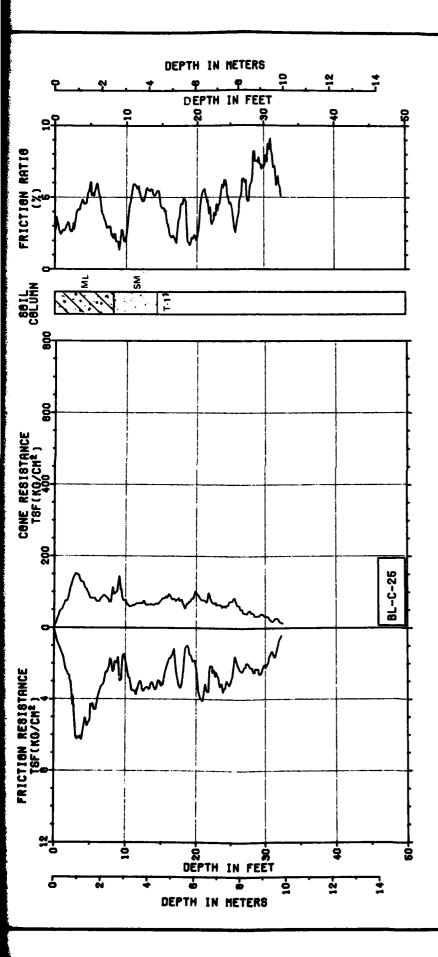






20 MAR 81





MX SITING INVESTIGATION

FIGURE

DEPARTMENT OF THE AIR FORCE - BMO

II-6-1 5 OF 10

UGRO NATIONAL, INC

٩

2

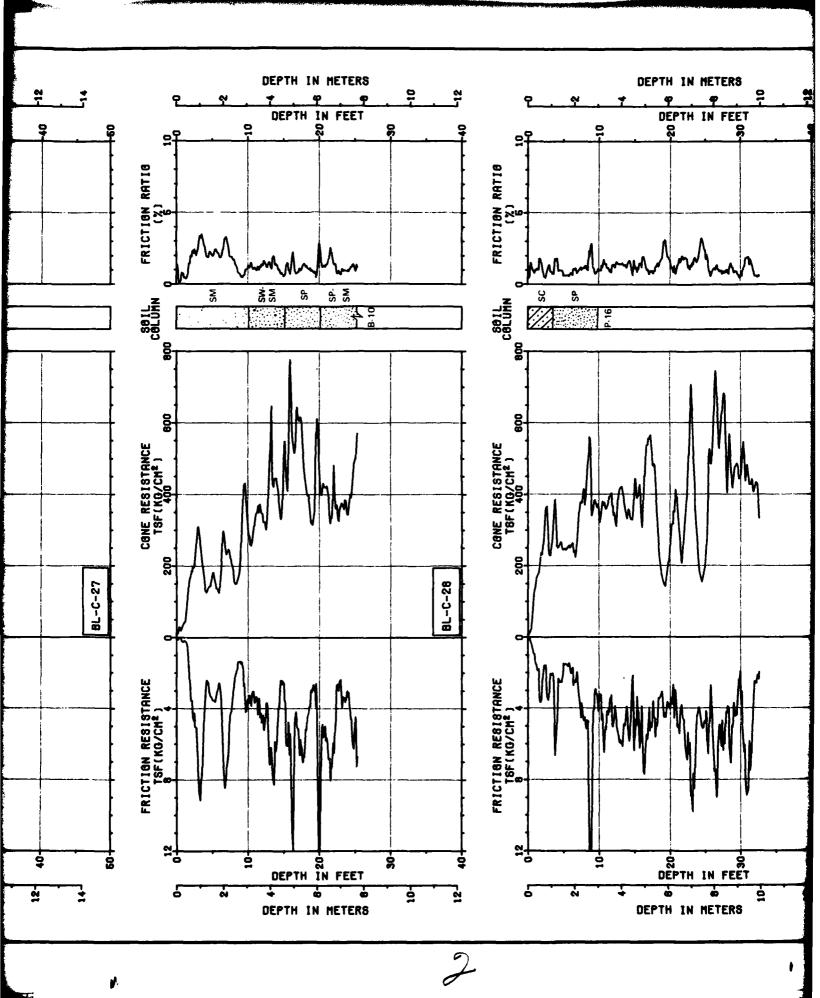
DEPTH IN FEET

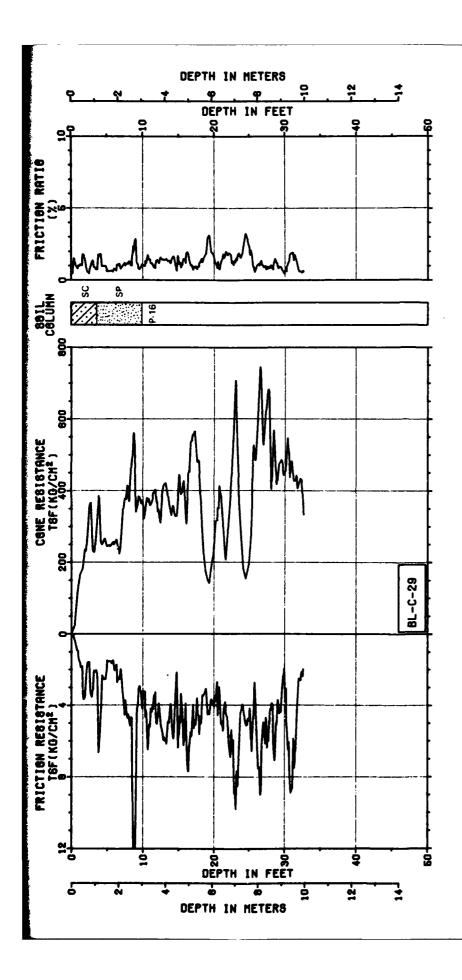
DEPTH IN METERS

12-

DEPTH IN FEET

DEPTH IN METERS



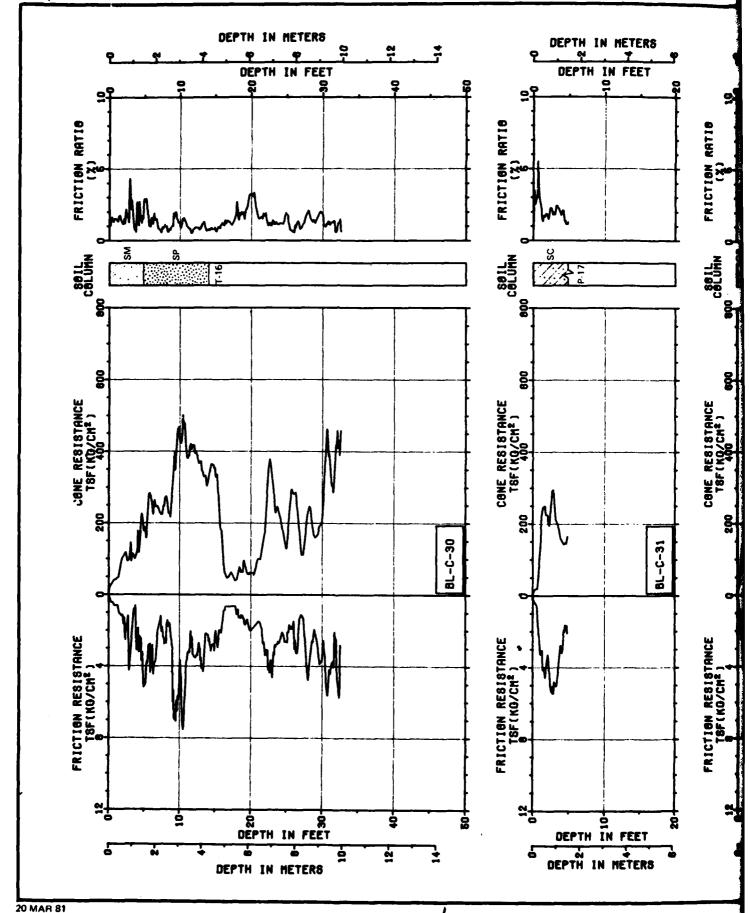


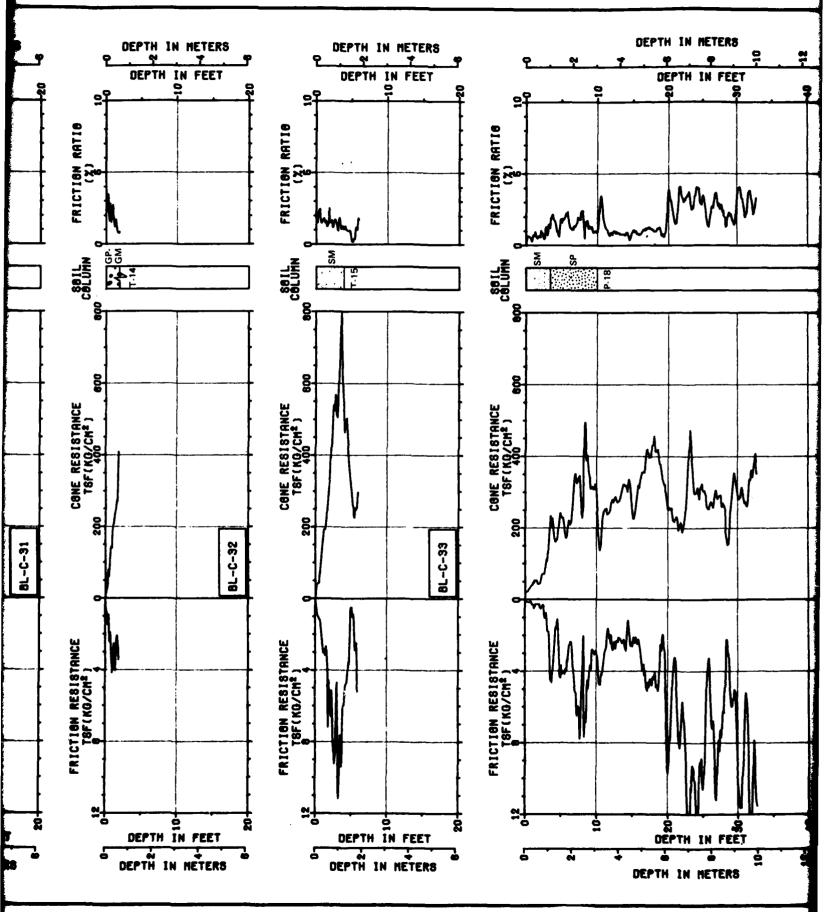
MX SITING INVESTIGATION

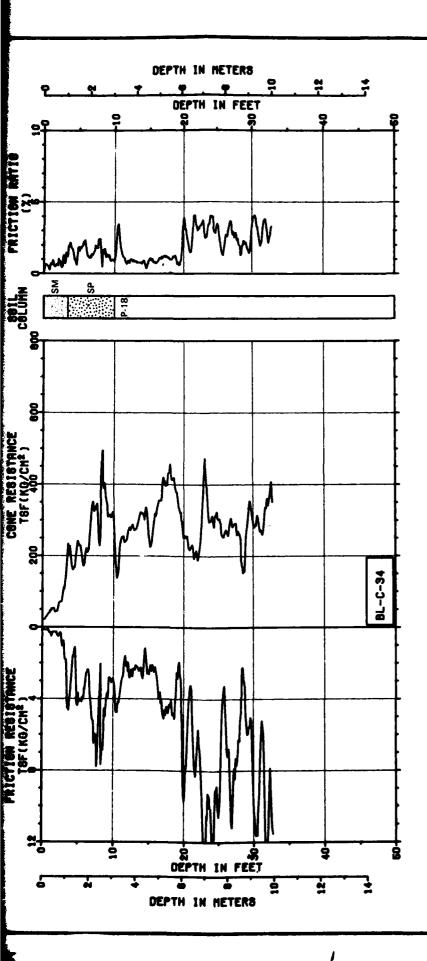
FIGURE 11-6-1 6 OF 18

DEPARTMENT OF THE AIR FORCE -- BMO

<u>lugro national, inc.</u>



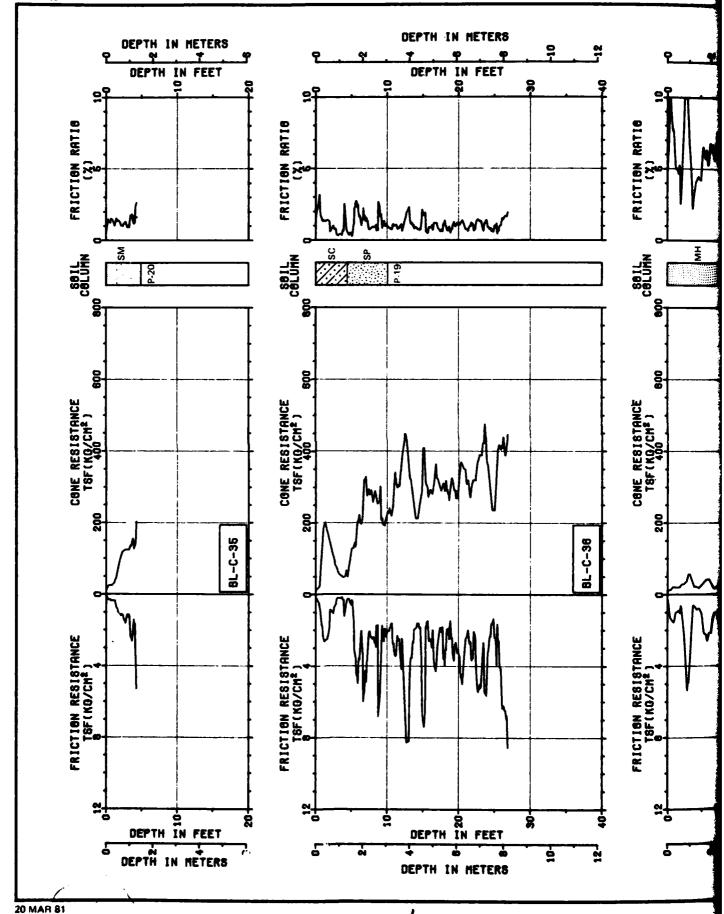


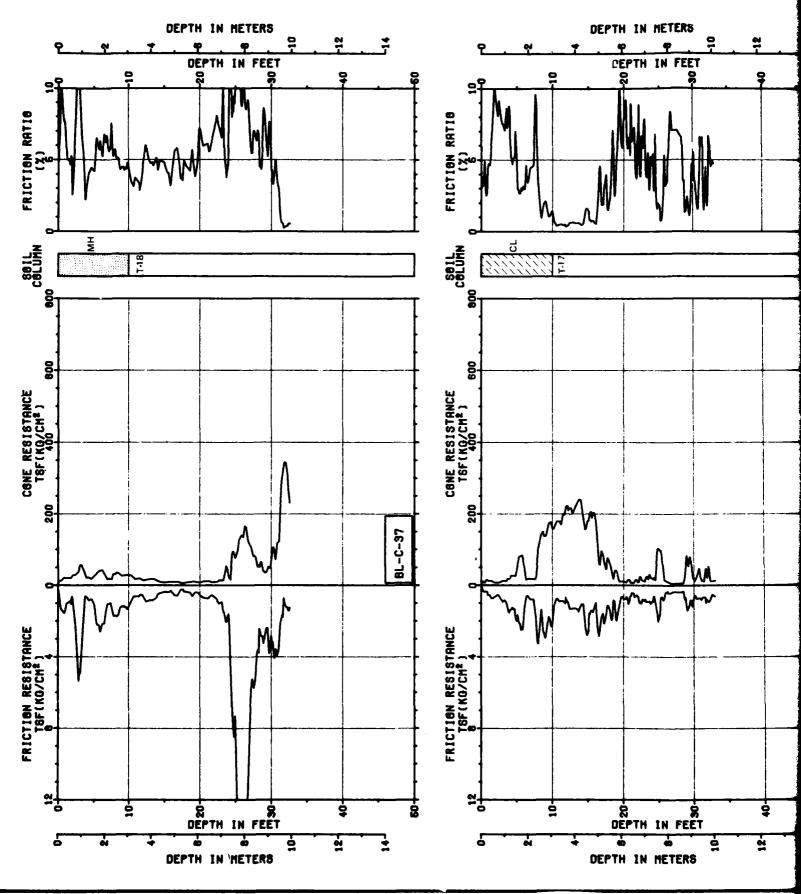


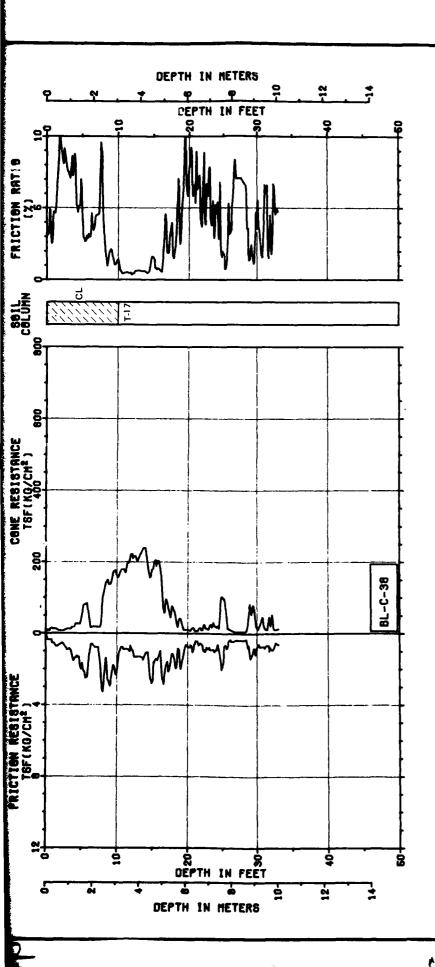
MX SITING INVESTIGATION

FIGURE II-6-1 7 OF 15

UBRO NATIONAL, INC.





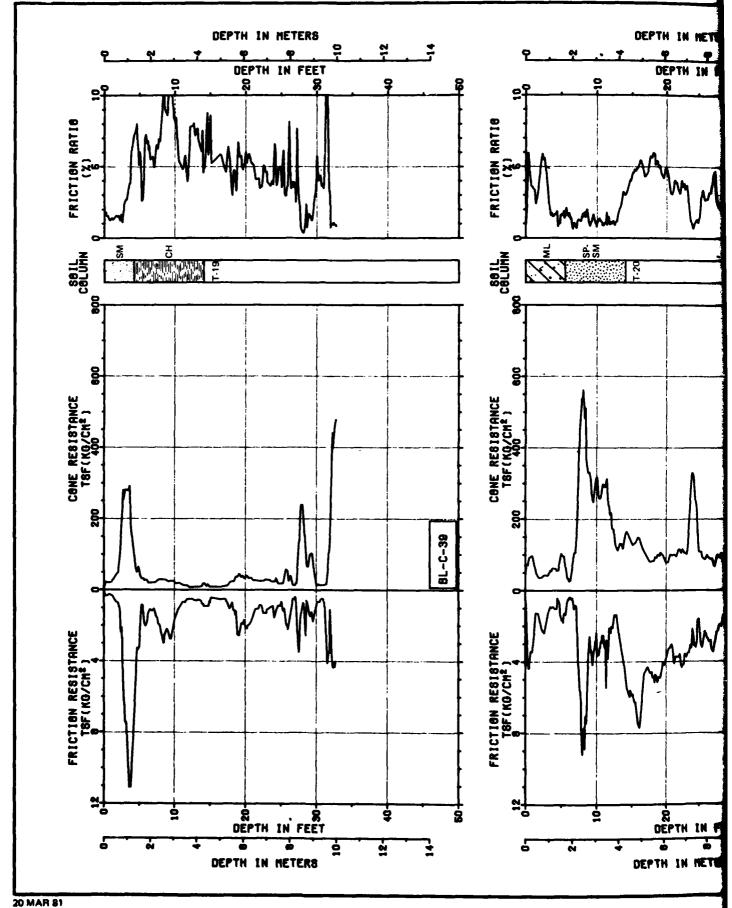


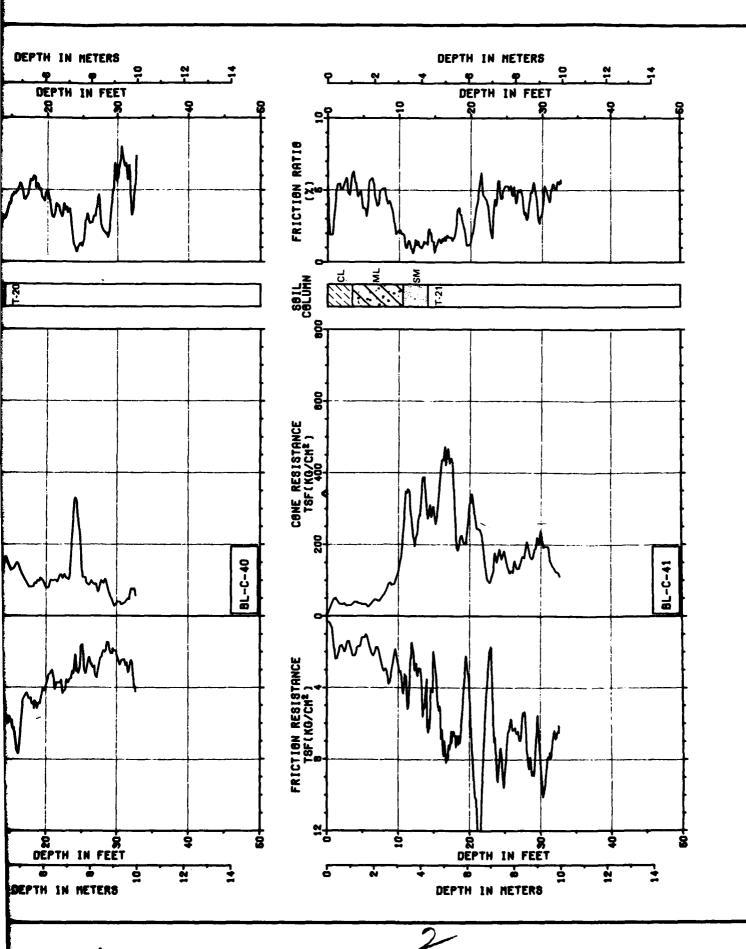
MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE — BMO

FIGURE II-6-1 8 OF 15

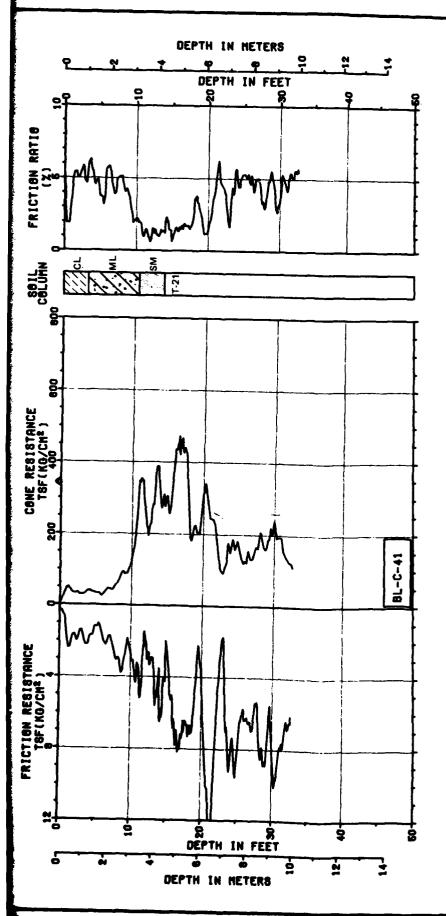
UGRO NATIONAL, INC.

>





CON

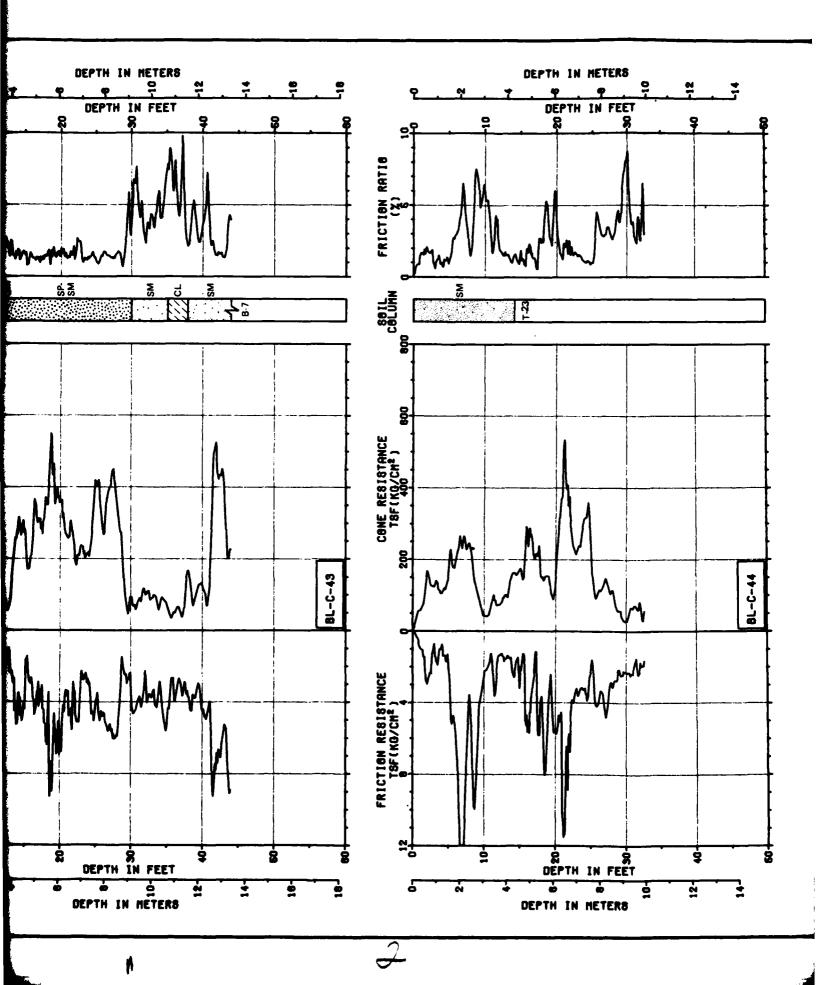


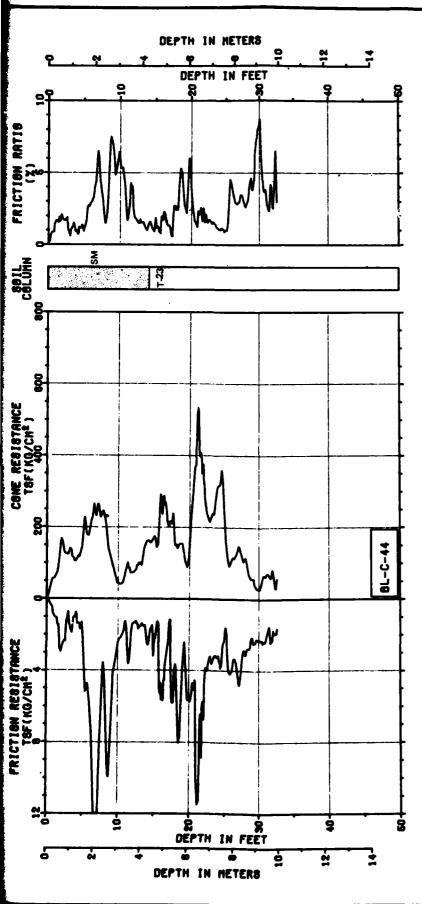
MX SITING INVESTIGATION

DEPARTMENT OF THE AIR FORCE - BMO

FIGURE []-6-1 9 OF 15

UGRO NATIONAL, INC.



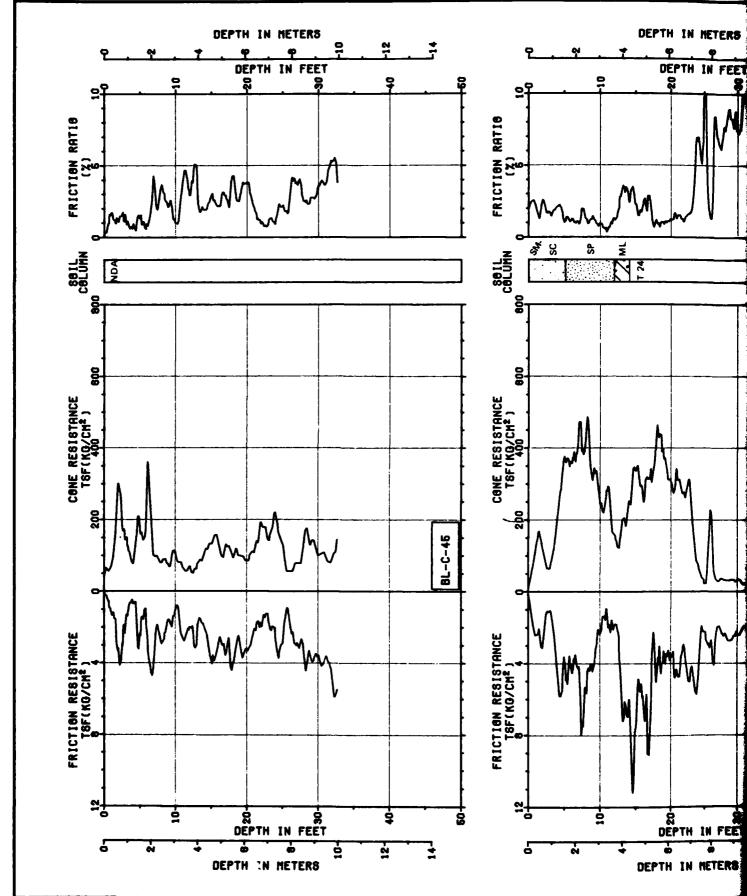


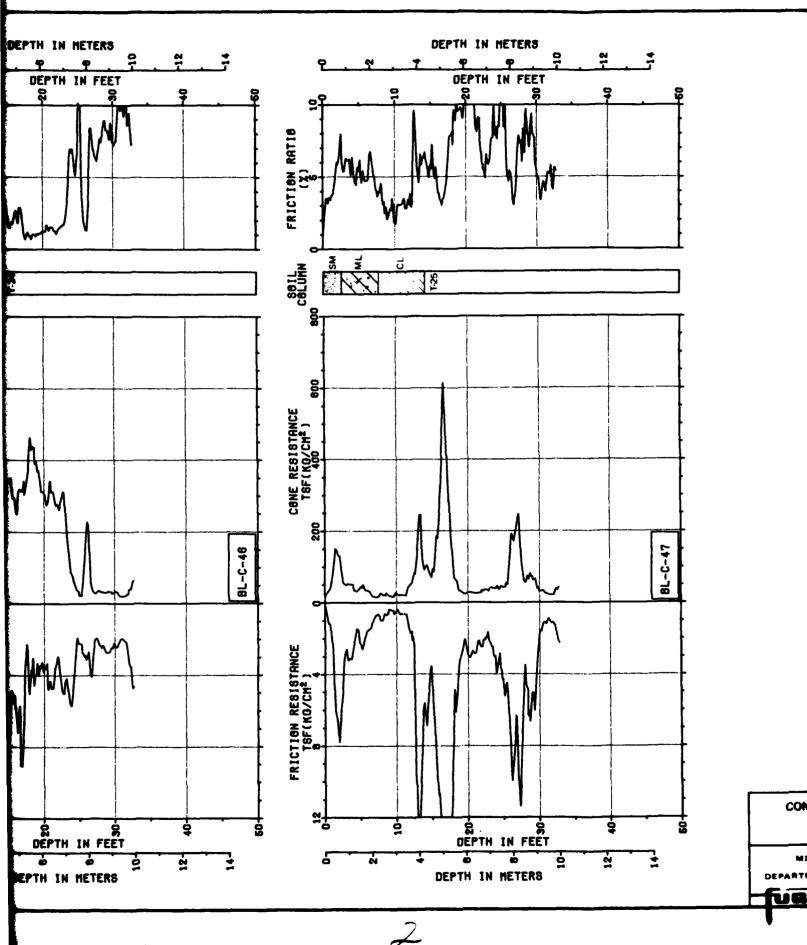
MX SITING INVESTIGATION

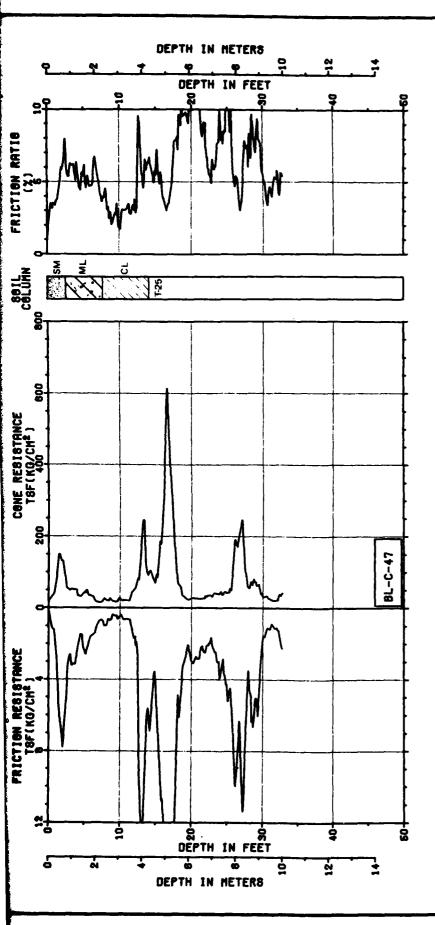
FIGURE II-6-1 10 OF 15

DEPARTMENT OF THE AIR FORCE - BMO

<u>ll, ing.</u>





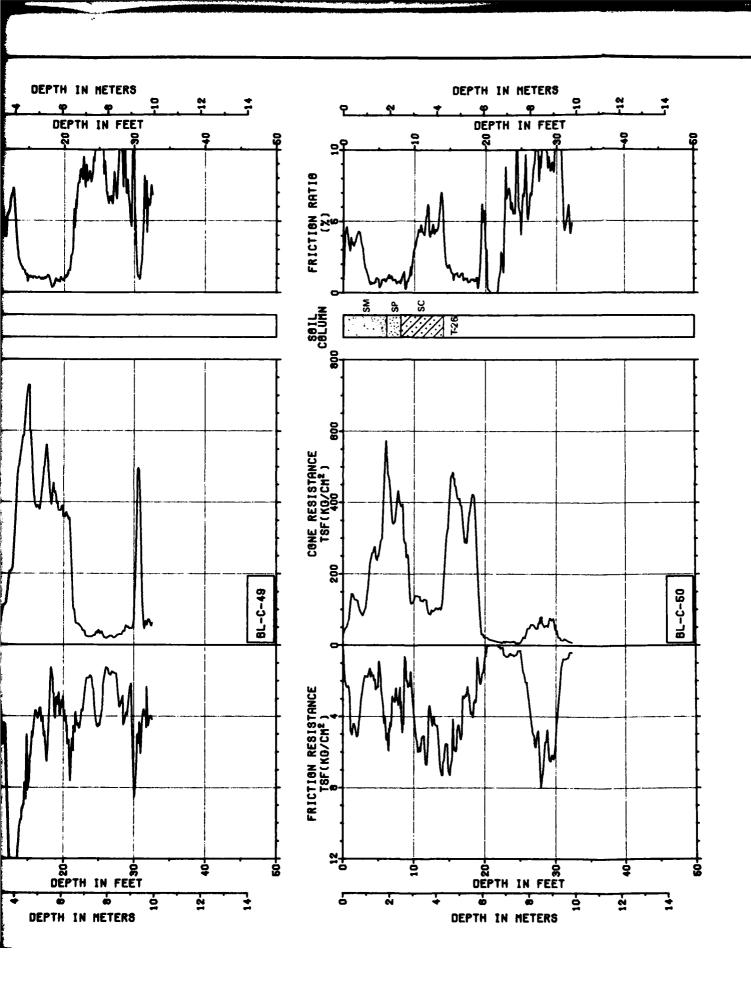


MX SITING INVESTIGATION

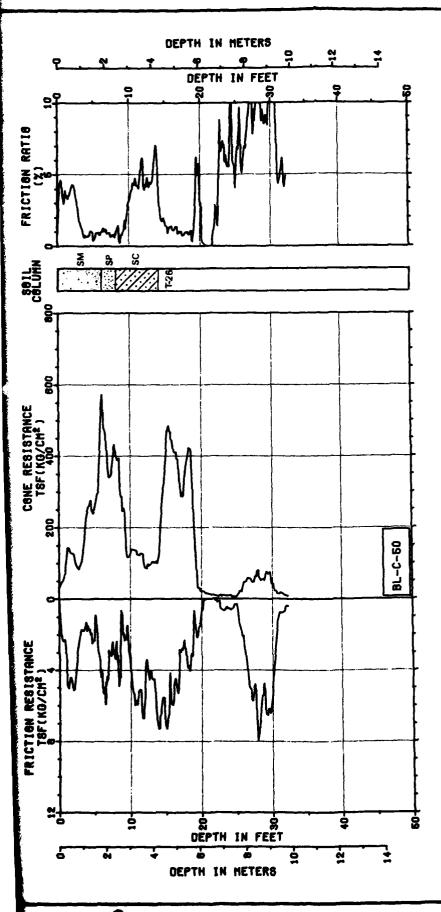
FIGURE ∏-6-1 11 OF 18

UGRO NATIONAL ING

The state of the state of the state of



DEM



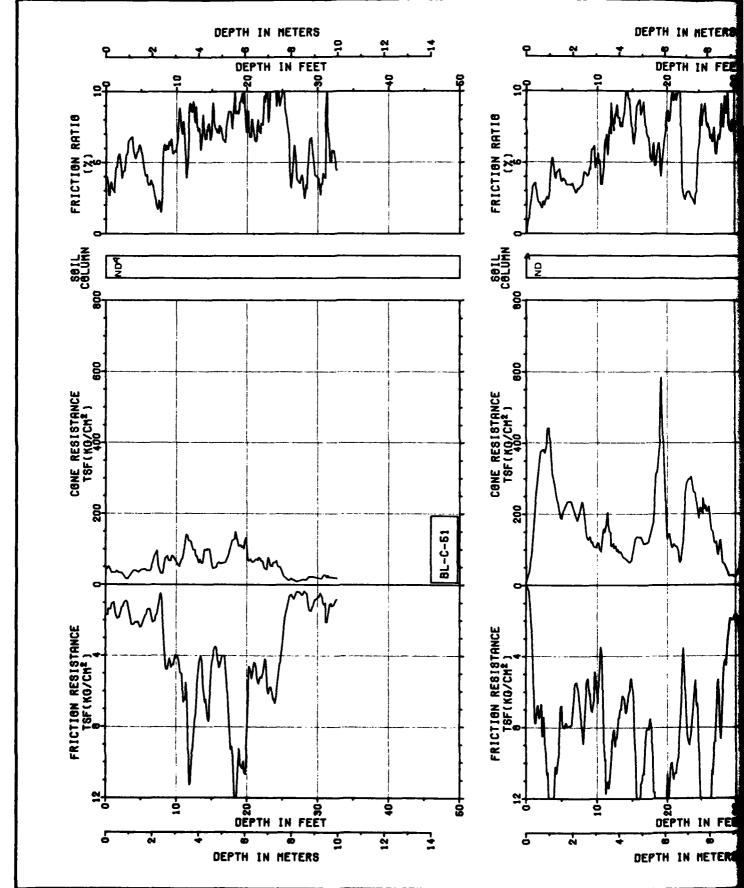
MX SITING INVESTIGATION

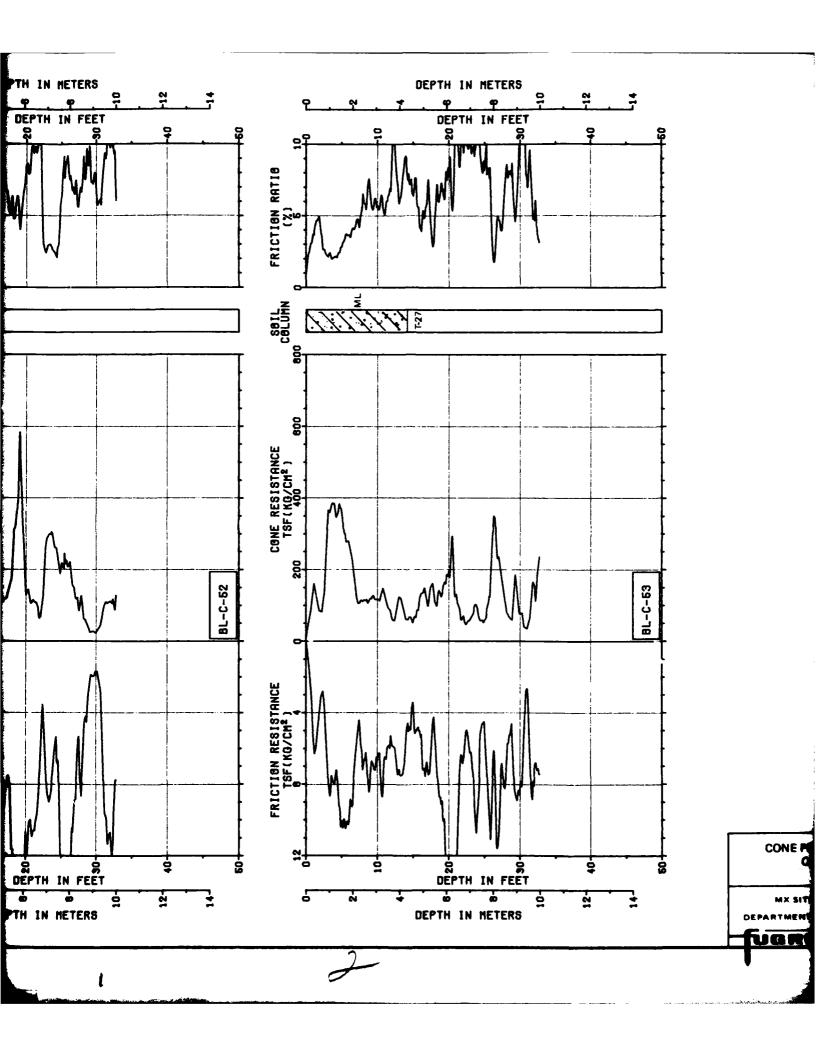
T-6-1 12 OF 18

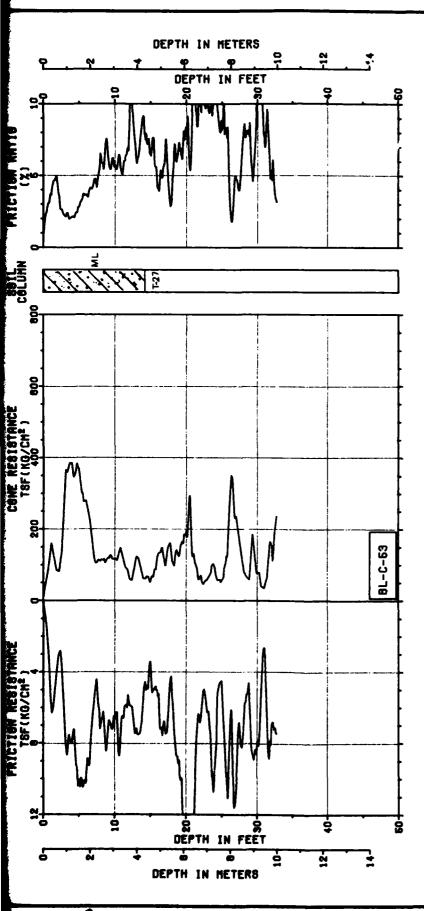
DEPARTMENT OF THE AIR FORCE - BMO

ING

2







MX SITING INVESTIGATION

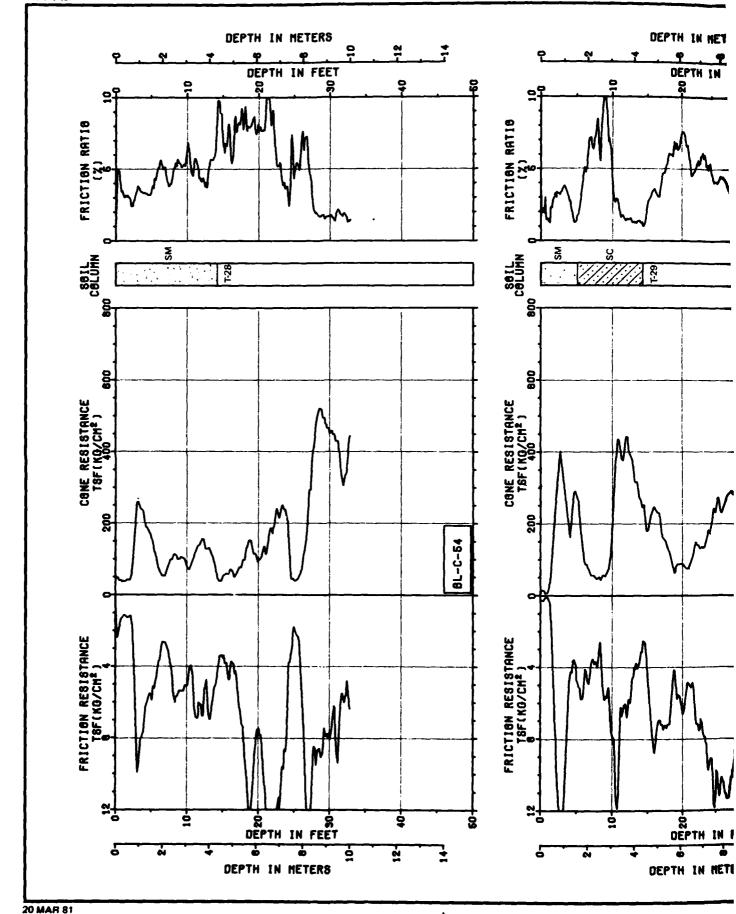
FIGURE

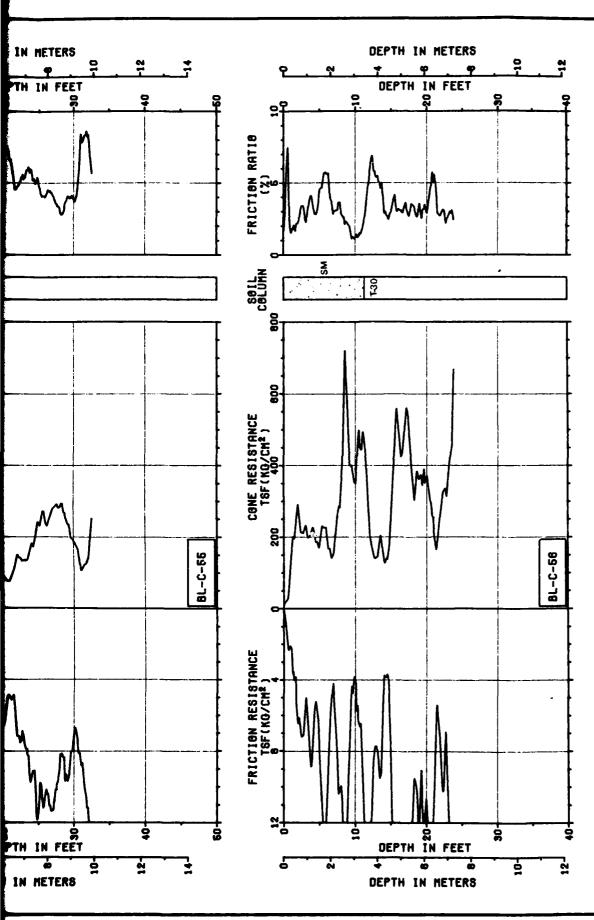
DEPARTMENT OF THE AIR FORCE - BM

II-6-1

UGRO NATIONAL, INC.

2

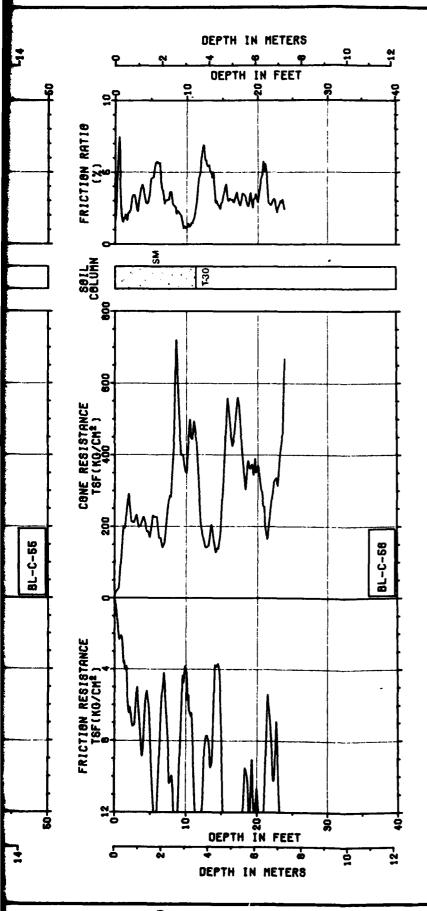




CONE PENETROMETER 1
OPERATIONAL BA

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORE

UGRO NATIO

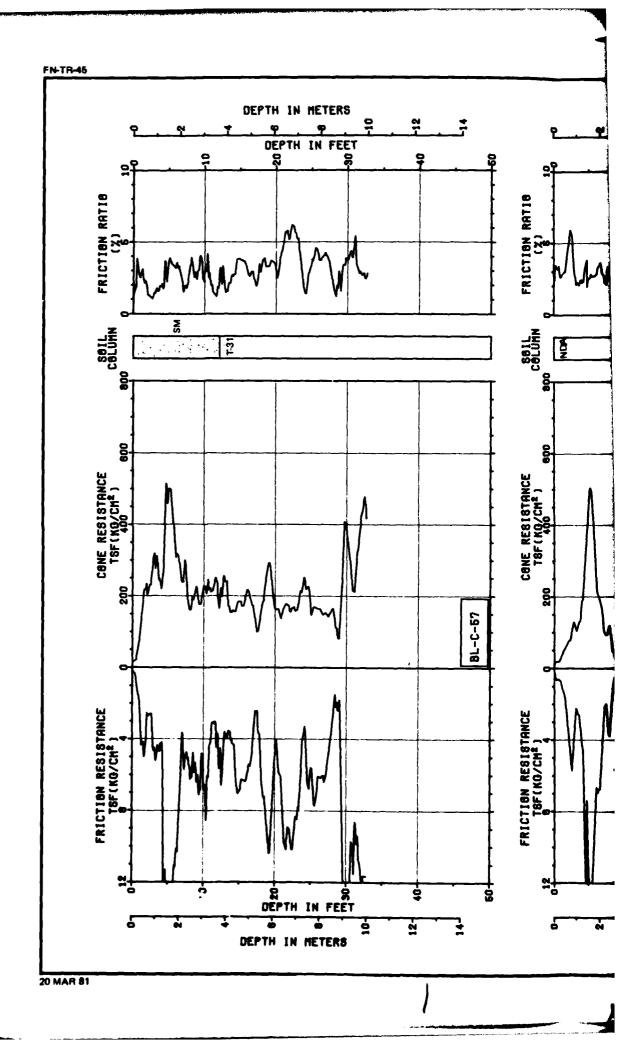


MX SITING INVESTIGATION

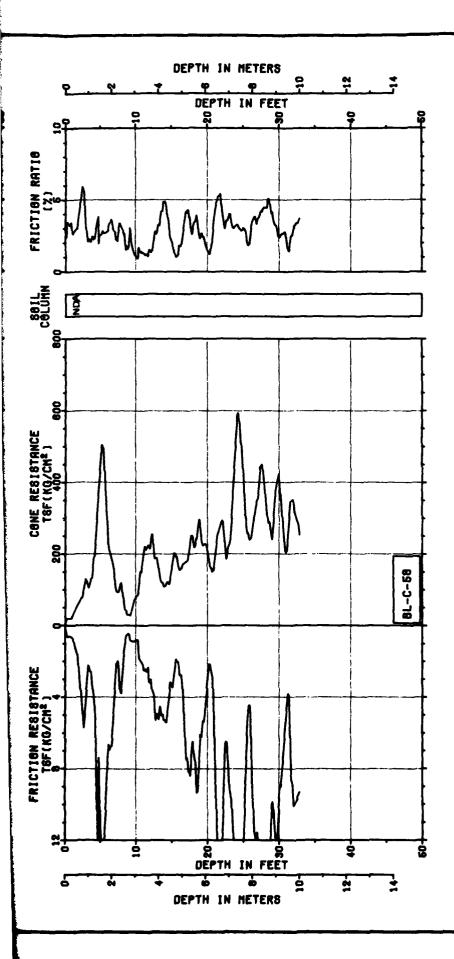
П-6-1 14 ОР 18

UGRO NATIONAL, INC

2



\*



MX SITING INVESTIGATION

FIGURE

DEPARTMENT OF THE AIR FORCE - BMO

∏-8-1 16 OF 18

UGRO NATIONAL, INC.

ب

SECTION 7.0

EXPLANATION OF SEISMIC REFRACTION DATA

## 7.0 EXPLANATION OF SEISMIC-REFRACTION DATA

Each figure shows seismic wave travel times plotted versus surface distance between the energy source (shot) and the detector (geophone) for a single seismic line. Distances are measured along the line from geophone number 1 which is designated as zero distance. Distances to the right (on the paper) of geophone 1 are positive. The direction arrow gives the approximate direction along the geophone array from geophone 1 to geophone 24.

## Travel Time Versus Distance Graph (Upper Half of Figure)

This is a travel time versus distance graph. The abscissa represents distance; the ordinate, time. The six vertical lines represent the locations of shots (designated as F, G, H, I, J, and K). The symbol, X, denotes travel times at geophones that were located to the right of a shot. The symbol,  $\Theta$ , denotes travel times that were located to the left of shots.

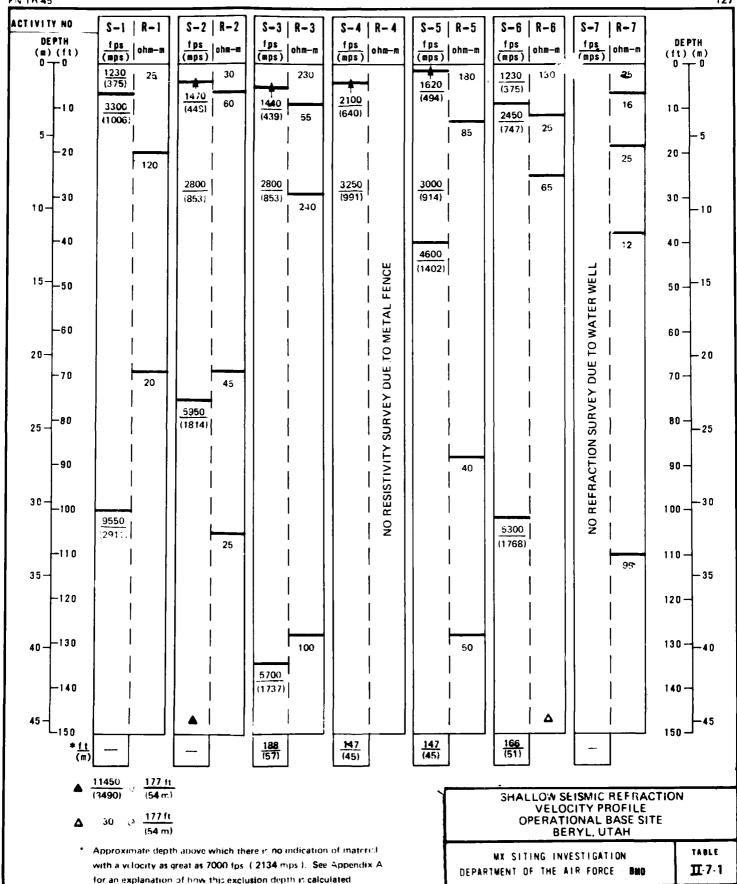
## Velocity Cross Section (Lower Half of Figure)

This is an interpreted velocity cross section beneath the seismic line. The top line represents the ground-surface profile. The short vertical lines crossing the top line mark the geophone positions. The depth scale is plotted relative to a point on the line which was arbitrarily chosen as "zero elevation" at the time the line was surveyed. The additional lines across the cross section represent the interpreted boundaries between layers of material with different compressional wave

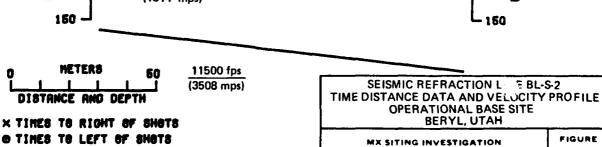
velocities. These boundaries are commonly called "refractors." The velocity interpreted to be representative of each layer is shown.

NOTE: There was no seismic refraction line at location BL-SR-7.

AFY-18



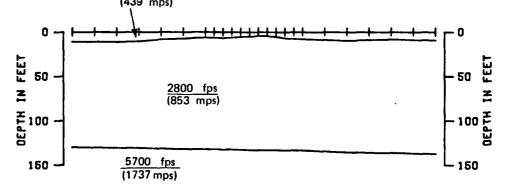
when the observed velocities are all less than 7000 fps. (  $2134\ \mathrm{mps}$  ).

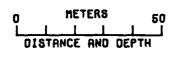


DEPARTMENT OF THE AIR FORCE - BMO

**∐-7-2** 

UGRO NATIONAL, INC





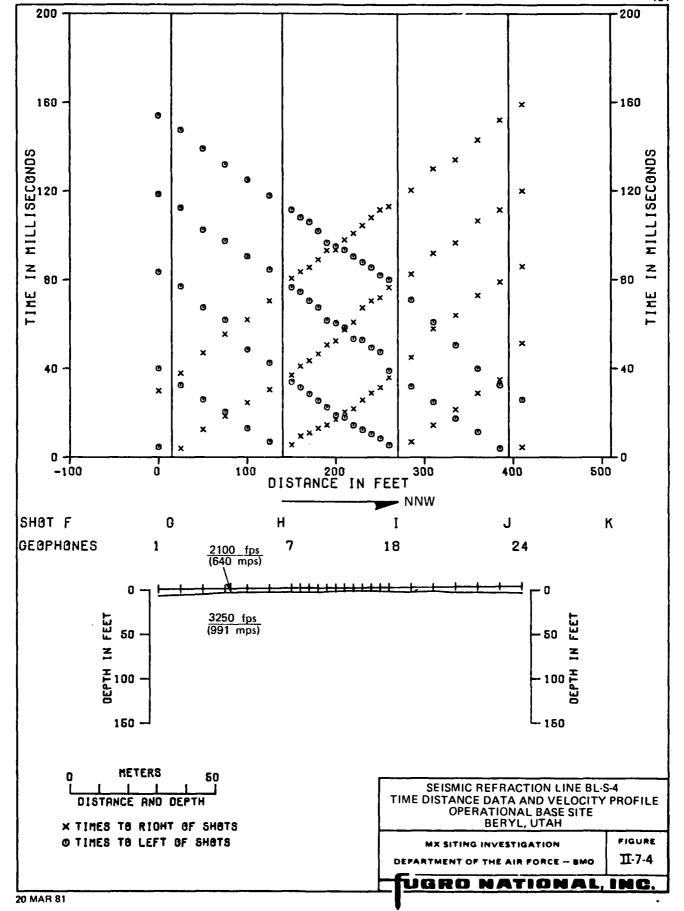
× TIMES TO RIGHT OF SHOTS O TIMES TO LEFT OF SHOTS

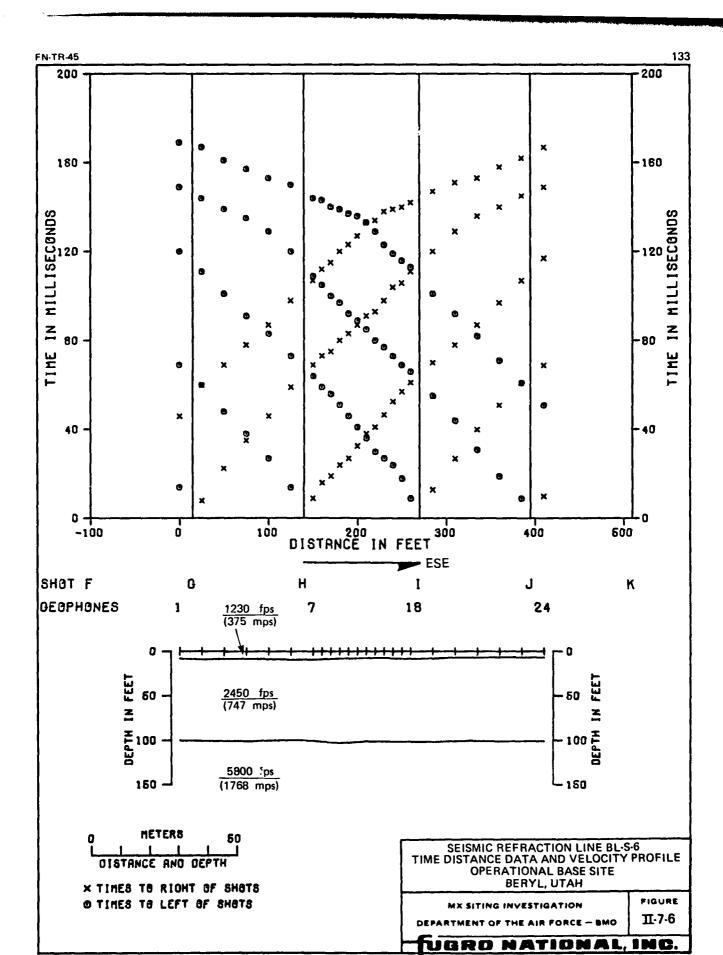
SEISMIC REFRACTION LINE BL-S-3 TIME DISTANCE DATA AND VELOCITY PROFILE **OPERATIONAL BASE SITE** BERYL, UTAH

MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - SMO FIGURE II-7-3

<u>JGRO NATIONAL, INC.</u>





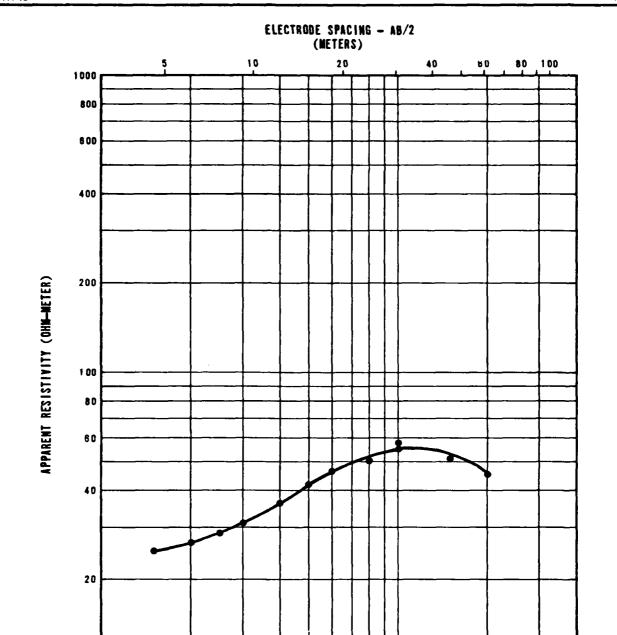


## 8.0 EXPLANATION OF ELECTRICAL RESISTIVITY DATA

Each figure in this section presents the data obtained from a resistivity sounding and a tabulated model of resistivity layers that would produce a curve similar to the observed curve. The upper portion of the figures is a graph in which measured apparent resistivity values in ohm-meters are plotted versus one-half the distance between the current electrodes.

The interpreted model tabulated at the bottom of the figures shows a combination of true resistivity layers and thicknesses obtained by matching theoretical curves to the field curve.

NOTE: There was no resistivity sounding done at location BL-SR-4.



60

ELECTRODE SPACING - AB/2
(FEET)

60 100

	INTERPRETED MODEL			
LAYER DEPTH		RESISTIVITY VALUES		
FEET	METERS	OHN-METER		
0	0	25		
21	6	120		
68	21	20		
	1			

20

10

RESISTIVITY SOUNDING BL-R-1 SOUNDING CURVE AND INTERPRETATION OPERATIONAL BASE SITE BERYL, UTAH

200

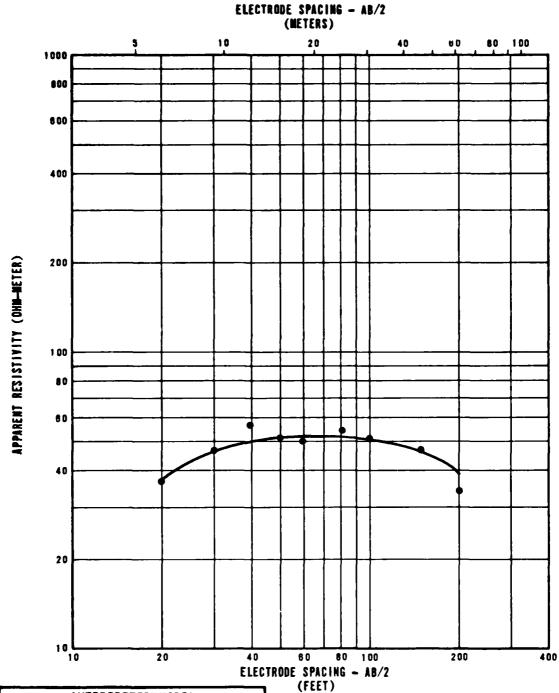
MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMG

FIEURE II-8-1

400

UGRO NATIONAL INC.

USAF-15



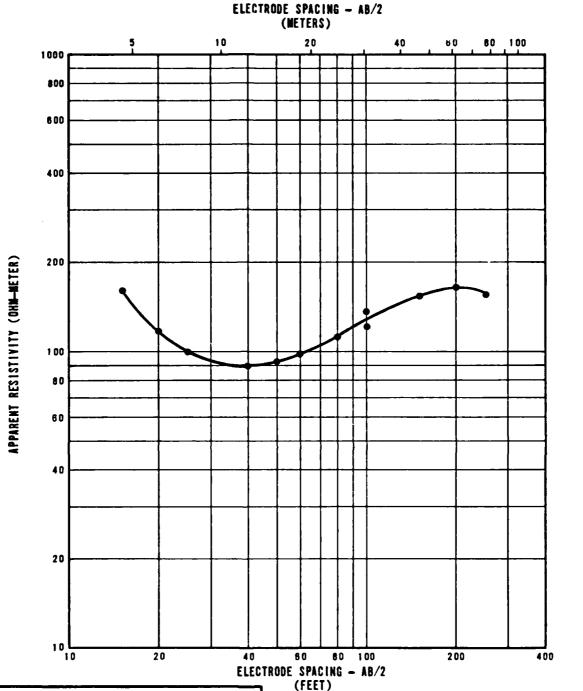
	INTERPRETED MODEL			
LAYE	R DEPTH	RESISTIVITY VALUES		
FEET	METERS	OHM-METER		
0	0	30		
6	2	60		
68	21	45		
104	32	25		
	1			

RESISTIVITY SOUNDING BL-R-2 SOUNDING CURVE AND INTERPRETATION OPERATIONAL BASE SITE BERYL, UTAH

UGRO NATIONAL INC.

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMO

II-8-2



INTERPRETED MODEL			
LAYER DEPTH		RESISTIVITY VALUE	
FEET	METERS	OHM-METER	
0	0	230	
8	2	55	
28	9	240	
128	39	100	
		1	

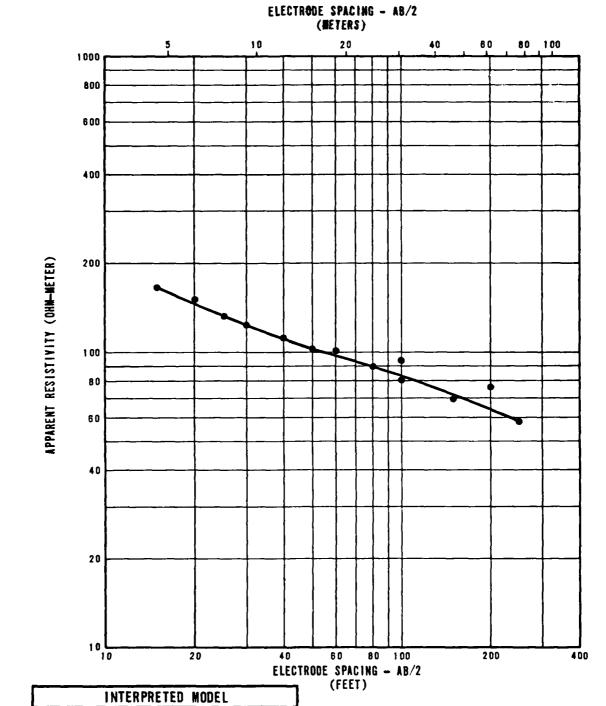
RESISTIVITY SOUNDING BL-R-3 SOUNDING CURVE AND INTERPRETATION OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMO

FIGURE II-8-3

UGRO NATIONAL

USAF-15



INTERPRETED MODEL			
LAYER DEPTH		RESISTIVITY VALUES	
FEET	METERS	OHM-METER	
0	0	180	
13	4	85	
88	27	40	
128	39	50	

RESISTIVITY SOUNDING BL-R-5 SOUNDING CURVE AND INTERPRETATION OPERATIONAL BASE SITE BERYL, UTAH

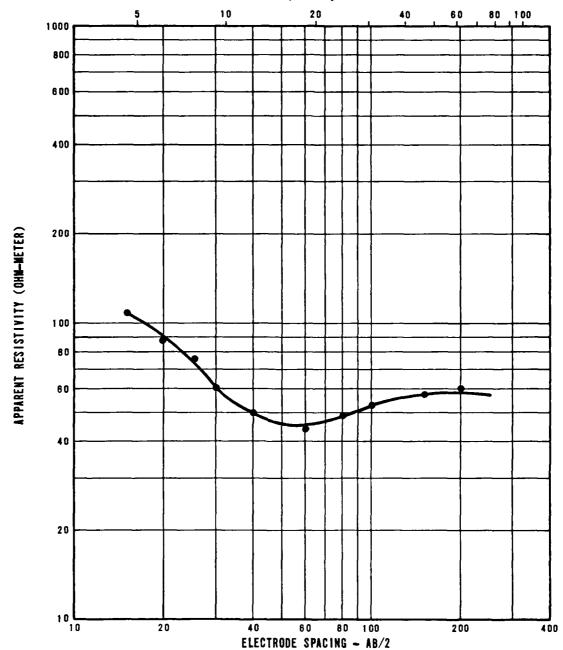
MX SITING INVESTIGATION DEPARTMENT OF THE AIR FORCE - BMD

II-8-4

UGRO NATIONAL, INC.

USAF-15





(FEET)

	INTERPRETED MODEL				
LAYE	R DEPTH	RESISTIVITY VALUES			
FEET	METERS	OHM-METER			
<u>O</u>	0	130			
12	4	25			
24	7	65			
177	.54	30			
	Ţ				

RESISTIVITY SOUNDING BL-R-6 SOUNDING CURVE AND INTERPRETATION OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BNO

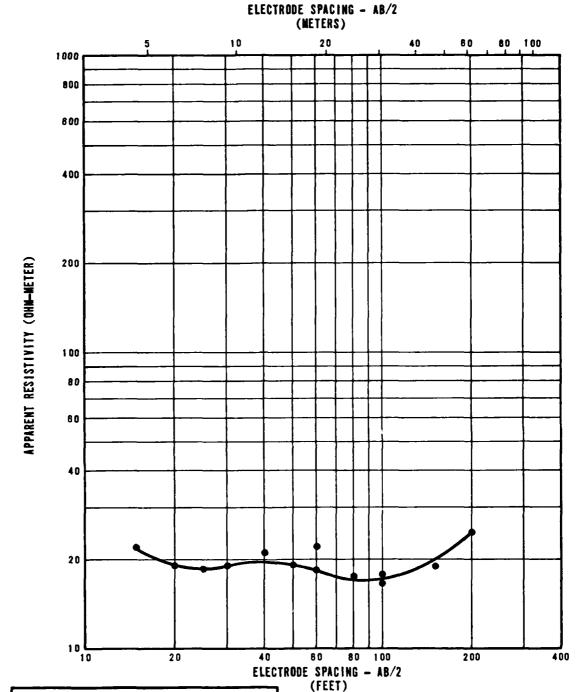
UGRO NATIONAL

FIGURE II-8-5

<u>, INC.</u>

20 MAR 81

USAF-15



	INTERPRETED MODEL				
LAYER DEPTH		RESISTIVITY VALUES			
FEET	METERS	OHM-METER			
0	0	25			
7	2	16			
20	6	25			
37	11	12			
110	34	95			

RESISTIVITY SOUNDING BL-R-7 SOUNDING CURVE AND INTERPRETATION OPERATIONAL BASE SITE BERYL, UTAH

MX SITING INVESTIGATION
DEPARTMENT OF THE AIR FORCE - BMG

II-8-6

UGRO NATIONAL

